RESOLUTION PC 17-041

A RESOLUTION OF THE PLANNING COMMISSION OF THE CITY OF EL PASO DE ROBLES RECOMMENDING APPROVAL TO THE CITY COUNCIL OF THE CITY OF EL PASO DE ROBLES ADOPTING A NEGATIVE DECLARATION FOR GENERAL PLAN AMENDMENT 16-001, REZONE 16-002, VESTING TENTATIVE PARCEL MAP PR 16-0231 & PLANNED DEVELOPMENT 13-006 AMENDMENT APPLICANT – MICHAEL MULLAHEY APN: 025-423-005 and 023

WHEREAS, North Coast Engineering, on behalf of Michael Mullahey, has filed an application requesting consideration of the following land use changes and entitlements in connection with the Mullahey Chrysler Dealership General Plan Amendment (the "Project"):

- **General Plan Amendment 16-001**: to change the existing land use designations as follows:
  - **Parcel 1 (PR 16-0231):** Business Park to Commercial Services
  - **Parcel 2 (PR 16-0231**): no change, remains Business Park
- **Rezone 16-002:** to change the existing zoning designations as follows:
  - **Parcel 1 (PR 16-0231):** PM (Planned Industrial) to C3 (Commercial/Light Industrial)
  - **Parcel 2 (PR 16-0231):** no change, remains PM (Planned Industrial)
- **Vesting Tentative Parcel Map 16-0231:**

  Request to subdivide a 1.8 acre site (APN 025-423-005) into two (2) parcels, where Parcel 1 would be .8 acres, and Parcel 2 would be 1 acre.

WHEREAS, pursuant to the Statutes and Guidelines of the California Environmental Quality Act (CEQA), Public Resources Code, Section 21000, et seq., and the City's Procedures for Implementing CEQA, an Initial Study and a Draft Negative Declaration ("ND") was prepared and circulated for a 20-day public review period beginning on September 5, 2017 and extended to October 3, 2017. The Draft ND/Initial Study dated September 5, 2017 is on file at the Paso Robles Community Development Department and available on line at http://www.prcity.com/government/departments/commdev/; and

WHEREAS, public notice of the proposed Draft ND was posted as required by Section 21092 of the Public Resources Code; and

WHEREAS, public hearings were conducted by the Planning Commission on September 12, 2017, to consider the Initial Study and the draft ND prepared for the proposed Project, and to accept public testimony on the proposed entitlements and environmental determination;

NOW, THEREFORE, BE IT RESOLVED, by the City Council of the City of Paso Robles, as follows:

Section 1. All of the recitals above are true and correct and incorporated herein.

Section 2. Based on the information and analysis contained in the Negative Declaration prepared for this project, the comments received during the public review period, and testimony received at the public hearing, the City Council finds that there is no substantial evidence supporting a fair argument that there would not be a significant impact on the environment. These findings are based on an independent review of the Initial Study, the Negative Declaration, and all comments received regarding the Negative Declaration, and based on the whole record. The City Council further finds that the Negative Declaration was prepared in compliance with CEQA and the CEQA Guidelines, that there is no substantial evidence that the Project will have a significant effect on the environment and the Negative Declaration reflects the independent judgment and analysis of the City Council.

Section 3. The City Council, based on its independent judgment and analysis, hereby adopts the Negative Declaration for the Mullahey Dealership General Plan Amendment Project, attached hereto as Exhibit A, including the comments received and responses thereto, attached hereto as Exhibit B, in accordance with the Statutes and Guidelines of the California Environmental Quality Act (CEQA) and the City's Procedures for Implementing CEQA. Exhibits A is hereby incorporated into this resolution.

PASSED AND ADOPTED by the Planning Commission of the City of Paso Robles this 12th day of September 2017 by the following vote:

AYES: Commissioners Barth, Davis, Agredano, Jorgensen, Brennan and Chairman Donaldson
NOES: Commissioner Rollins
ABSTAIN:
ABSENT:

John Donaldson, Chairman

ATTEST:

Warren Frace, Planning Commission Secretary

Exhibit A – Negative Declaration

EXHIBIT - A

ENVIRONMENTAL INITIAL STUDY CHECKLIST FORM
CITY OF PASO ROBLES

1. **PROJECT TITLE:** **Mullahey – Auto Service Expansion**

   Concurrent Entitlements: **General Plan Amendment 16-001, Rezone 16-002, PD 13-008 Amendment, and Vesting Tentative Parcel Map PR 16-0231**

2. **LEAD AGENCY:** City of Paso Robles
   1000 Spring Street
   Paso Robles, CA 93446

   Contact:
   Phone: (805) 237-3970
   Email:

3. **PROJECT LOCATION:** The subject 1.8 acre parcel (APN 025-423-005) is located at the south end of Danley Ct., south of Wisteria Lane, East of Golden Hill Road. The existing Mullahey dealership is located at 2520 Golden Hill Road, at the corner of Tractor Way.

4. **PROJECT PROPONENT:** **Michael Mullahey**

   Contact Person: **Same**

   Phone: **(805) 481-3673**
   Email: mjmullahey@charter.net

5. **GENERAL PLAN DESIGNATION:** **BP (Business Park)**

6. **ZONING:** **PM (Planned Industrial)**

7. **PROJECT DESCRIPTION:**

   The project consists of subdividing a 1.8 acre site (APN 025-423-005) into two (2) parcels, where Parcel 1 would be .8 acres, and Parcel 2 would be 1 acre. Along with the subdivision is a request to amend the General Plan and Zoning designations of new Parcel 1, and amend the existing development plan for the Mullahey Chrysler Dealership to add new Parcel 1 to as an expansion to the dealership.

In order to accommodate the proposed project, it is necessary to: (1) amend the General Plan - Land Use Element, land use designation diagram; (2) Zoning Map; (3) process Vesting Tentative Parcel Map PR 16-0231; and (4) amend PD 13-006 for the Mullahey Dealership as follows:

a) General Plan Amendment

To change the existing land use designations as follows:
- **Parcel 1 (PR 16-0231):** Business Park to Commercial Services
- **Parcel 2 (PR 16-0231**): no change, remains Business Park

b) Zoning Amendment

To change the existing zoning designations as follows:
- **Parcel 1 (PR 16-0231):** PM (Planned Industrial) to C3 (Commercial/Light Industrial)
- **Parcel 2 (PR 16-0231):** no change, remains PM (Planned Industrial)

c) Vesting Tentative Parcel Map 16-0231

- Proposing to subdivide a 1.8 acre site (APN 025-423-005) into two (2) parcels, where Parcel 1 would be .8 acres, and Parcel 2 would be 1 acre. See Attachment 5 – Parcel Map.

d) PD 13-006 Amendment

- Amend the development plan establishing the Mullahey dealership to include new Parcel 1 to allow for an expansion to the dealership for service, repair and parking. See Attachment 6 – Site Plan.

8. ENVIRONMENTAL SETTING: The 1.8 acre site is lot 5 of Tract 2269. The site is vacant site that was developed with curb, gutter, sidewalk and utilities with the original development of Tract 2269.

A Mitigated Negative Declaration (MND) was approved for Tract 2269 (Res. 98-001) that identified that with the development of Tract 2269, airport compatibility, circulation, water, drainage, open space, and aesthetics, would be impacts that would need further mitigation to reduce the impacts to less than significant. The mitigation measures are outlined in the Tentative Tract Resolution (Res. 98-014) and the Development Plan Resolution for PD 97-013 (Res. 98-002) and will be discussed in the corresponding section of this Initial Study Checklist. Generally, most of the mitigation measures listed in Res. 98-014 were completed with the public improvements and the recording of the tract map. This Initial Study indicates that the proposed Mullahey expansion project has no additional environmental impacts.

9. OTHER AGENCIES WHOSE APPROVAL IS REQUIRED (AND PERMITS NEEDED): None.

Exhibit A

ENVIRONMENTAL FACTORS POTENTIALLY AFFECTED:

The environmental factors checked below would be potentially affected by this project, involving at least one impact that is a "Potentially Significant Impact" as indicated by the checklist on the following pages.

☐ Aesthetics	☐ Agriculture and Forestry Resources	☐ Air Quality
☐ Biological Resources	☐ Cultural Resources	☐ Geology /Soils
☐ Greenhouse Gas Emissions	☐ Hazards & Hazardous Materials	☐ Hydrology / Water Quality
☐ Land Use / Planning	☐ Mineral Resources	☐ Noise
☐ Population / Housing	☐ Public Services	☐ Recreation
☐ Transportation/Traffic	☐ Utilities / Service Systems	☐ Mandatory Findings of Significance

DETERMINATION: (To be completed by the Lead Agency)

On the basis of this initial evaluation:

☒ I find that the proposed project COULD NOT have a significant effect on the environment, and a NEGATIVE DECLARATION will be prepared.

☐ I find that although the proposed project could have a significant effect on the environment, there will not be a significant effect in this case because revisions in the project have been made by or agreed to by the project proponent. A MITIGATED NEGATIVE DECLARATION will be prepared.

☐ I find that the proposed project MAY have a significant effect on the environment, and an ENVIRONMENTAL IMPACT REPORT is required.

☐ I find that the proposed project MAY have a "potentially significant impact" or "potentially significant unless mitigated" impact on the environment, but at least one effect 1) has been adequately analyzed in an earlier document pursuant to applicable legal standards, and 2) has been addressed by mitigation measures based on the earlier analysis as described on attached sheets. An ENVIRONMENTAL IMPACT REPORT is required, but it must analyze only the effects that remain to be addressed.

☐ I find that although the proposed project could have a significant effect on the environment, because all potentially significant effects (a) have been analyzed adequately in an earlier EIR or NEGATIVE DECLARATION pursuant to applicable standards, and (b) have been avoided or mitigated pursuant to that earlier EIR or NEGATIVE DECLARATION, including revisions or mitigation measures that are imposed upon the proposed project, nothing further is required.

Signature: _____ Date _____

Exhibit A

EVALUATION OF ENVIRONMENTAL IMPACTS:

1. A brief explanation is required for all answers except "No Impact" answers that are adequately supported by the information sources a lead agency cites in the parentheses following each question. A "No Impact" answer is adequately supported if the referenced information sources show that the impact simply does not apply to projects like the one involved (e.g., the project falls outside a fault rupture zone). A "No Impact" answer should be explained where it is based on project-specific factors as well as general standards (e.g., the project will not expose sensitive receptors to pollutants, based on a project-specific screening analysis).

2. All answers must take account of the whole action involved. Answers should address off-site as well as on-site, cumulative as well as project-level, indirect as well as direct, and construction as well as operational impacts.

3. "Once the lead agency has determined that a particular physical impact may occur, then the checklist answers must indicate whether the impact is potentially significant, less than significant with mitigation, or less than significant. "Potentially Significant Impact" is appropriate if there is substantial evidence that an effect may be significant. If there are one or more "Potentially Significant Impact" entries when the determination is made, an EIR is required.

4. "Negative Declaration: Less Than Significant With Mitigation Incorporated" applies where the incorporation of mitigation measures has reduced an effect from ""Potentially Significant Impact" to a "Less Than Significant Impact." The lead agency must describe the mitigation measures, and briefly explain how they reduce the effect to a less than significant level (mitigation measures from "Earlier Analyses," as described in (5) below, may be cross-referenced).

5. Earlier analyses may be used where, pursuant to the tiering, program EIR, or other CEQA process, an effect has been adequately analyzed in an earlier EIR or negative declaration. Section 15063(c)(3)(D). In this case, a brief discussion should identify the following:

    a. Earlier Analysis Used. Identify and state where they are available for review.
    b. Impacts Adequately Addressed. Identify which effects from the above checklist were within the scope of and adequately analyzed in an earlier document pursuant to applicable legal standards, and state whether such effects were addressed by mitigation measures based on the earlier analysis.
    c. Mitigation Measures. For effects that are "Less than Significant with Mitigation Measures Incorporated," describe the mitigation measures which were incorporated or refined from the earlier document and the extent to which they address site-specific conditions for the project.

6. Lead agencies are encouraged to incorporate into the checklist references to information sources for potential impacts (e.g., general plans, zoning ordinances). Reference to a previously prepared or outside document should, where appropriate, include a reference to the page or pages where the statement is substantiated.

7. Supporting Information Sources: A source list should be attached, and other sources used or individuals contacted should be cited in the discussion.

8. The explanation of each issue should identify:
    a. the significance criteria or threshold, if any, used to evaluate each question; and
    b. the mitigation measure identified, if any, to reduce the impact to less than significance

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
I. AESTHETICS: Would the project:				
a. Have a substantial adverse effect on a scenic vista?	☐	☐	☐	☒
Discussion: The project site is not located within a scenic vista.				
b. Substantially damage scenic resources, including, but not limited to, trees, rock outcroppings, and historic buildings within a state scenic highway?	☐	☐	☐	☒
Discussion: The site is not considered a scenic resource and is not located along a state scenic highway, and there are no historic buildings located on this site.				
c. Substantially degrade the existing visual character or quality of the site and its surroundings?	☐	☐	☒	☐
Discussion: Aesthetics was one of the impacts that were identified in the MND for Tract 2281. Condition No. 3 and 13 of the Res. 98-002 indicated the use of decorative masonry materials for any walls along Golden Hill Road and the eastern tract boundary. Also indicated was the requirement to use non-reflective building materials. Condition No. 24 in the Res. 98-014 indicated a landscaping plan for landscape screening along the tract eastern boundary. Since this project is not adjacent to Golden Hill Road, a decorative masonry wall is not required. The project proposes to utilize mainly metal panels for siding and roofing for the new service building. The neutral color of the metal siding and roofing will prevent it from being reflective. Lots 5 (project site) does not border the eastern boundary of Tract 2269, therefore the conditions related to the landscaping along the eastern boundary would not apply to this project. However, the project has provided a landscape plan that will help complement the site and building architecture.				
d. Create a new source of substantial light or glare which would adversely affect day or nighttime views in the area? (Sources: 1, 2, 10)	☐	☐	☐	☒
Discussion: Any new exterior lighting will be required to be shielded so that it does not produce off-site glare.				
II. AGRICULTURE AND FOREST RESOURCES: In determining whether impacts to agricultural resources are significant environmental effects, lead agencies may refer to the California Agricultural Land Evaluation and Site Assessment Model (1997) prepared by the California Dept. of Conservation as an optional model to use in assessing impacts on agriculture and farmland. Would the project:				
a. Convert Prime Farmland, Unique Farmland, or Farmland of Statewide Importance (Farmland), as shown on the maps prepared pursuant to the Farmland Mapping and Monitoring Program of the California Resources Agency, to non-agricultural use?	☐	☐	☐	☒

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

Discussion: The project is not located on agriculturally zoned land and there are no agricultural activities taking place on the site.

b. Conflict with existing zoning for agricultural use, or a Williamson Act contract? ☐ ☐ ☐ ☒

Discussion: See discussion section for Section II.a.

c. Conflict with existing zoning for, or cause rezoning of, forest, land (as defined in Public Resources Code section 12220(g)), timberland (as defined by Public Resources Code section 4526), or timberland zoned Timberland Production (as defined by Government Code section 5114(g))? ☐ ☐ ☐ ☒

Discussion: The project is not located on agriculturally zoned land and there are no agricultural activities taking place on the site.

d. Result in the loss of forest land or conversion of forest land to non-forest use? ☐ ☐ ☐ ☒

Discussion: The project is not located on land zoned for forest purposes.

e. Involve other changes in the existing environment which, due to their location or nature, could result in conversion of Farmland, to non-agricultural use or conversion of forest land to non-forest use? ☐ ☐ ☐ ☒

Discussion: This project would not result in the conversion of farmland or forest land.

III. AIR QUALITY: Where available, the significance criteria established by the applicable air quality management or air pollution control district may be relied upon to make the following determinations. Would the project:

a. Conflict with or obstruct implementation of the applicable air quality plan? (Source: 11) ☐ ☐ ☒ ☐

Discussion: The San Luis Obispo County area is a non-attainment area for the State standards for ozone and suspended particulate matter. The SLO County Air Pollution Control District (APCD) administers a permit system to ensure that stationary sources do not collectively create emissions which would cause local and state standards to be exceeded. The potential for future project development to create adverse air quality impacts falls generally into two categories: Short term and Long term impacts.

Short term impacts are associated with the grading and development portion of a project where earth work generates dust, but the impact ends when construction is complete. Long term impacts are related to the ongoing operational characteristics of a project and are generally related to vehicular trip generation and the level of offensiveness of the onsite activity being developed.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

There will be short term impacts associated with grading for the proposed construction, standard conditions required by the City as well as the APCD will be implemented.

When reviewing the grading of the 1.8-acre site, since the disturbed area of grading activity is approximately 1 acre, it falls under the 4-acre threshold described in footnote 2 of Table 2-1 of the APCD CEQA Handbook (April 2012), indicating that the pollutants produced as a result of construction activities is less than the 2.5 ton PM 10 quarterly threshold. Therefore impacts to air quality as a result of this grading project, are considered less than significant and no mitigation is required. Standard conditions related to dust control will be required with the issuance of a grading permit for this project.

Furthermore, a condition of approval will be added that requires the project be designed to incorporate all feasible standard measures outlined in condition No. 9 of Res. 98-002, related to site design measures related to energy efficiency.

b.	Violate any air quality standard or contribute substantially to an existing or projected air quality violation? (Source: 11)	☐	☐	☐	☒

Discussion: See Section III.a

c.	Result in a cumulatively considerable net increase of any criteria pollutant for which the project region is non-attainment under an applicable federal or state ambient air quality standard (including releasing emissions which exceed quantitative thresholds for ozone precursors)? (Source: 11)	☐	☐	☐	☒

Discussion: See Section III.a

d.	Expose sensitive receptors to substantial pollutant concentrations? (Source: 11)	☐	☐	☐	☒

Discussion: Besides the short term impacts from the actual grading, there will not be a significant impact to sensitive receptors.

e.	Create objectionable odors affecting a substantial number of people? (Source: 11)	☐	☐	☐	☒

Discussion: Based on the automotive repair and associated parking lot being in an industrial area, with significant setbacks to neighboring properties/buildings, and since auto repair use does not produce odor, is not anticipated that this project will have an affect a substantial number of people. Therefore, there is no impact.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
IV. BIOLOGICAL RESOURCES: Would the project:				
a. Have a substantial adverse effect, either directly or through habitat modifications, on any species identified as a candidate, sensitive, or special status species in local or regional plans, policies, or regulations, or by the California Department of Fish and Game or U.S. Fish and Wildlife Service?	☐	☐	☐	☒
b. Have a substantial adverse effect on any riparian habitat or other sensitive natural community identified in local or regional plans, policies, regulations or by the California Department of Fish and Game or US Fish and Wildlife Service?	☐	☐	☐	☒
c. Have a substantial adverse effect on federally protected wetlands as defined by Section 404 of the Clean Water Act (including, but not limited to, marsh, vernal pool, coastal, etc.) through direct removal, filling, hydrological interruption, or other means?	☐	☐	☐	☒
d. Interfere substantially with the movement of any native resident or migratory fish or wildlife species or with established native resident or migratory wildlife corridors, or impede the use of native wildlife nursery sites?	☐	☐	☐	☒
e. Conflict with any local policies or ordinances protecting biological resources, such as a tree preservation policy or ordinance?	☐	☐	☐	☒
f. Conflict with the provisions of an adopted Habitat Conservation Plan, Natural Community Conservation Plan, or other approved local, regional, or state habitat conservation plan?	☐	☐	☐	☒

Discussion (a-f):

Any biological resource mitigation requirements that were required with the development of Tract 2269 have been completed. The subject lot was rough graded with the development of Tract 2269. The public improvements including streets, curb, gutter and sidewalk improvements have been completed for the projects frontage on Danley Court. Since this lot has been developed, including grading, street improvements and utilities and since the lot is flat and has no resources except for seasonal grasses, the development of Lot 5 of Tract 2269 will not have an impact on biological services.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
V. CULTURAL RESOURCES: Would the project:				
a. Cause a substantial adverse change in the significance of a historical resource as defined in §15064.5?	☐	☐	☐	☒
b. Cause a substantial adverse change in the significance of an archaeological resource pursuant to §15064.5?	☐	☐	☐	☒
c. Directly or indirectly destroy a unique paleontological resource or site or unique geologic feature?	☐	☐	☐	☒
d. Disturb any human remains, including those interred outside of formal cemeteries?	☐	☐	☒	☐

Discussion (a-d):

An Archeological Survey was conducted in 1996, by Clay Singer, in relation to a 226 acre site that included the land within Tract 2269. The Study indicated that no prehistoric resources of any kind were identified and the Study concluded that development of the project at that time (Golf Course) should have no impact on known or cultural resources. The following standard condition will be applied to this project.

In the event that buried or otherwise unknown cultural resources are discovered during construction work in the area of the find, work shall be suspended and the City of Paso Robles should be contacted immediately, and appropriate mitigations measures shall be developed by qualified archeologist or historian if necessary, at the developers expense.

Additionally, in relation to AB18, the City sent a letter out to seven tribes inviting consultation pursuant to SB 18. The City received one request by Patti Dutton, Tribal Administrator for the Salinan Tribe of Monterey & San Luis Obispo Counties. Ms. Dutton requested that a Phase I cultural study be done for the project.

The project site is located in an area that is not adjacent to a creek or stream, or in an area that typically considered culturally significant.

Based on this project being evaluated as part of archeological survey in 1996, where no prehistoric resources of any kind were found, and since this particular parcel was rough graded in the early 2000, including the installation of road improvements and utilities to this site, the development of this parcel does not warrant a new cultural survey. The standard condition related to work being suspended if cultural resources are discovered, will be applied to the project. Therefore this projects impacts on Cultural Resources is less than significant.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

VI. GEOLOGY AND SOILS: Would the project:

a. Expose people or structures to potential substantial adverse effects, including the risk of loss, injury, or death involving:

i. Rupture of a known earthquake fault, as delineated on the most recent Alquist-Priolo Earthquake Fault Zoning Map issued by the State Geologist for the area or based on other substantial evidence of a known fault? Refer to Division of Mines and Geology Special Publication 42. (Sources: 1, 2, & 3) ☐ ☐ ☒ ☐

Discussion: The potential for and mitigation of impacts that may result from fault rupture in the project area are identified and addressed in the General Plan EIR, pg. 4.5-8. There are two known fault zones on either side of the Salinas Rivers valley. The Rinconada Fault system runs on the west side of the valley, and grazes the City on its western boundary. The San Andreas Fault is on the east side of the valley and is situated about 30 miles east of Paso Robles. The City of Paso Robles recognizes these geologic influences in the application of the Uniform Building Code to all new development within the City. Review of available information and examinations indicate that neither of these faults is active with respect to ground rupture in Paso Robles. Soils and geotechnical reports and structural engineering in accordance with local seismic influences would be applied in conjunction with any new development proposal. Based on standard conditions of approval, the potential for fault rupture and exposure of persons or property to seismic hazards is not considered significant. There are no Alquist-Priolo Earthquake Fault Zones within City limits.

ii. Strong seismic ground shaking? (Sources: 1, 2, & 3) ☐ ☐ ☒ ☐

Discussion: The proposed project will be constructed to current CBC codes. The General Plan EIR identified impacts resulting from ground shaking as less than significant and provided mitigation measures that will be incorporated into the design of this project including adequate structural design and not constructing over active or potentially active faults.

iii. Seismic-related ground failure, including liquefaction? (Sources: 1, 2 & 3) ☐ ☐ ☒ ☐

Discussion: Per the General Plan EIR, the project site is located in an area with soil conditions that have a potential for liquefaction or other type of ground failure due to seismic events and soil conditions. To implement the EIR's mitigation measures to reduce this potential impact, the City has a standard condition to require submittal of soils and geotechnical reports, which include site-specific analysis of liquefaction potential for all building permits for new construction, and incorporation of the recommendations of said reports into the design of the project.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
iv. Landslides?	☐	☐	☐	☒

Discussion: See discussions above.

b. Result in substantial soil erosion or the loss of topsoil? (Sources: 1, 2, & 3)	☐	☐	☒	☐

Discussion: Per the General Plan EIR the soil condition is not erosive or otherwise unstable. As such, no significant impacts are anticipated. A geotechnical/ soils analysis will be required prior to issuance of building permits that will evaluate the site specific soil stability and suitability of grading and retaining walls proposed. This study will determine the necessary grading techniques that will ensure that potential impacts due to soil stability will not occur. An erosion control plan shall be required to be approved by the City Engineer prior to commencement of site grading.

c. Be located on a geologic unit or soil that is unstable, or that would become unstable as a result of the project, and potentially result in on- or off-site landslide, lateral spreading, subsidence, liquefaction or collapse?	☐	☐	☐	☐

Discussion: See response to item a.iii, above.

d. Be located on expansive soil, as defined in Table 18-1-B of the Uniform Building Code (1994), creating substantial risks to life or property?	☐	☐	☒	☐

Discussion: See response to item a.iii, above.

e. Have soils incapable of adequately supporting the use of septic tanks or alternative waste water disposal systems where sewers are not available for the disposal of waste water?	☐	☐	☐	☒

Discussion: The building will be hooked up to the City's sanitary sewer system, therefore there is no impact.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
VII. GREENHOUSE GAS EMISSIONS: Would the project:				
a. Generate greenhouse gas emissions, either directly or indirectly, that may have a significant impact on the environment?	☐	☐	☒	☐
b. Conflict with any applicable plan, policy, or regulation of an agency adopted for the purpose of reducing the emissions of greenhouse gasses?	☐	☐	☒	☐

Discussion (a-b):

When reviewing the grading of the 4-acre site with the APCD CEQA Handbook (April 2012), the project would produce less than the 25 lbs/day of ROG+NOx and therefore be considered less than significant and no mitigation is required for operational or long-term impacts based on outdoor storage land use. Standard conditions related to dust control will be required with the issuance of a grading permit for this project.

VIII. HAZARDS AND HAZARDOUS MATERIALS: Would the project:				
a. Create a significant hazard to the public or the environment through the routine transport, use, or disposal of hazardous materials?	☐	☐	☐	☒
b. Create a significant hazard to the public or the environment through reasonably foreseeable upset and accident conditions involving the release of hazardous materials into the environment?	☐	☐	☐	☒
c. Emit hazardous emissions or handle hazardous or acutely hazardous materials, substances, or waste within one-quarter mile of an existing or proposed school?	☐	☐	☐	☒
d. Be located on a site which is included on a list of hazardous materials sites compiled pursuant to Government Code Section 65962.5 and, as a result, would it create a significant hazard to the public or the environment?	☐	☐	☒	☐

Discussion (a-d): the project will include a parking lot area for employee and cars waiting for service, along with the construction of a 3,000 square foot, 5-bay service building. the transport of wine grapes, processed wine, and the byproduct of the wine (pumice). The auto repair facility will be required to comply with all local and State requirements for automotive related fluids handling and disposal. The site is vacant and not

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

included on a hazardous materials site list. The development and operation of the auto repair facility would not create a hazard, or use/produce hazardous materials, that are not already controlled by the County and State permitting for automotive repair facilities.

e. For a project located within an airport land use plan or, where such a plan has not been adopted, within two miles of a public airport or public use airport, would the project result in a safety hazard for people residing or working in the project area?	☐	☐	☒	☐

Discussion (e): The project is in the vicinity of the City's Municipal Airport. It is located within Safety Zone 5 as outlined in the City's Airport Land Use Plan. According to the Airport Land Use Compatibility Matrix, wineries are considered 'compatible' in Zone 5, without any conditions, therefore impacts related to safety from the airport would be less than significant.

f. For a project within the vicinity of a private airstrip, would the project result in a safety hazard for people residing or working in the project area?	☐	☐	☐	☒

Discussion (f): There are no know private air strips in the vicinity of the project site, therefore there is no impact.

g. Impair implementation of or physically interfere with an adopted emergency response plan or emergency evacuation plan?	☐	☐	☐	☒

h. Expose people or structures to a significant risk of loss, injury or death involving wildland fires, including where wildlands are adjacent to urbanized areas or where residences are intermixed with wildlands?	☐	☐	☐	☒

Discussion (g,h):

The development of the facility within the existing industrial park will not expose people to wildland fires, and is not adjacent to wildlands, therefore there will not be an impact.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

IX. HYDROLOGY AND WATER QUALITY: Would the project:

a. Violate any water quality standards or waste discharge requirements? ☐ ☐ ☒ ☐

Discussion: A preliminary grading and drainage plan has been designed for the project by North Coast Engineering. The proposed project is designed to retain stormwater on-site through installation of various low-impact development (LID) features. The project has been designed to reduce impervious surfaces, preserve existing vegetation, and promote groundwater recharge by employing bioretention through implementation of these measures. Thus, water quality standards will be maintained and discharge requirements will be in compliance with State and local regulations. Therefore, impacts to water quality and discharge will be less than significant.

b. Substantially deplete groundwater supplies or interfere substantially with groundwater recharge such that there would be a net deficit in aquifer volume or a lowering of the local groundwater table level (e.g., Would the production rate of pre-existing nearby wells drop to a level which would not support existing land uses or planned uses for which permits have been granted)? Would decreased rainfall infiltration or groundwater recharge reduce stream baseflow? (Source: 7) ☐ ☐ ☒ ☐

Discussion: The addition of the auto repair building and parking lot area along with the associated landscaping will not deplete groundwater supplies or have a significant impact on groundwater. The project will be required to hook up to City water. This project impact on groundwater is less than significant.

c. Substantially alter the existing drainage pattern of the site or area, including through the alteration of the course of a stream or river, in a manner which would result in substantial erosion or siltation on- or off-site? (Source: 10) ☐ ☐ ☐ ☒

See Discussion for IXa.

d. Substantially alter the existing drainage pattern of the site or area, including through the alteration of the course of a stream or river, or substantially increase the rate or amount of surface runoff in a manner which would result in flooding on- or off-site? (Source: 10) ☐ ☐ ☐ ☒

See Discussion for IXa.

		Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
e.	Create or contribute runoff water which would exceed the capacity of existing or planned stormwater drainage systems or provide substantial additional sources of polluted runoff? (Source: 10)	☐	☐	☐	☒
	See Discussion for IXa.				
f.	Otherwise substantially degrade water quality?	☐	☐	☐	☒
	See Discussion for IXa.				
g.	Place housing within a 100-year flood hazard area as mapped on a federal Flood Hazard Boundary or Flood Insurance Rate Map or other flood hazard delineation map?	☐	☐	☐	☒
h.	Place within a 100-year flood hazard area structures which would impede or redirect flood flows?	☐	☐	☐	☒
i.	Expose people or structures to a significant risk of loss, injury or death involving flooding, including flooding as a result of the failure of a levee or dam?	☐	☐	☐	☒
j.	Inundation by mudflow?	☐	☐	☐	☒
k.	Conflict with any Best Management Practices found within the City's Storm Water Management Plan?	☐	☐	☒	☐
l.	Substantially decrease or degrade watershed storage of runoff, wetlands, riparian areas, aquatic habitat, or associated buffer zones?	☐	☐	☒	☐

Discussion (c-l):

The site is not located within a flood hazard area and the subject buildings will be utilizing City water and sewer systems. The projects impacts related to hydrological and water quality issues will be less than significant since the project will be required to comply with the City's standards related to site drainage, storm water run-off, water quality and water supply.

The Regional Water Quality Control Board adopted storm water management requirements for development projects in the Central Coast region. Upon the Board's direction, the City has adopted a Storm Water Ordinance requiring all projects to implement low impact development best management practices to mitigate impacts to the quality of storm water run-off and to limit the increase in the rate and volume of storm water

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact

run-off to the maximum extent practical.

These new requirements include on-site retention of stormwater. The applicant has prepared a storm water control plan offering a site assessment of constraints and opportunities and corresponding storm water management strategies to meet stormwater quality treatment and retention requirements in compliance with the regulations. The grading plan refects these requirements with three bio-retention treatment areas.

Thus, water quality standards will be maintained and discharge requirements will be in compliance with State and local regulations. Therefore, impacts to water quality and discharge will be less than significant.

X. LAND USE AND PLANNING: Would the project:

a. Physically divide an established community?	☐	☐	☐	☒

Discussion: The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot. The project will not divide an established community.

b. Conflict with any applicable land use plan, policy, or regulation of an agency with jurisdiction over the project (including, but not limited to the general plan, specific plan, local coastal program, or zoning ordinance) adopted for the purpose of avoiding or mitigating an environmental effect?	☐	☐	☐	☒

Discussion:

The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot.

With the change in land use and zoning designations for Parcel 1, the auto repair use as an accessory to the new car dealership is permitted. Parcel 1 will remain with the business park designation and be consistent with the existing land use and zoning for the business park (Tract 2269). Therefore, there will not be a conflict with the City's General Plan, Zoning Ordinance.

c. Conflict with any applicable habitat conservation plan or natural community conservation plan?	☐	☐	☐	☒

Discussion: There are no habitat conservation plans or natural community conservation plans established in this area of the City. Therefore there is no impact.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
XI. MINERAL RESOURCES: Would the project:				
a. Result in the loss of availability of a known mineral resource that would be of value to the region and the residents of the state? (Source: 1)	☐	☐	☐	☒
Discussion: There are no known mineral resources at this project site.				
b. Result in the loss of availability of a locally-important mineral resource recovery site delineated on a local general plan, specific plan or other land use plan? (Source: 1)	☐	☐	☐	☒
Discussion: There are no known mineral resources at this project site.				
XII. NOISE: Would the project result in:				
a. Exposure of persons to or generation of noise levels in excess of standards established in the local general plan or noise ordinance, or applicable standards of other agencies? (Source: 1)	☐	☐	☒	☐
Discussion: There will be the generation of noise as a result of the auto repair component of this project such as air tools and air compressors. That being said, the new auto service building will be surrounded by existing and future commercial/industrial uses. Since the auto repair use will have hours of operation between 7am and 7pm, noise from the business will be insignificant. It is not anticipated that the noise generated from the auto repair use would exceed the 70db threshold outlined in the Noise Element (Figure N-4) for industrial uses, or exceed the stationary noise decibel thresholds listed in Table N-5. Therefore, the projects impacts on noise exposure will be less than significant.				
b. Exposure of persons to or generation of excessive groundborne vibration or groundborne noise levels?	☐	☐	☒	☐
Discussion: There may be temporary vibrations related to the grading and compaction of the site in preparation for construction. The construction phase of the project will be required to comply with the City's noise level requirements, including hours of construction activity, and as a result of these standard construction requirements, impacts from vibrations as a result of construction activity will be less than significant.				
c. A substantial permanent increase in ambient noise levels in the project vicinity above levels existing without the project?	☐	☐	☒	☐
Discussion: See section XIIa				

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
d. A substantial temporary or periodic increase in ambient noise levels in the project vicinity above levels existing without the project?	☐	☐	☒	☐
Discussion: See section XIIa				
e. For a project located within an airport land use plan or, where such a plan has not been adopted, within two miles of a public airport or public use airport, would the project expose people residing or working in the project area to excessive noise levels? (Sources: 1, 4)	☐	☐	☒	☐

Discussion: The project is located within Safety Zone 5 of the Airport Land Use Plan, and is just over 1 mile of the Airport property. Auto dealerships and auto repair are considered compatible uses with the Airport for Zone 5, and therefore impacts on customers and employees of the dealership from noise related to aircraft would be less than significant.

XIII. POPULATION AND HOUSING: Would the project:

a. Induce substantial population growth in an area, either directly (for example, by proposing new homes and businesses) or indirectly (for example, through extension of roads or other infrastructure)? (Source: 1)	☐	☐	☐	☒
b. Displace substantial numbers of existing housing, necessitating the construction of replacement housing elsewhere?	☐	☐	☐	☒
c. Displace substantial numbers of people, necessitating the construction of replacement housing elsewhere?	☐	☐	☐	☒

Discussion (a-c):

The project will not create induce population growth, displace housing or people.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
XIV. PUBLIC SERVICES: Would the project result in substantial adverse physical impacts associated with the provision of new or physically altered governmental facilities, need for new or physically altered governmental facilities, the construction of which could cause significant environmental impacts, in order to maintain acceptable service ratios, response times or other performance objectives for any of the public services:				
a. Fire protection? (Sources: 1,10)	☐	☐	☐	☒
b. Police protection? (Sources: 1,10)	☐	☐	☐	☒
c. Schools?	☐	☐	☐	☒
d. Parks?	☐	☐	☐	☒
e. Other public facilities? (Sources: 1,10)	☐	☐	☐	☒

Discussion (a-e):

The project will be located within an existing industrial/business park. The addition of the building will not create a significant impact to public services.

XV. RECREATION				
a. Would the project increase the use of existing neighborhood and regional parks or other recreational facilities such that substantial physical deterioration of the facility would occur or be accelerated?	☐	☐	☐	☒
b. Does the project include recreational facilities or require the construction or expansion of recreational facilities which might have an adverse physical effect on the environment?	☐	☐	☐	☒

Discussion (a&b):

The project will not impact recreational facilities.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
XVI. TRANSPORTATION/TRAFFIC: Would the project:				
a. Conflict with an applicable plan, ordinance or policy establishing measures or effectiveness for the performance of the circulation system, taking into account all modes of transportation including mass transit and non-motorized travel and relevant components of the circulation system, including but not limited to intersections, streets, highways and freeways, pedestrian and bicycle paths, and mass transit?	☐	☐	☒	☐
b. Conflict with an applicable congestion management program, including, but not limited to level of service standards and travel demand measures, or other standards established by the county congestion management agency for designated roads or highways?	☐	☐	☒	☐

Discussion (a,b):

The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot.

Based on the proposed auto repair expansion to the existing dealership is a permitted use in the C3 zone, consistent with the CS land use designation, and subject to the standard condition of paying traffic impact fees, impacts from the development and operation of this project on the circulation system in the area of this project will be less than significant.

c. Result in a change in air traffic patterns, including either an increase in traffic levels or a change in location that results in substantial safety risks?	☐	☐	☐	☒

Discussion (c):

The development of this project within the established industrial subdivision will not impact air traffic patterns or increase air traffic levels.

d. Substantially increase hazards due to a design feature (e.g., sharp curves or dangerous intersections) or incompatible uses (e.g., farm equipment)?	☐	☐	☐	☒

Discussion (d): The proposed project would utilize the exisiting driveways off of Danley Court and from Tractor Street. There will be no hazards from design features.

		Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
e.	Result in inadequate emergency access?	☐	☐	☐	☒

Discussion (e):

The project has been reviewed by the City's Emergency Services Department, and based on the property having multiple access points to multiple streets, the ability for emergency access to the site is acceptable, and therefore considered adequate.

f.	Conflict with adopted policies, plans, or programs regarding public transit, bicycle, or pedestrian facilities, or otherwise decrease the performance or safety of such facilities?	☐	☐	☐	☒

Discussion (a-f):

The development of this project within an established industrial park would not conflict with adopted public transit, bicycle or pedestrian facilities, or decrease performance or safety of the facilities.

XVII. UTILITIES AND SERVICE SYSTEMS: Would the project:

a.	Exceed wastewater treatment requirements of the applicable Regional Water Quality Control Board?	☐	☐	☒	☐

Discussion: The project will comply with all applicable wastewater treatment requirements as required by the City, the Regional Water Quality Control Board, and the State Water Board Therefore, there will be less than significant impacts resulting from wastewater treatment from this project.

b.	Require or result in the construction of new water or wastewater treatment facilities or expansion of existing facilities, the construction of which could cause significant environmental effects?	☐	☐	☐	☒

Discussion:

The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot. The project will not necessitate the need for new treatment facilities, therefore, there is no impact.

c.	Require or result in the construction of new storm water drainage facilities or expansion of existing facilities, the construction of which could cause significant environmental effects?	☐	☐	☐	☒

Discussion: (c):
The project is located within an existing industrial subdivision where the infrastructure including storm drain systems have been installed. No new off-site storm drainage facilities will be required to be constructed with this project, therefore there is no impact.

		Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
d.	Have sufficient water supplies available to serve the project from existing entitlements and resources, or are new or expanded entitlements needed?	☐	☐	☒	☐

Discussion: As noted in section IX on Hydrology, the project can be served with existing water resource allocations available and will not require expansion of new water resource entitlements.

e.	Result in a determination by the wastewater treatment provider which serves or may serve the project that it has adequate capacity to serve the project's projected demand in addition to the provider's existing commitments?	☐	☐	☐	☒

Discussion: The auto repair use and parking lot will not have an impact on the City's waste water treatment facility.

f.	Be served by a landfill with sufficient permitted capacity to accommodate the project's solid waste disposal needs?	☐	☐	☐	☒

Discussion: Per the City's Landfill Master Plan, the City's landfill has adequate capacity to accommodate construction-related and operational solid waste disposal for this project.

g.	Comply with federal, state, and local statutes and regulations related to solid waste?	☐	☐	☐	☒

Discussion: The project will comply with all federal, state, and local solid waste regulations.

XVIII. MANDATORY FINDINGS OF SIGNIFICANCE

a.	Does the project have the potential to degrade the quality of the environment, substantially reduce the habitat of a fish or wildlife species, cause a fish or wildlife population to drop below self-sustaining levels, threaten to eliminate a plant or animal community, reduce the number or restrict the range of a rare or endangered plant or animal or eliminate important examples of the major periods of California history or prehistory?	☐	☐	☒	☐

Discussion: The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot. The site is routinely maintained and mowed, so impact to fish, wildlife, of plant habitat is less than significant.

	Potentially Significant Impact	Less Than Significant with Mitigation Incorporated	Less Than Significant Impact	No Impact
b. Does the project have impacts that are individually limited, but cumulatively considerable? ("Cumulatively considerable" means that the incremental effects of a project are considerable when viewed in connection with the effects of past projects, the effects of other current projects, and the effects of probable future projects)?	☐	☐	☐	☒

Discussion: The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot.

Therefore, the project will not have impacts that are individually limited, but cumulatively considerable.

c. Does the project have environmental effects which will cause substantial adverse effects on human beings, either directly or indirectly?	☐	☐	☐	☒

Discussion: The project consists of subdividing a 1.8-acre site into two parcels where Parcel 1 is .8 acre and Parcel 2 would be 2 acres. The land use and zoning designations for Parcel 1 would be changed from Business Park to Commercial light-industrial. Then a development plan is proposed to construct a 3,000 square foot auto repair building with associated parking lot.

Therefore, the project will not cause substantial adverse effects to human beings, either directly or indirectly.

Exhibit A

EARLIER ANALYSIS AND BACKGROUND MATERIALS.

Earlier analyses may be used where, pursuant to tiering, program EIR, or other CEQA process, one or more effects have been adequately analyzed in an earlier EIR or negative declaration. Section 15063 (c)(3)(D).

Earlier Documents Prepared and Utilized in this Analysis and Background / Explanatory Materials

Reference #	Document Title	Available for Review at:
1	City of Paso Robles General Plan	City of Paso Robles Community Development Department 1000 Spring Street Paso Robles, CA 93446
2	City of Paso Robles Zoning Code	Same as above
3	City of Paso Robles Environmental Impact Report for General Plan Update	Same as above
4	2005 Airport Land Use Plan	Same as above
5	City of Paso Robles Municipal Code	Same as above
6	City of Paso Robles Water Master Plan	Same as above
7	City of Paso Robles Urban Water Management Plan 2005	Same as above
8	City of Paso Robles Sewer Master Plan	Same as above
9	City of Paso Robles Housing Element	Same as above
10	City of Paso Robles Standard Conditions of Approval for New Development	Same as above
11	San Luis Obispo County Air Pollution Control District Guidelines for Impact Thresholds	APCD 3433 Roberto Court San Luis Obispo, CA 93401
12	San Luis Obispo County – Land Use Element	San Luis Obispo County Department of Planning County Government Center San Luis Obispo, CA 93408
13	USDA, Soils Conservation Service, Soil Survey of San Luis Obispo County, Paso Robles Area, 1983	Soil Conservation Offices Paso Robles, Ca 93446
14	Resolution 98-001, MND for Tract 2269	City of Paso Robles Community Development Department

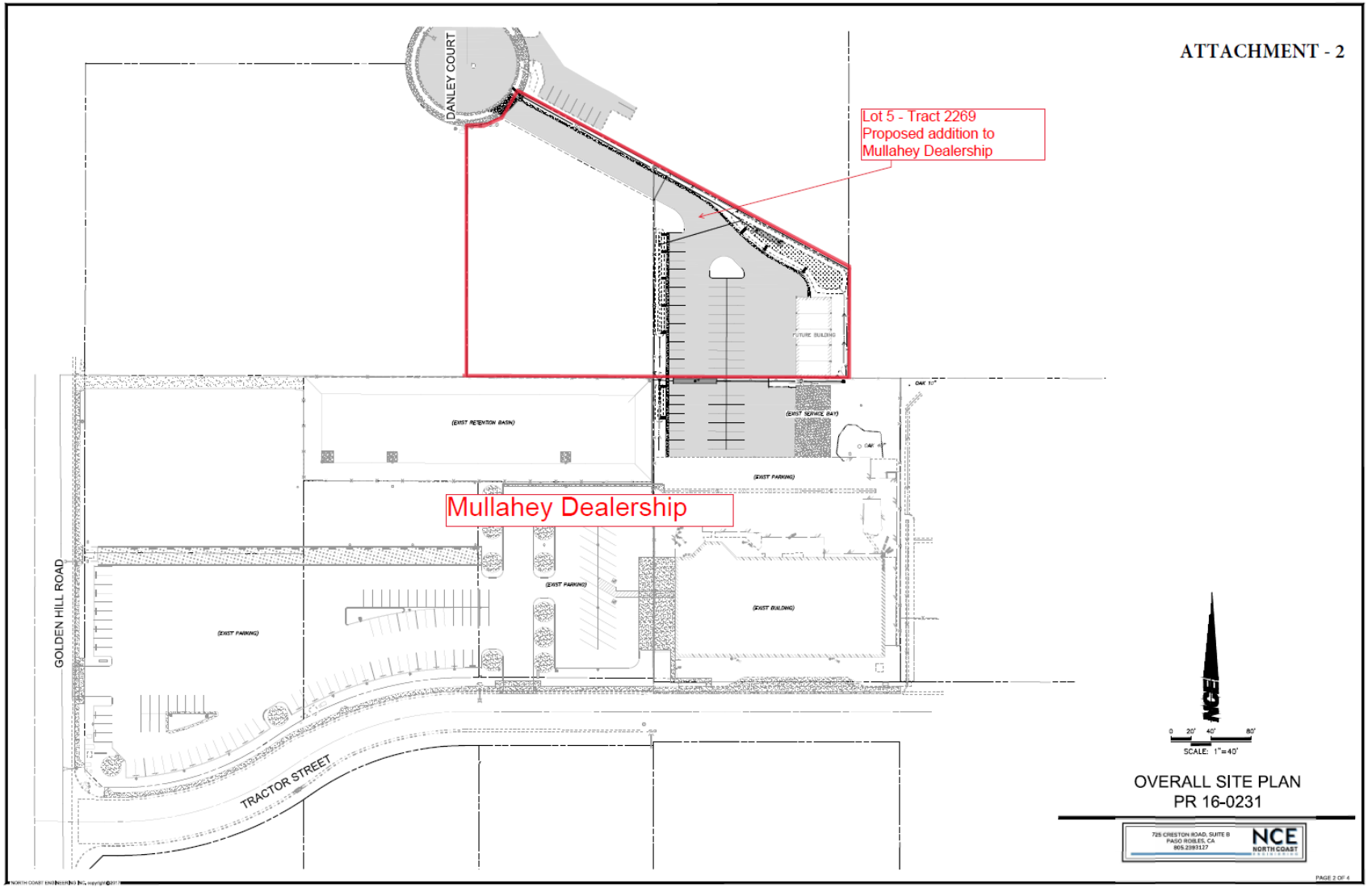
Exhibit A

Attachments:

1. Vicinity Map
2. Site Plan
3. Storm Water Quality Management Plan

Attachment - 1
Exhibit A

Exhibit A


ATTACHMENT - 2
DANLEY COURT
Lot 5 - Tract 2269
Proposed addition to
Mullahey Dealership
Mullahey Dealership
GOLDEN HILL ROAD
TRACTOR STREET
(EXIST RETENTION BASIN)
(EXIST SERVICE BAY)
(EXIST PARKING)
(EXIST BUILDING)
FUTURE BUILDING
OAK 10"
OAK 6"
NCE
0 20' 40' 80'
SCALE: 1"=40'
OVERALL SITE PLAN
PR 16-0231
725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.2393127
NCE
NORTH COAST
PAGE 2 OF 4

STORMWATER CONTROL PLAN AND STORM DRAIN REPORT

For

Mullahey Dodge

Paso Robles, CA

Michael Mullahey
330 Traffic Way
Arroyo Grande, CA 93420
(805) 481-3673

Prepared BY:

North Coast Engineering, Inc.
725 Creston Road, Suite B
Paso Robles, CA 93446

April 2017

Table of Contents

TABLES:

EXHIBITS :

I. Introduction

Project Name: Mullahey Dodge

Application Number: _____

Name of Applicant: Michael Mullahey

The Property

Location: The proposed commercial parking lot expansion is located on a 1.11 acre, 48,564 ft^2 lot north of Highway 46 between Golden Hill Road and Oakwood Street. (See Exhibit A – Vicinity and Location Maps)

Address: 2630 Danley Court
Paso Robles, CA 93446

Assessor's Parcel Numbers: 025-425-013 and 025-421-009

Existing property description: The southern lot of the two-lot project site is comprised of a decomposed granite parking lot and a concrete truck pad that was part of the existing Mullahey Dodge commercial project that was completed in 2015. The southern lot is relatively flat and generally slopes towards the existing detention basin with a design volume of 179,308 ft^3. A 12-foot emergency water passage was designed to allow overflow to discharge to Golden Hill Road. Included in the sizing of the existing basin was the 2015 southern lot improvements. The northern lot of the project site generally slopes to the northwest at 0.5% to 1.5% with site runoff discharging to Danley Court. The runoff travels north along Danley Court to Wisteria Lane to Golden Hill Road to a manmade channel at the end of Golden Hill Road then sheet flows to the Salinas River.

The Project

Project Type: Commercial, Uncovered parking lot and car service bays

Project Description: The proposed Mullahey Dodge project in located on two lots in Paso Robles, CA and involves the development of a 62 stall, HMA parking lot, an 81-foot x 35-foot car service building. Development will include a 24 – foot access drive from Danley Court to the project site and the associated frontage improvements at the cul-de-sac. Project development will also include the construction of one 77-foot and one 42-foot underground stormwater storage chamber system.

Area Breakdown

The total project site was designed with two different systems. One system was designed to handle the development to occur on the northern lot of the Mullahey Dodge site and one system was designed for the replacement of impervious material on the southern lot developed in 2015.

Northern Lot (new development)

Impervious Area, (HMA pavement and car service building):	27,142 s.f. = 0.62 acres
Landscape and Bioswale Areas:	10,081 s.f. = 0.23 acres

Southern Lot (existing development)

Impervious Area (HMA pavement):	10,310 s.f. = 0.24 acres
Landscape and Bioswale Areas:	1,030 s.f. = 0.02 acres
Total Project Area:	48,563 s.f. = 1.11 acres

Table 1 -Pervious/Impervious Area Summary

PRE-CONSTRUCTION

Condition	Pre-Construction Area (af)/(acres)	Post-Construction Area (sf)/(acres)
Impervious: (Building, Flatwork, Paving)(1)	10,153 / 0.23	37,340 / 0.86
Vegetation: (Lawns, Landscaping)	38,411 / 0.88	11,224 / 0.25
Total Area	**48,564 / 1.11**	**48,564 / 1.11**

Notes:

1) Impervious area includes replaced paving area

The Purpose

The purpose of this Stormwater Control Plan is to outline the site planning, Low Impact Development (LID) concepts, best management practices (BMP's) and Stormwater Control Measures (SCMs) that will be employed in the design and development of the private commercial parking lot expansion at 2630 Danley Court. This report will demonstrate that the Post-Construction Stormwater Management Requirements (PCRs) will be met, as outlined in the Central Coast Region Resolution No. R3-2013-0032 prepared by the California Regional Water Quality Control Board Central Coast Region. These requirements went into effect on March 6, 2014. The requirements, methodology of analysis and results will be outlined in the remainder of this report.

Compliance with these requirements maintains the hydrologic function of the site, promotes groundwater recharge and mitigates water quality impacts caused by the addition of impervious surfaces.

The peak flows and runoff volumes were calculated using the SCS methodology in the Autodesk Storm and Sanitary Sewer Program (SSA). The underground storage basins were analyzed using a conservative 0.5 in/hr infiltration rate. In November 2013, Earth Systems Pacific performed infiltration testing in 3 locations on the recently developed site to the south with tested infiltration rates ranging from 3.00 in/hr to > 10.00 in/hr. The results of the testing are presented in a report by Earth Systems Pacific, dated November 20, 2013; this report has been included for reference (See Exhibit I – Geotechnical Engineering Report).

II. Requirements

Post-Construction Stormwater Management Requirements (PCRs)

Site statistics:

The total new and/or replaced impervious surface area is **48,561 sf** (See Table 1: Pervious/Impervious Areas Summary).

The project site is in Water Management Zone 1 (See Exhibit B - Watershed Management Zones (WMZs)).

The project is in the Salinas Valley Ground Water Basin (See Exhibit C - Ground Water Basin).

Because the proposed redevelopment creates or replaces more than 2,500 s.f. of impervious surface the project is considered a Regulated Project and may be subject to the following Performance Requirements:

Table 2 - Performance Requirement Summary

Performance Requirement	Impervious Threshold	Applies:
No. 1 Site Design and Runoff Reduction	> 5,500 s.f.	Yes
No. 2 Water Quality Treatment	> 5,000 s.f.	Yes
No. 3 Runoff Retention	> 15,000 s.f.	Yes
No. 4 Peak Management	> 22,500 s.f.	Yes

Drainage Management Areas (DMAs)

Drainage Management Areas (DMAs) were delineated to support a decentralized approach to stormwater management. Points of Interest (POIs) were identified at critical discharge points or important stormwater drainage locations (see Exhibit E - Drainage Management Areas (DMAs) and Table 3: DMA Breakdown).

Drainage Management Areas (DMAs)

Drainage Management Areas (DMAs) were delineated to support the decentralized approach to storm water management. Points of interest (POIs) were identified at critical discharge points or important storm water drainage locations (see Exhibit E – Drainage Management Areas (DMAs) and Table 3: DMA Breakdown).

Table 3 – DMA Break Down

Condition	DMA-1	DMA-2	DMA-3	DMA-4	DMA-5	**Total**
	(ft^2)	(ft^2)	(ft^2)	(ft^2)	(ft^2)	**(ft^2)**
Impervious Surfaces	4,320	1,418	15,651	5,754	1,831.3	**37,340**
Vegetation	3,862	1,577	3,275	1,367	585.3	**11,224**
Total	**8,182**	**2,995**	**18,926**	**7,121**	**2,416.6**	**48,564**
AREA (AC)	0.19	0.07	0.43	0.16	0.06	1.11
CN	87.43	85.34	93.85	93.39	95.28	91.06

Retention Volumes Summary

The retention volumes of the two subsurface storage basins are summarized in Table 4. These volumes do not reflect additional storage due to infiltration. Additional storage due to infiltration is approximately 2,192 ft^3.

Table 4 – Retention Volume Summary

Basin	Physical Retention Storage (ft^3)
1	590
2	510
Total	**1,100**

III. Methodology

The onsite DMA boundaries and off-site watershed boundaries were determined using the grading from the project plans and topographic map of the existing surface, and by a visual inspection of the watershed. The existing ground cover was determined by a visual inspection of the watershed.

Existing and proposed on-site impervious areas were determined from the topographic map and from the project plans.

24-hour rainfall totals for the 85th and 95th percentile storms were determined from the City Standard Drawing 1010, Section 5.2, and the 2-year, 10-year and 100-year rainfall totals were determined from NOAA Atlas 14 (See Exhibit-E).

Hydrology was calculated using the SCS unit hydrograph method within the Autodesk Storm and Sanitary Sewer (SSA) computer program. The SSA program was also employed to route runoff through the proposed storm drain system and model detention and retention. Detailed Calculation are provided for reference as Exhibit H.

Method 2 (Routing Method) was used to determine that SCM capture volume, as prescribed in Attachment D of the PCRs.

An average infiltration rate of 0.5 in/hr was used in modeling the system. This was based on the infiltration test results provide in Exhibit I.

Performance Requirement No. 1
Site Design and Runoff Reduction

Since the project is a Regulated Project, it is subject to this performance requirement; the following components will be utilized to satisfy this requirement.

Site assessment summary:

The following site assessment measures were used to identify opportunities and constraints to implement LID Stormwater Control Measures. The site plan was developed and designed taking the following into account (See Exhibit H - Project Plans):

- Site topography
- Hydrologic features including contiguous natural areas
- Soil types and hydrologic soil groups
- Vegetative cover/trees
- Run-on characteristics (source and estimated runoff from offsite which discharge to the project area)
- Existing drainage infrastructure for the site and nearby areas including the location of municipal storm drains
- Utilities
- Easements
- Zoning/Land Use
- Setbacks
- Other pertinent overlay(s)

Site design measures used:

- Define development envelope and identify areas that are most suitable for development
- Construct streets and parking lot aisles to the minimum widths necessary, provided that public safety or mobility uses are not compromised
- Conform the site layout along natural topography to the maximum extent practicable
- Preserve native vegetation where practicable

Runoff Reduction Measures:

- Direct runoff from parking areas and circulation improvements safely onto vegetated areas and/or bio-retention areas to the maximum extent practicable
- Infiltrate runoff in subsurface storage area.

Performance Requirement No. 2
Water Quality Treatment

Projects that create and/or replace 5,000 square feet or more of impervious surface must treat stormwater runoff from existing, new, and replaced impervious surfaces on sites where runoff from existing impervious surfaces cannot be separated from runoff from new and replaced impervious surfaces. Water Quality Treatment must be treated onsite and designed to treat stormwater runoff equal to the volume of runoff generated by the 85th percentile 24-hour storm event, of 1 inch. Water Quality Treatment may implement a treatment system that use multiple systems to complete Water Quality Treatment.

- The Water Quality Treatment system must first implement Low Impact Development T reatment Systems
- Then may implement Bio-filtration Systems
- Then, finally may implement Non-Retention Based Treatment Systems.

Projects subject to Performance Requirement No. 2 must also include design strategies required by Performance Requirement No. 1.

1. Low Impact Development Treatment System

The project bio-filtration basin, underdrain system, and subsurface storage chambers have been designed to retain and infiltrate the entire 95th percentile storm to meet requirement 3, runoff retention. The 85th percentile volume requirement is 1,819 ft^3, which is less than the 95th percentile retention requirement of 2,923 ft^3. Therefore, the proposed system meets water quality treatment requirements. Elements from the following systems were employed in the project for an extra level of water quality treatment, but are not required to meet these requirements.

2. Bio-filtration treatment systems with the following design parameters:

Small localized bio-filtration ponds and swales have been incorporated into the design. The bio-filtration basins have an approximate retention volume of 1,930 ft^3 combined.

a. Minimum surface reservoir volume equal to the bio-filtration treatment system surface area times a depth of 6 inches.

The localized bio-filtration swales and ponds generally have a surface depth of 6", and the bio-filtration basin has a surface storage depth of 18".

b. Minimum planting medium depth of 24 inches.

The BSM is 24" deep in the bio-filtration basin.

The planting medium must sustain a minimum infiltration rate of 5 inches per hour throughout the life of the project and must maximize runoff retention and pollutant removal. A mixture of:

i. Sand - 60% to 70% meeting ASTM C33

ii. Compost - 30% to 40% may be used

This soil mixture has been specified in the project plans and is assumed to have a minimum infiltration rate of 5 in/hr.

c. Proper plant selection

Proper plans have been selected per the Landscape plans

d. No compaction of soils beneath the bio-filtration facility (ripping/loosening of soils required if compacted).

Provided in all facilities

e. No liners or other barriers interfering with infiltration, except for situations where lateral infiltration is not technically feasible.

The liner for the proposed underground storage chambers is intended to prevent migration of runoff towards the building foundation or up into the paving subgrade but doesn't impeded infiltration downward into the underlying soil.

2. Non-Retention Based Treatment Systems must collectively achieve at least one of the following hydraulic sizing criteria:

The proposed system is a retention based system, Hydraulic Design Criteria does not apply

a. Hydraulic Sizing Criteria for Non-Retention Based Treatment Systems:

i. Volume Hydraulic Design Basis must be based on the 85th percentile 24-hour storm event of 1 inch.

ii. Flow Hydraulic Design Basis must be based on the flow of runoff resulting from a rain event equal to at least 0.2 inches per hour intensity.

iii. See Performance Requirement No. 3: Runoff Retention for information to calculate the required water quality volume.

Requirement 2 Volume Calculations

ENTIRE PROJECT AREA	48,564	ft^2
RETENTION TRIBUTARY AREA	43,488	ft^2
POST CONSTRUCT IMPERVIOUS AREA (1)	37,340	ft^2
Post Developed i (2)	0.77	
Post-Developed C (3)	0.56	
85th Percentile Rainfall	0.89	in
85TH PERCENTILE RETENTION REQUIREMENT (4)	**1,819**	**ft^3**

(1) Includes AC paving, concrete flatwork, and buildings.

(2) i = Post Construction Impervious Area / Entire Project Area

(3) $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

(4) 85th Retention Requirement (ft^3) = C x 85th percentile rainfall (ft.) x Retention Tributary Area (ft^2)

Performance Requirement No. 3
Runoff Retention

Projects that create and/or replace 1 5,000 square feet or more of impervious surface must retain runoff for optimal management of watershed processes. Projects subject to Performance Requirement No. 3 must also include design strategies required by Performance Requirement No. 2 and 1.

1. Adjustments for Redevelopment – Replaced impervious surface, shall be multiplied by 0.5 when calculating the volume of runoff subject to Runoff Retention Performance Requirements.
2. Retention must meet the following performance requirements:
   a. Prevent offsite discharge from events up to the 95th percentile 24-hour rainfall event, 1.43 inches.
   b. Achieve retention by:
      i. optimizing soil infiltration

      Employed by installing BSM and gravel.

      ii. Storage

      Employed in surface and sub-surface retention

      iii. rainwater harvesting

      Not used

      iv. evapotranspiration

      Employed in vegetated bio-filtration swales, ponds and basins

The 95th percentile storage requirements were calculated in accordance with the procedures outlined in Appendix D of the PCRs. These calculations are included below.

Table 5– Retention Volume Summary

Basin	Physical Retention Storage (ft^3)	Total Retention Storage including Infiltration (ft^3)
1	1,100	3,030

Retention Analysis Summary
The retention system is comprised of two underground storage chambers, 6" and 8"pipes connecting the inlets to the storage chambers, the gravel bed at 40% porosity, the BSM layer at

40% porosity, and surface storage between 6" and 18" depth in the flat lower portion of the retention basin. The total storage volume of this system is 1,100 cubic feet. This volume does not reflect additional storage due to infiltration. Per the analysis results the retention basin infiltrated 2,370 ft^3 during the 95th percentile storm, draining the basin completely in less than 72 hours. Detailed SSA results are provided as Exhibit E.

Storage Volume with Infiltration = 3,030 ft^3 > 2,923 ft^3

Therefore, Performance Requirement No. 3 is satisfied.

Requirement 3 Volume Calculations

ENTIRE PROJECT AREA	48,564	ft^2
EXISTING IMPERVIOUS AREA	10,153	ft^2
REPLACED IMPERVIOUS AREA	10,153	ft^2
UNDISTURBED OR PLANTED AREAS (defined per B.4.d.iv.1)	0	ft^2
IMPERVIOUS SURFACE AREAS THAT DISCHARGE TO INFILTRATING AREAS (defined per B.4.d.iv.2)	0	ft^2
REDEVELOPMENT AREA ADJUSTMENT (per Appendix D 1.b)	5,077	ft^2
RETENTION TRIBUTARY AREA	43,488	ft^2
POST CONSTRUCT IMPERVIOUS AREA (1)	37,340	ft^2
95th Percentile Rainfall	1.43	in
Post Developed i (2)	0.77	
Post-Developed C (3)	0.56	
95TH PERCENTILE RETENTION REQUIREMENT (4)	**2,923**	**ft^3**

(1) Includes AC paving, concrete flatwork, and buildings.

(2) i = Post Construction Impervious Area / Entire Project Area

(3) $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

(4) 95th Retention Requirement (ft^3) = C x 95th percentile rainfall (ft.) x Retention Tributary Area (ft^2)

The proposed system provides 3,030 ft^3 of storage, exceeding the 2,923 ft^3 requirement , therefore Performance Requirement No. 3 is satisfied.

Performance Requirement No. 4
Peak Management

Since the project is a Regulated Project, is in WMZ 1 and creates greater than 22,500 Projects that create and/or replace 22,500 square feet or more of impervious surface must retain runoff for the optimal management of watershed processes. Projects subject to Performance Requirement No. 4 must also include design strategies required by Performance Requirement No. 3, 2 and 1. Post-development peak flows, discharged from the site, shall not exceed pre-project peak flows for the 2 through 10-year storm events.

The following rainfall totals were used in the analysis to model the 2-year and 10-year design storms in the SSA model:

- 2-Year 24-hour Rainfall Depth = **2.11 inches**
- 10-Year 24-hour Rainfall Depth = **3.68 inches**
  - 2-Year and 10-Year Rainfall depths were determined from NOAA Atlas 14 (See Exhibit D)

Runoff from the project is routed through the bio-filtration basin and storage chambers. The northern lot is discharged through 2' wide. 0.5' sidewalk underdrain to Danley Court and the southern lot is discharged into the exist detention pond located adjacent to the site.
The results of the SSA analysis of the 2- and 10- year pre-developed and post-developed design storms are summarized in Table 3. Detailed analysis results are provided in Exhibit G - Detailed SSA Results.

Table 6 – Detention Flow Summary (Northern Lot)

Design Storm	Pre-Developed (cfs)	Post-Developed (cfs)
2 Year	0.90	0.27
10 Year	1.89	0.76

Table 7 – Detention Flow Summary (Southern Lot)

Design Storm	Pre-Developed (cfs)	Post-Developed (cfs)
2 Year	0.34	0.16
10 Year	0.66	0.55

Performance requirement 4 is meet by reducing the 2-year and 10-year peak flows to a level lower than the pre-developed rate.

IV. Conclusions

The project incorporates the Runoff Reduction Measures and Structural Storm Water Control Measures (SCMs) described in this report.

- **Performance Requirement No. 1 is satisfied**
- **Performance Requirement No. 2 is satisfied**
- **Performance Requirement No. 3 is satisfied**
- **Performance Requirement No. 4 is satisfied**
- **City Drainage requirements are satisfied.**

The selection, sizing, and design of the Storm Water Control Measures (SCMs) meet all of the applicable Water Quality Treatment, Runoff Retention and Peak Management Performance Requirements

Statement of Compliance

There is no documentation needed to demonstrate infeasibility where on-site compliance cannot be achieved because it doesn't apply because all RWQCB were met.

The Water Quality Treatment, Runoff Retention and Peak Management Performance Requirements will be met on-site.

Operations and Maintenance (O&M) Plan

The following inspection schedule and operation and maintenance plan must be followed to ensure the long-term operation of the proposed structural control measures (SCMs). The owner of the facilities must provide the means to finance operation and maintenance of the facilities in perpetuity.

Inspection and maintenance items include:

- Bio-infiltration basin surface and plantings.
- Sub-surface storage chambers
- Drain Inlets
- Drain Inlet Filters

Routine Maintenance to be performed on a continuous basis.

- Remove leaves and debris from drain inlet grates and inlet filters.
- Remove trash and debris from the bio-filtration basin.
- Observe health of biofiltration plants and apply fertilizer and adjust irrigation as required.
- Remove and replace dead landscape plantings from bio-filtration basin.
- Sweep parking lots and remove trash and debris.

Annual Inspection and Maintenance to be performed before the rainy season.

- Inspect drain inlet catch basins for debris and sedimentation. Remove debris and trash. Remove siltation if greater that 2" deep.
- Inspect inlet filters and remove silt and debris. Replace filter medium pouch annually.
- Inspect silt traps and remove trash and debris. If silt is greater than 2" deep the silt trap shall be jetted with water and vacuumed.
- Inspect the silt trap sorbent pads, remove when pads are a uniform dark color per manufacturer's recommendations. The sorbent pads shall be replaced at a maximum 5-year interval regardless of the pad color.
- Inspect the underdrain system with a fiber optics camera and determine sediment depth using a stadia rod. Remove silt by water jetting and vacuuming when silt exceeds 1" in depth.

Inspect underground storage chambers with fiber optics camera and determine sediment depth by stadia rod. Remove silt by jetting with water and vacuuming, when silt depth exceeds 2".

Owner of facilities

The owner of the facilities is: **Michael Mullahey**

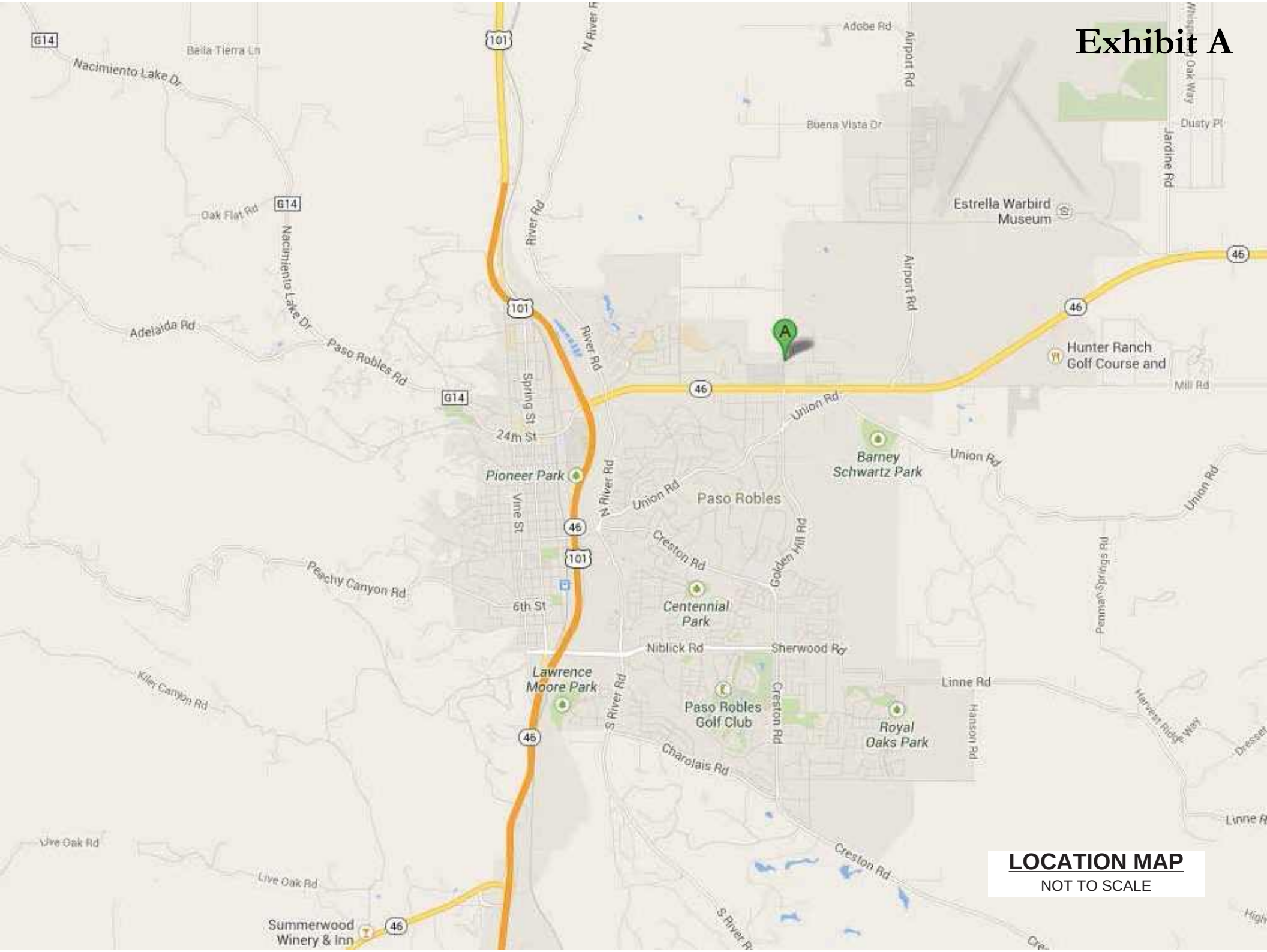
Exhibit A: Vicinity Map and Location Map

Exhibit A

VICINITY MAP

N.T.S.

Exhibit A



LOCATION MAP

NOT TO SCALE

Exhibit B: Watershed Management Zone (WMZs)

Exhibit A

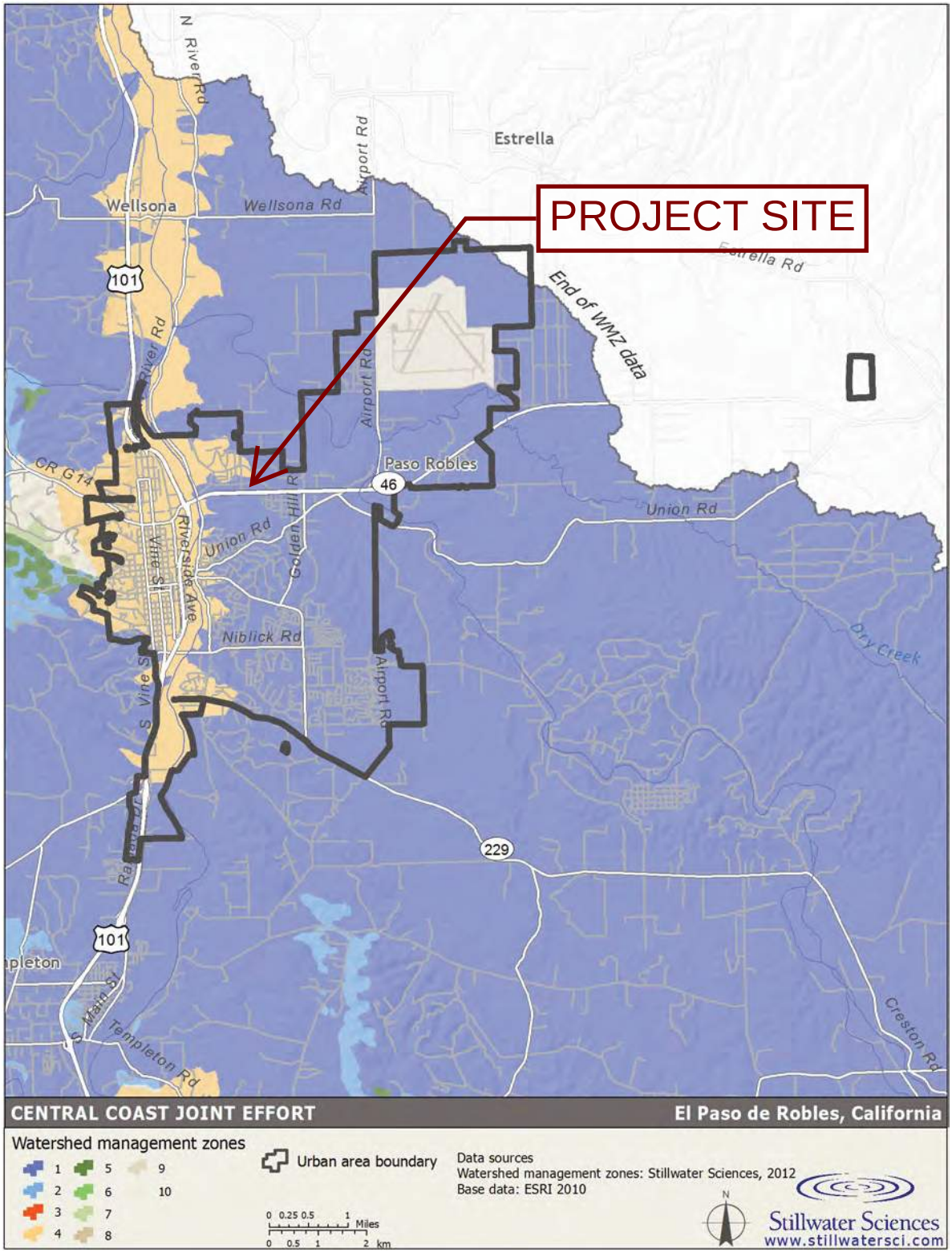
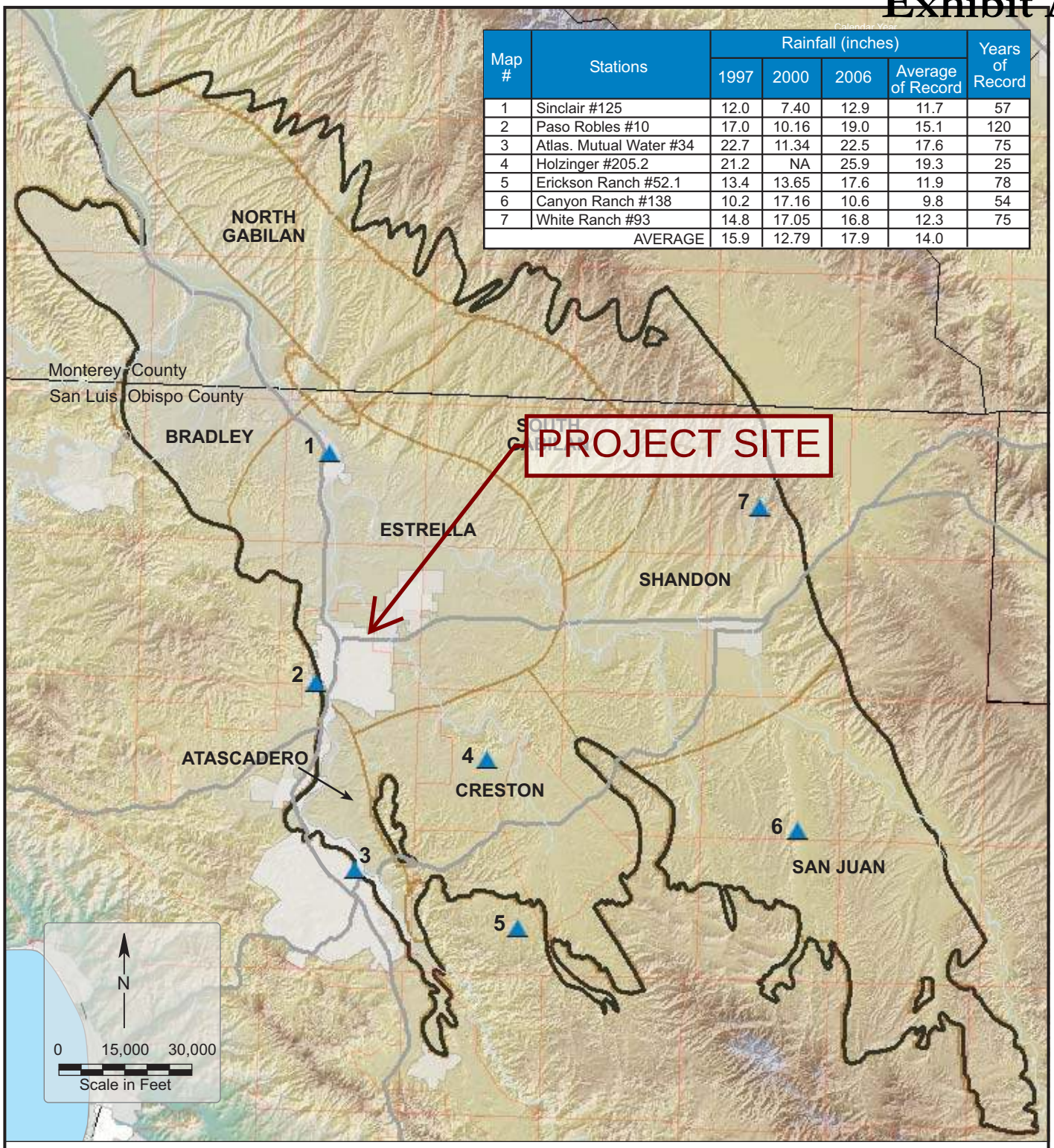

PROJECT SITE
Estrella
Wellsona
Wellsona Rd
N River Rd
Airport Rd
101
River Rd
End of WMZ data
Estrella Rd
CR G14
Paso Robles
46
Union Rd
Golden Hill Rd
Riverside Ave
S Vine St
Niblick Rd
Dry Creek
229
Templeton
Main St
Templeton Rd
Creston Rd
CENTRAL COAST JOINT EFFORT
El Paso de Robles, California
Watershed management zones
1
2
3
4
5
6
7
8
9
10
Urban area boundary
0 0.25 0.5 1 Miles
0 0.5 1 2 km
Data sources
Watershed management zones: Stillwater Sciences, 2012
Base data: ESRI 2010
N
Stillwater Sciences
www.stillwatersci.com

Exhibit C: Groundwater Basin

Exhibit A

Map #	Stations	Rainfall (inches)				Years of Record
		1997	2000	2006	Average of Record	
1	Sinclair #125	12.0	7.40	12.9	11.7	57
2	Paso Robles #10	17.0	10.16	19.0	15.1	120
3	Atlas. Mutual Water #34	22.7	11.34	22.5	17.6	75
4	Holzinger #205.2	21.2	NA	25.9	19.3	25
5	Erickson Ranch #52.1	13.4	13.65	17.6	11.9	78
6	Canyon Ranch #138	10.2	17.16	10.6	9.8	54
7	White Ranch #93	14.8	17.05	16.8	12.3	75
	AVERAGE	15.9	12.79	17.9	14.0	



Legend

- Rainfall Station
- Streams
- State Highways
- Township and Range Grid
- Basin Boundary
- Cities/Communities
- Subareas
- County Line

May 2009

TODD ENGINEERS
Alameda, California

Figure 1
Paso Robles Groundwater Basin

Exhibit D: 85th & 95th Percentile Rainfall

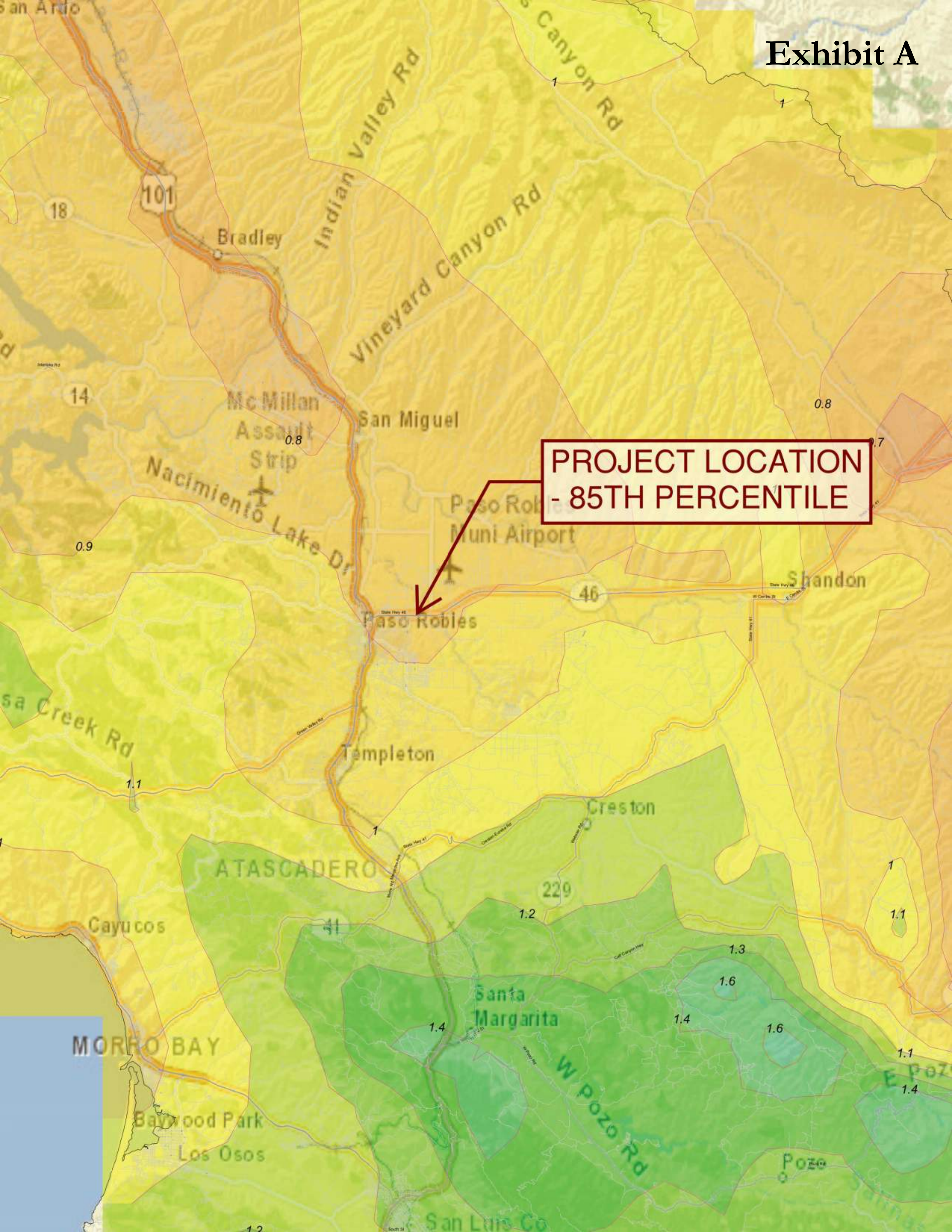


Exhibit A
PROJECT LOCATION
- 85TH PERCENTILE
Paso Robles
Paso Robles
Muni Airport
San Miguel
Bradley
Shandon
Templeton
Creston
ATASCADERO
Santa
Margarita
MORRO BAY
Cayucos
Baywood Park
Los Osos
Pozo
McMillan
Assault
Strip
Indian Valley Rd
Vineyard Canyon Rd
Nacimiento Lake Dr
W Pozo Rd
101
46
41
229
18
14
0.8
0.9
1
1.1
1.2
1.3
1.4
1.6

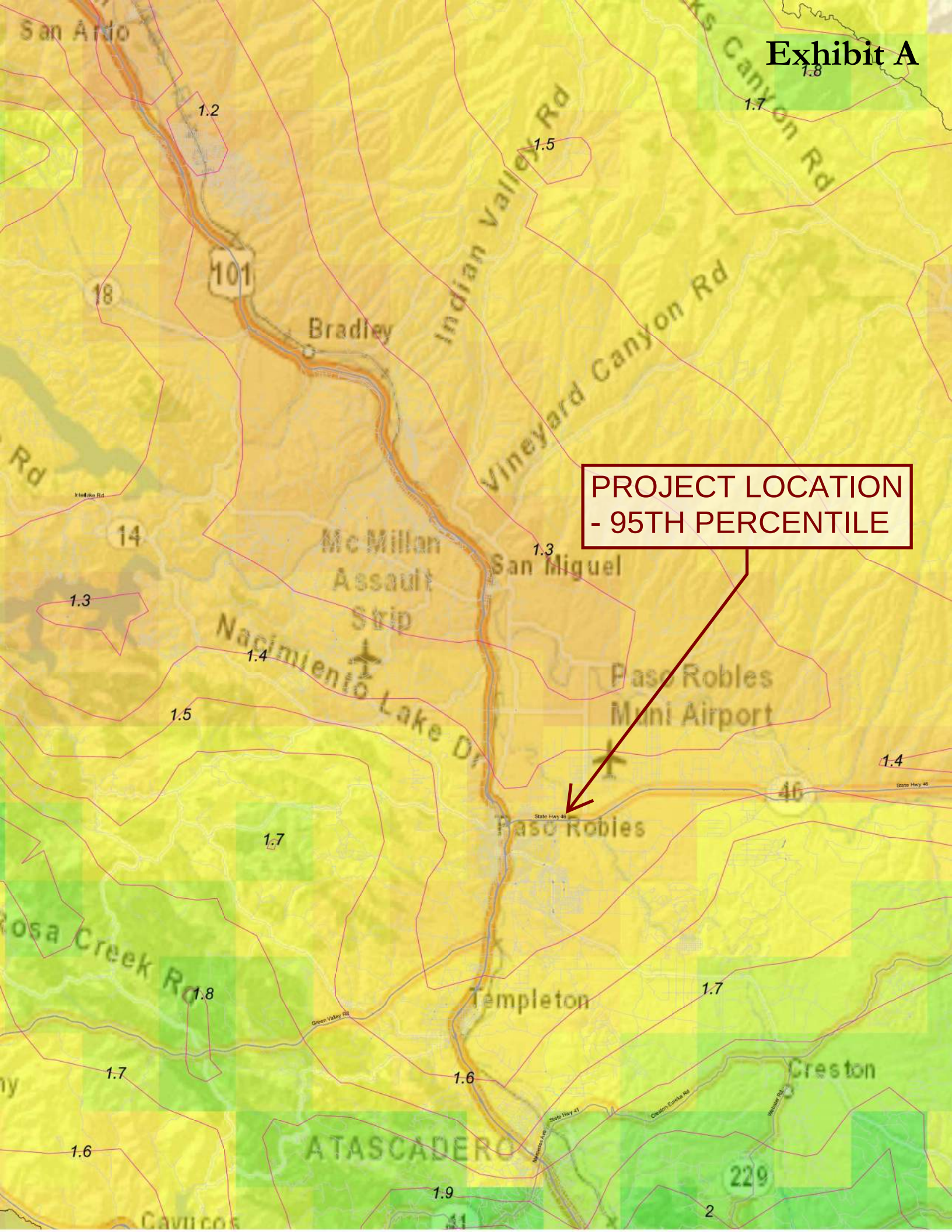
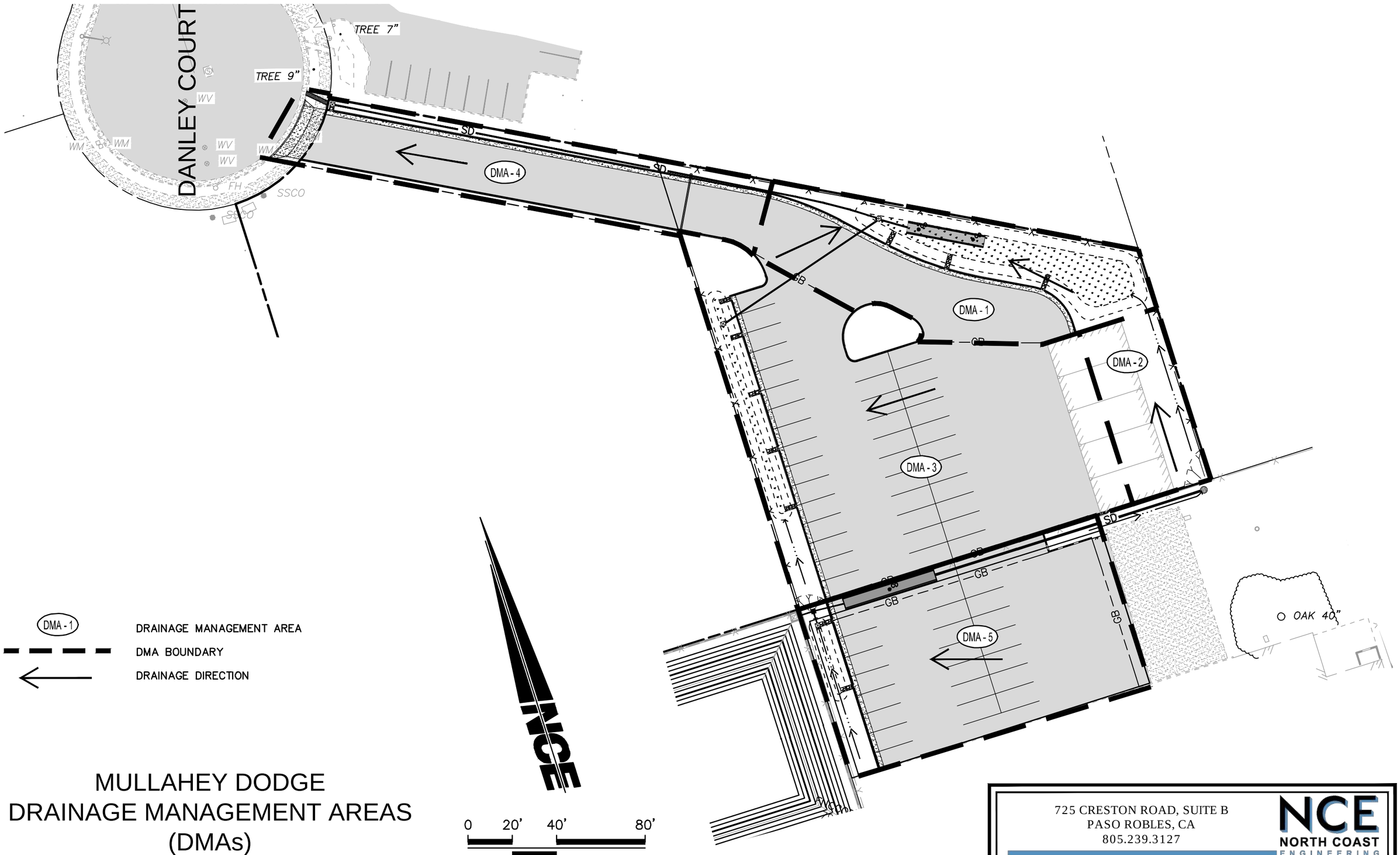


Exhibit A
PROJECT LOCATION
- 95TH PERCENTILE
San Ardo
Bradley
Indian Valley Rd
Vineyard Canyon Rd
McMillan
Assault
Strip
San Miguel
Nacimiento Lake Dr
Paso Robles
Muni Airport
Paso Robles
Templeton
Creston
ATASCADERO
Cayucos
101
46
229
1.2
1.5
1.7
1.8
1.3
1.4
1.6
1.9
2

Exhibit E: Drainage Management Areas (DMAs)

Exhibit A



DMA - 1 — DRAINAGE MANAGEMENT AREA

DMA BOUNDARY

DRAINAGE DIRECTION

NCE

0 20' 40' 80'

SCALE: 1"=40'

MULLAHEY DODGE
DRAINAGE MANAGEMENT AREAS (DMAs)

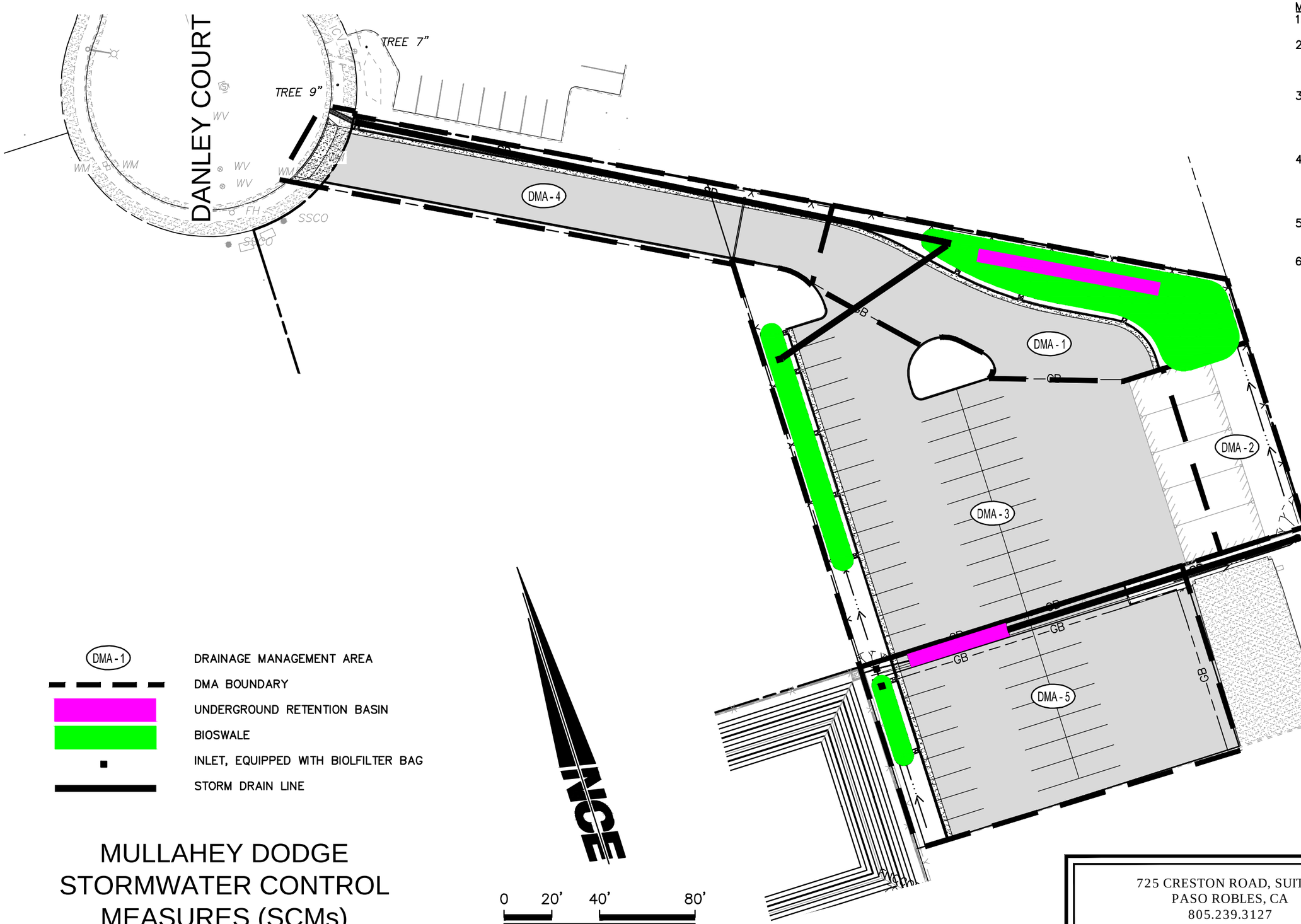
725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.239.3127

NCE NORTH COAST ENGINEERING

R:\PROJ\00136\Design\Working\00136 SWCP Quantities.dwg, DMAS, 4/5/2017 1:22:12 PM, EXL, 1:1

Exhibit F: Stormwater Control Measures (SCMs)

Exhibit A



MAINTENANCE

1. INSPECT INLETS BEFORE AND AFTER EACH RAINFALL
2. CHECK FOR AND REMOVE ANY ACCUMULATED SEDIMENT THAT MAY HAVE ENTERED INLETS
3. CHECK FOR AND REMOVE ACCUMULATED DEBRIS AND SEDIMENT THAT MAY BE PRESENT IN BIO-RETENTION SWALES
4. INSPECT FOR SCOUR AND RIP RAP DISPLACEMENT AROUND STORMDRAIN OUTLETS. REPAIR OR REPLACE AS NEEDED
5. INSPECT BIO-RETENTION SWALES FOR PROPER VEGETATION
6. INSPECT BIO-RETENTION SOIL MEDIA AND REPLACE EVERY FIVE YEARS

DMA - 1 — DRAINAGE MANAGEMENT AREA

DMA BOUNDARY

UNDERGROUND RETENTION BASIN

BIOSWALE

INLET, EQUIPPED WITH BIOLFILTER BAG

STORM DRAIN LINE

NCE

0 20' 40' 80'

SCALE: 1"=40'

MULLAHEY DODGE STORMWATER CONTROL MEASURES (SCMs)

725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.239.3127

NCE NORTH COAST ENGINEERING

R:\PROJ\00136\Design\Working\00136 SWCP Quantities.dwg, SCMS, 4/5/2017 1:27:13 PM, EXL, 1:1

Exhibit G: Detailed SSA Results

Exhibit A

Project Description

File Name ..... 2017-02-21_00136 SWCP.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	SCS TR-55
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	Jan 01, 2020	00:00:00
End Analysis On	Jan 04, 2020	00:00:00
Start Reporting On	Jan 01, 2020	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	1	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	5
Nodes	7
Junctions	2
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	3
Links	5
Channels	0
Pipes	3
Pumps	0
Orifices	1
Weirs	1
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	95TH	Cumulative	inches	California	San Luis Obispo (Atascadero)	2	1.43	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	DMA-1	0.19	87.43	1.43	0.51	0.09	0.07	0 00:04:25
2	DMA-2	0.07	85.34	1.43	0.40	0.03	0.02	0 00:04:08
3	DMA-3	0.43	93.85	1.43	0.86	0.37	0.29	0 00:05:46
4	DMA-4	0.16	93.39	1.43	0.83	0.14	0.11	0 00:06:16
5	PRE-SUB	0.85	91.43	1.43	0.71	0.60	0.50	0 00:02:18

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	Junction	789.15	794.90	789.15	0.00	50.00	0.24	793.45	0.00	1.45	0 00:00	0.00	0.00
2	Jun-04	Junction	788.67	794.60	788.67	0.00	0.00	0.32	793.45	0.00	1.15	0 00:00	0.00	0.00
3	OUT-POST	Outfall	793.30					0.03	793.30					
4	OUT-PRE	Outfall	0.00					0.48	0.00					
5	DI-4*	Storage Node	790.60	794.10	790.60		0.00	0.10	793.45				0.00	0.00
6	SUB-1	Storage Node	788.10	795.10	788.10		0.00	0.33	793.45				0.00	0.00
7	SURF-WST	Storage Node	793.90	794.90	793.90		0.00	0.29	794.45				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	Link-11	Pipe	Jun-04	DI-4*	245.00	789.90	790.60	-0.2900	8.000	0.0150	0.09	0.56	0.17	1.26	0.67	1.00	3209.00	SURCHARGED
2	Link-16	Pipe	DI-2	Jun-04	85.00	789.15	788.67	0.5600	8.000	0.0150	0.23	0.79	0.29	0.92	0.67	1.00	3713.00	SURCHARGED
3	Link-17	Pipe	SUB-1	Jun-04	13.00	788.60	788.67	-0.5400	8.000	0.0150	0.25	0.77	0.32	0.91	0.67	1.00	3717.00	SURCHARGED
4	Orfice-01	Orifice	SURF-WST	DI-2		793.90	789.15		18.000		0.24							
5	UNDR	Weir	DI-4*	OUT-POST		790.60	793.30				0.03							

Exhibit A

Subbasin Hydrology

Subbasin : DMA-1

Input Data

Area (ac)	0.19
Weighted Curve Number	87.43
Rain Gage ID	NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.11	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.19		87.43

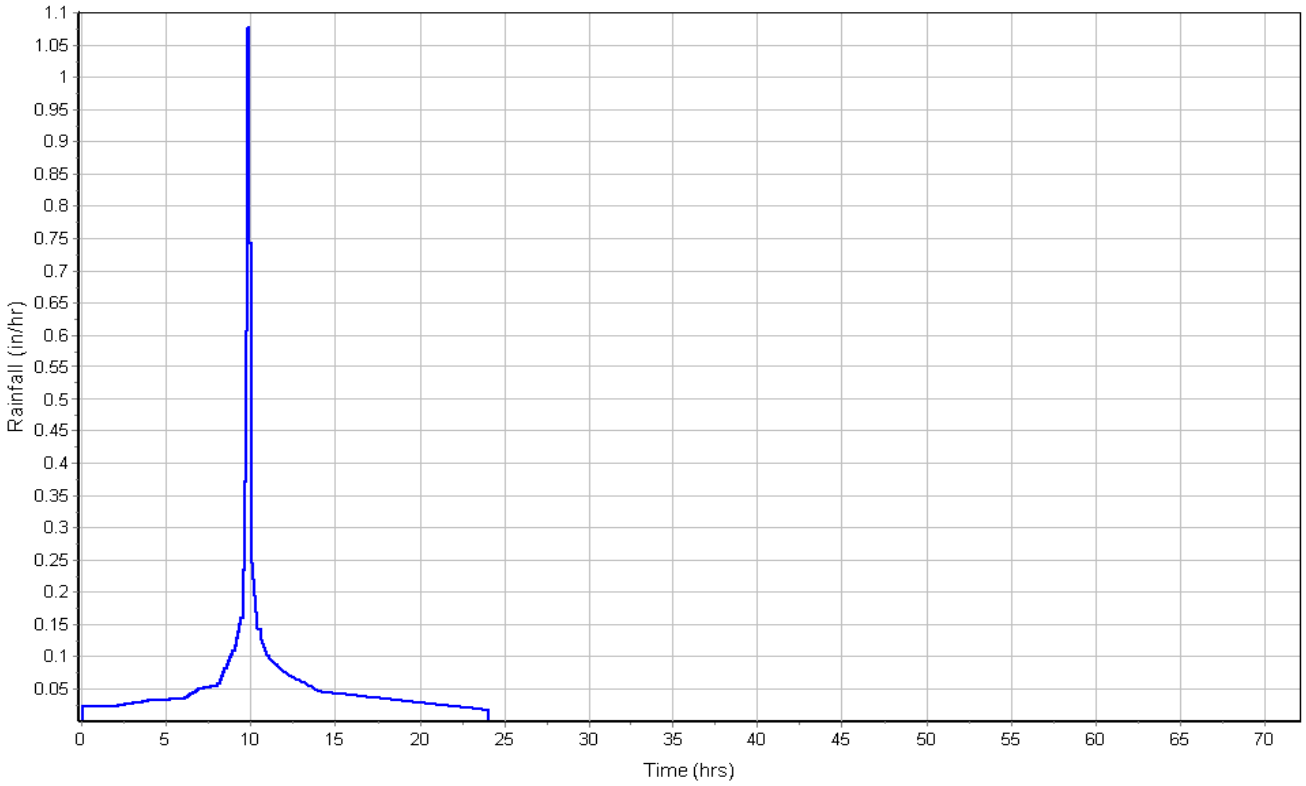
Subbasin Runoff Results

Total Rainfall (in)	1.43
Total Runoff (in)	0.51
Peak Runoff (cfs)	0.07
Weighted Curve Number	87.43
Time of Concentration (days hh:mm:ss)	0 00:04:26

Exhibit A

Subbasin : DMA-1

Rainfall Intensity Graph



Runoff Hydrograph

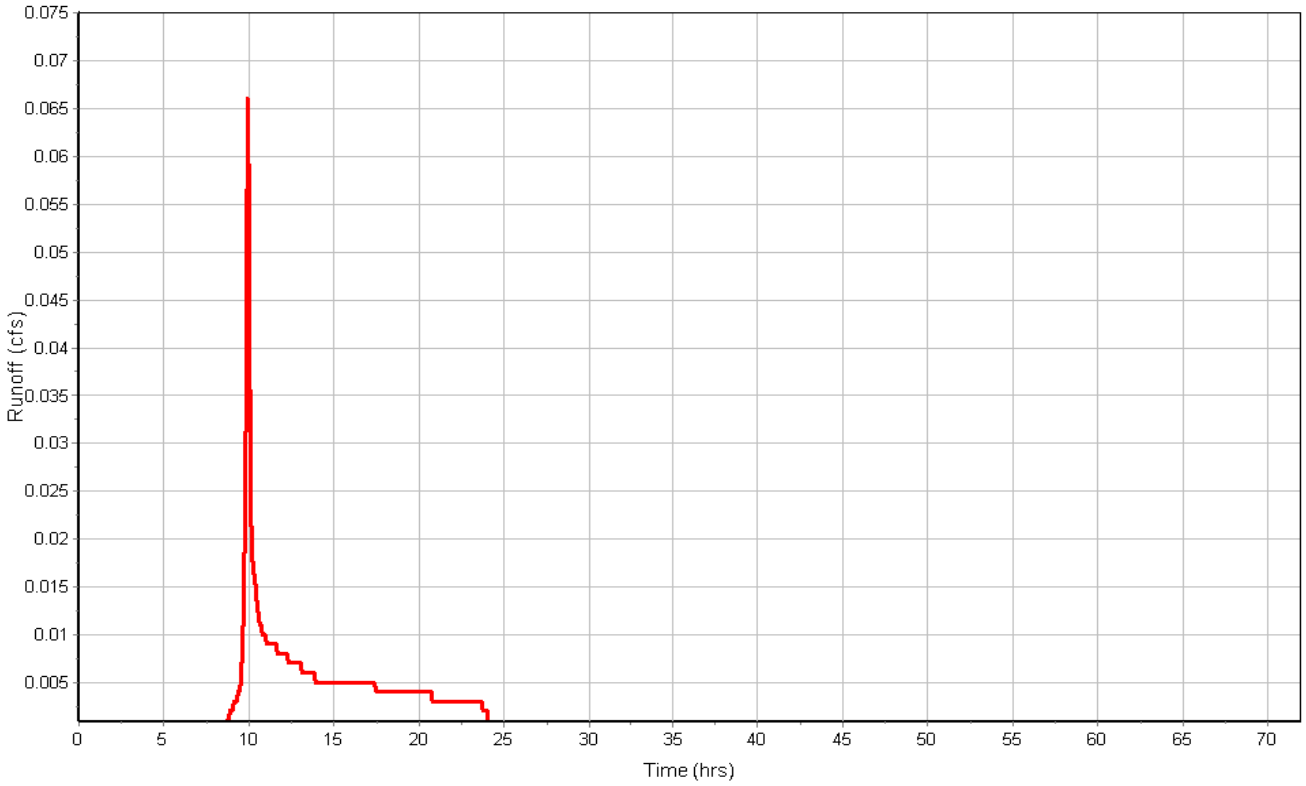


Exhibit A

Subbasin : DMA-2

Input Data

Area (ac) 0.07
Weighted Curve Number 85.34
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	98.00
-	0.04	-	74.00
Composite Area & Weighted CN	0.07		85.34

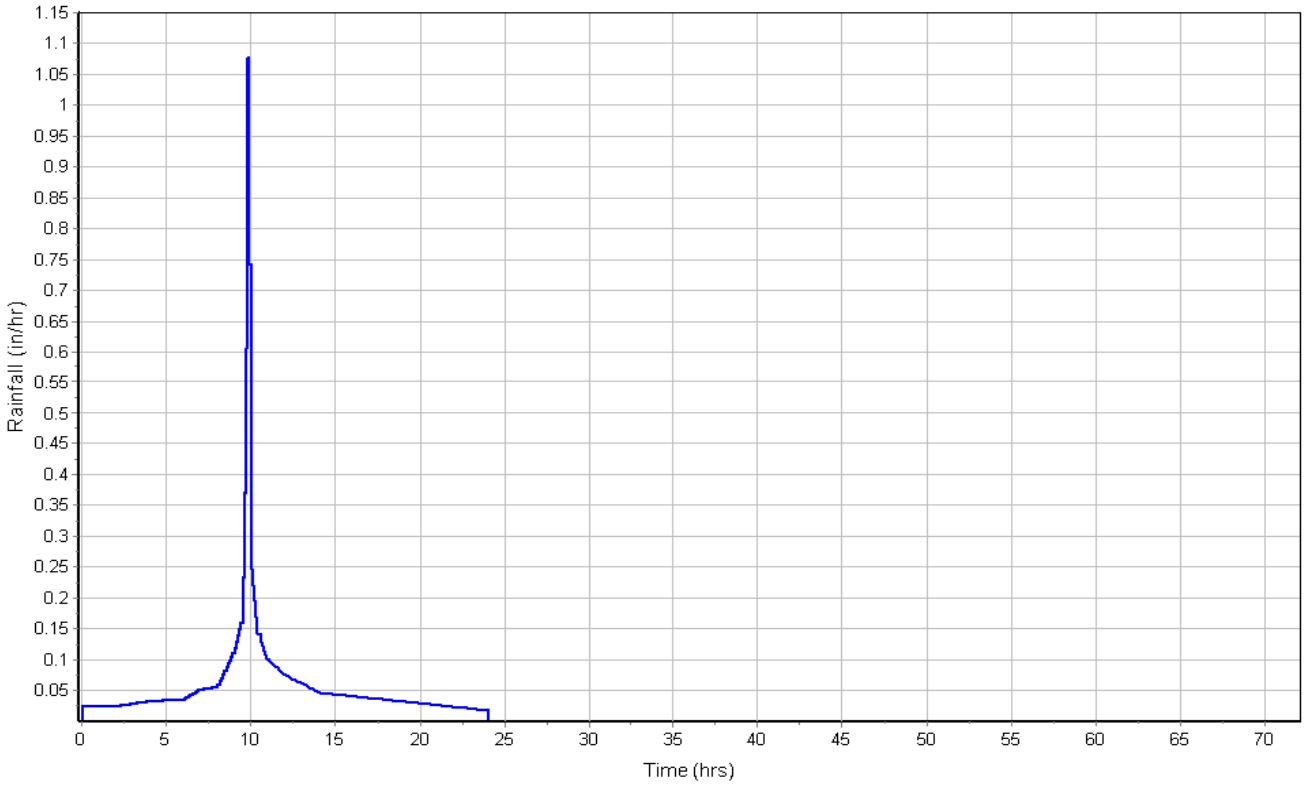
Subbasin Runoff Results

Total Rainfall (in) 1.43
Total Runoff (in) 0.40
Peak Runoff (cfs) 0.02
Weighted Curve Number 85.34
Time of Concentration (days hh:mm:ss) 0 00:04:08

Exhibit A

Subbasin : DMA-2

Rainfall Intensity Graph



Runoff Hydrograph

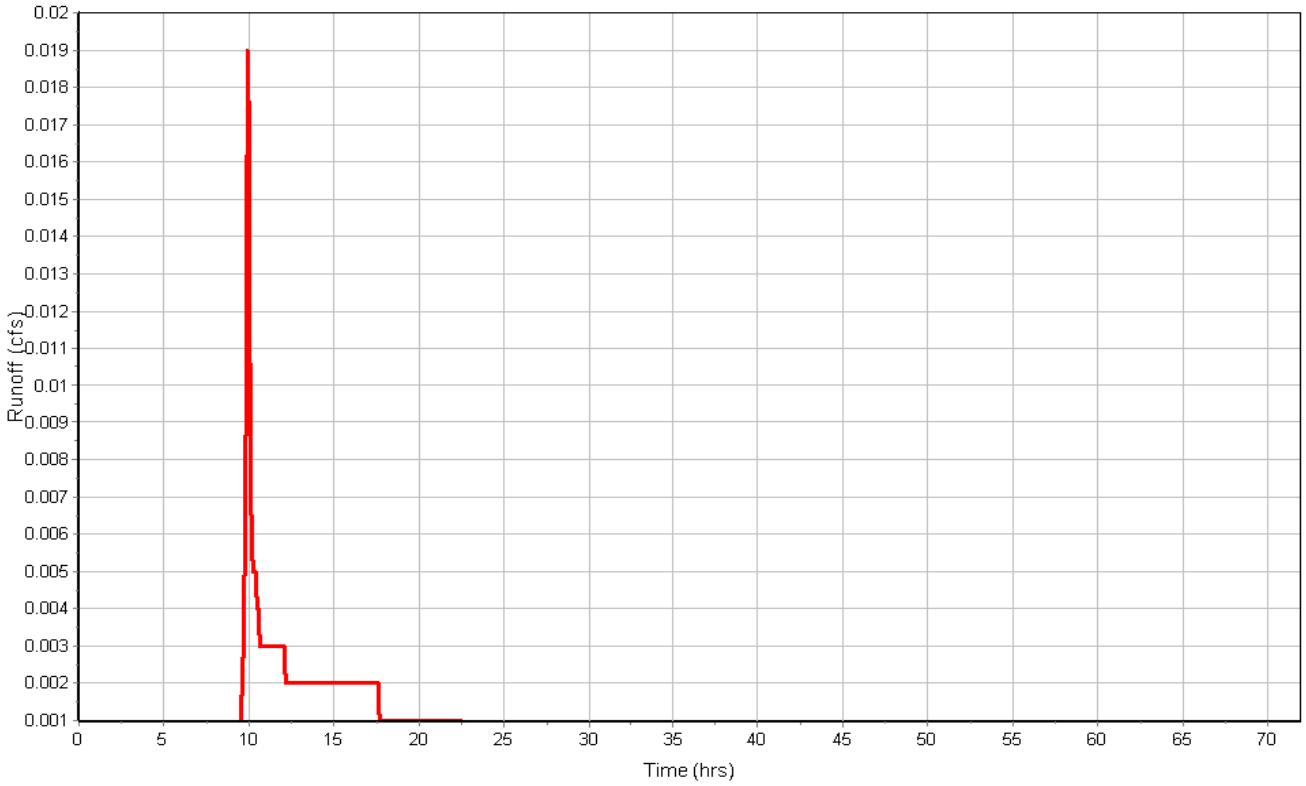


Exhibit A

Subbasin : DMA-3

Input Data

Area (ac) 0.43
Weighted Curve Number 93.85
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.36	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.44		93.85

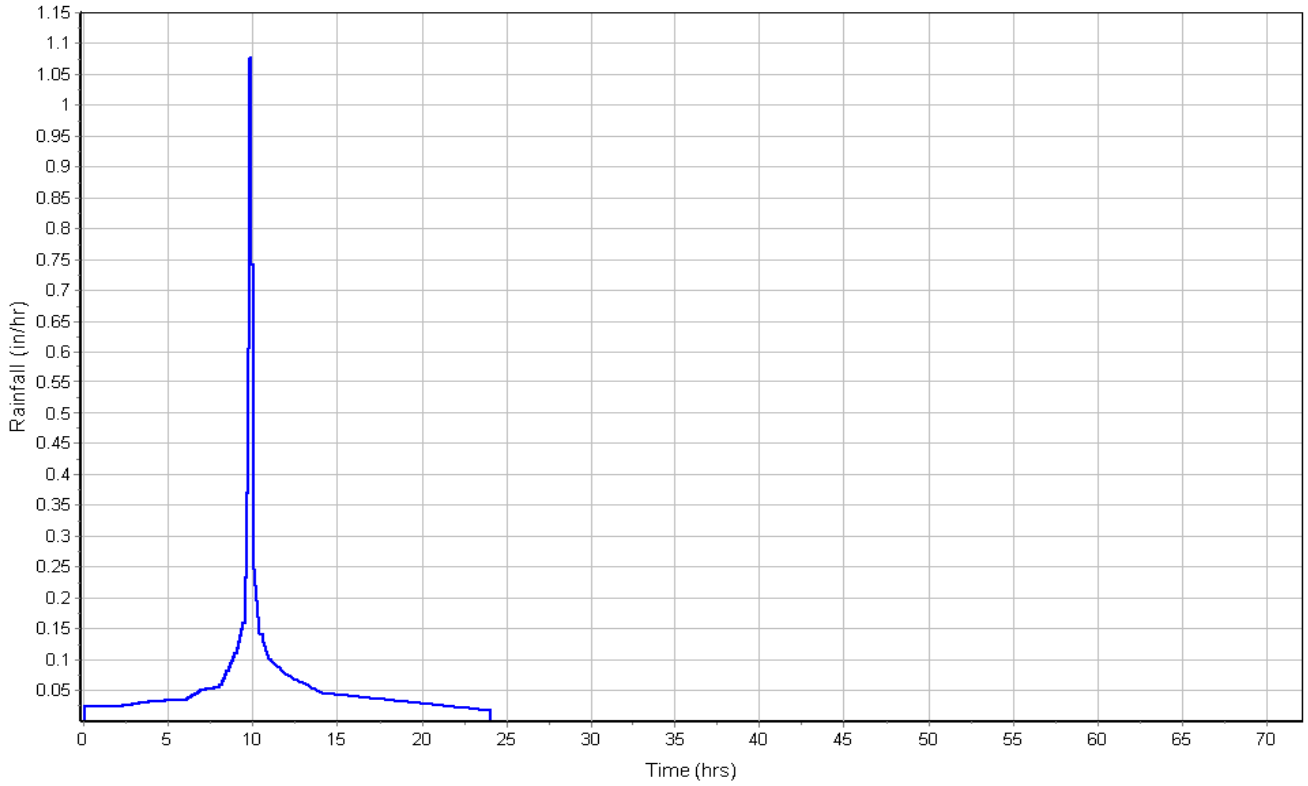
Subbasin Runoff Results

Total Rainfall (in) 1.43
Total Runoff (in) 0.86
Peak Runoff (cfs) 0.29
Weighted Curve Number 93.85
Time of Concentration (days hh:mm:ss) 0 00:05:46

Exhibit A

Subbasin : DMA-3

Rainfall Intensity Graph



Runoff Hydrograph

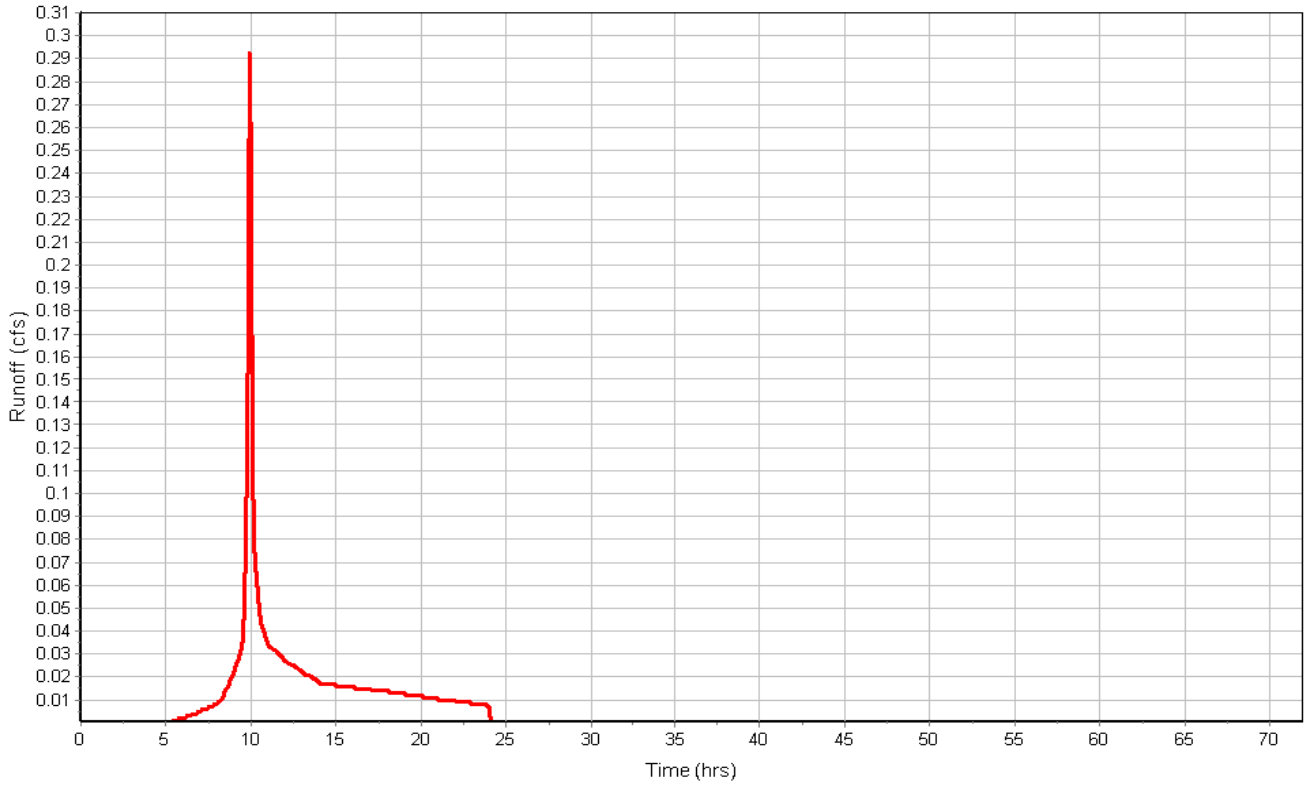


Exhibit A

Subbasin : DMA-4

Input Data

Area (ac)	0.16
Weighted Curve Number	93.39
Rain Gage ID	NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.13	-	98.00
-	0.03	-	74.00
Composite Area & Weighted CN	0.16		93.39

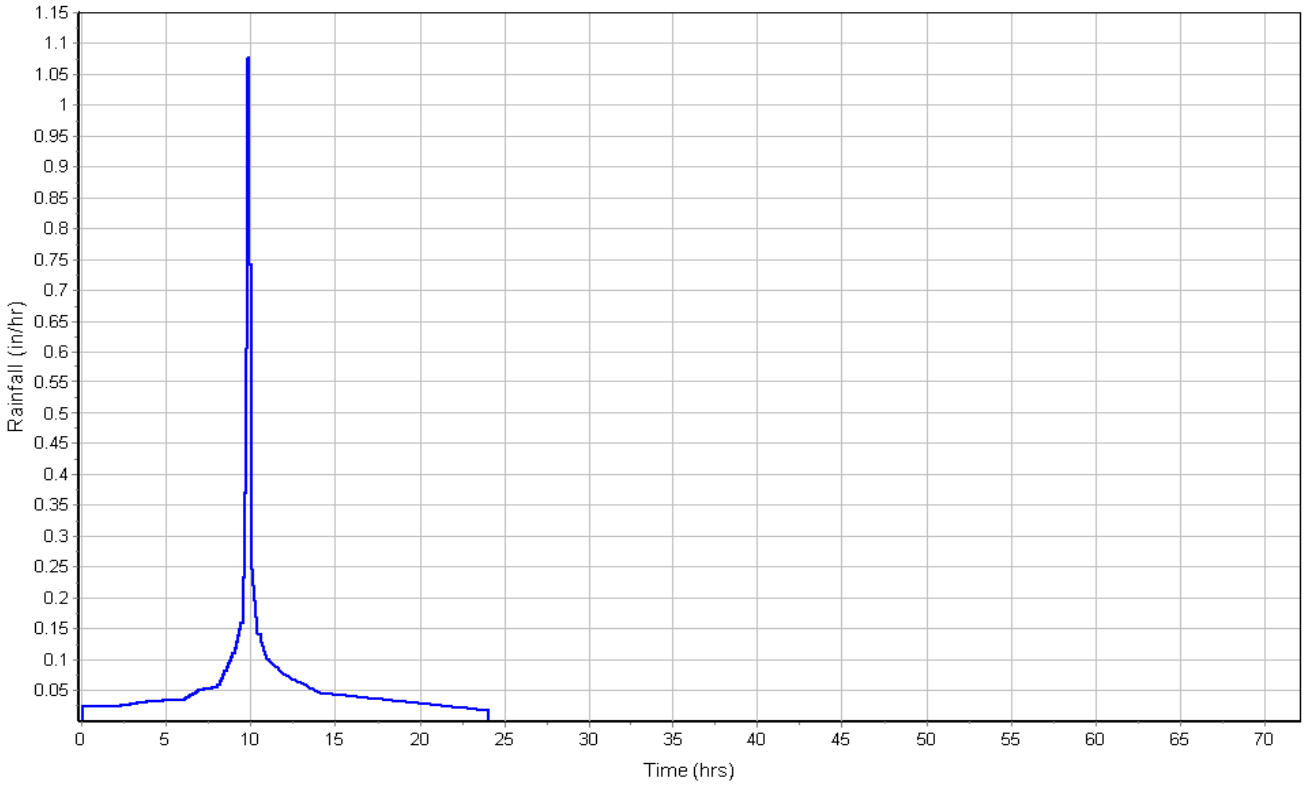
Subbasin Runoff Results

Total Rainfall (in)	1.43
Total Runoff (in)	0.83
Peak Runoff (cfs)	0.11
Weighted Curve Number	93.39
Time of Concentration (days hh:mm:ss)	0 00:06:17

Exhibit A

Subbasin : DMA-4

Rainfall Intensity Graph



Runoff Hydrograph

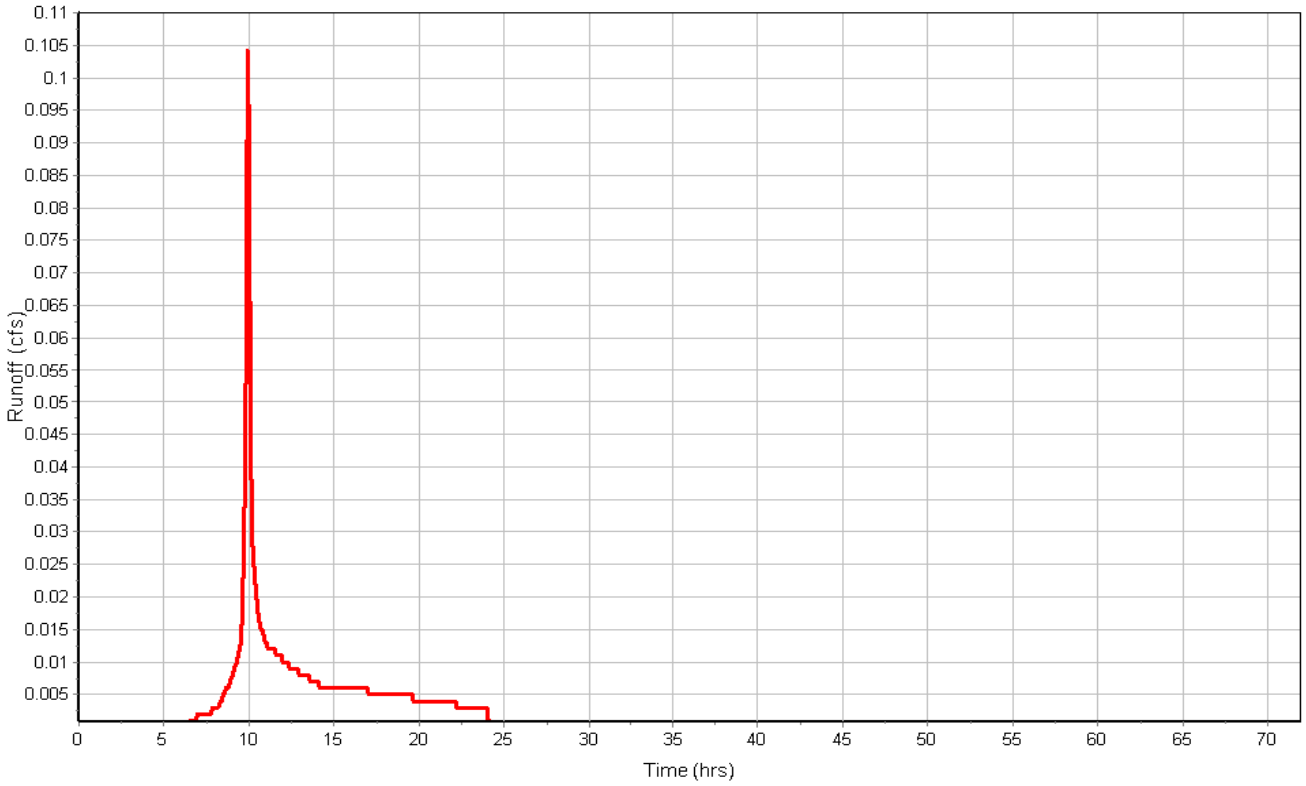


Exhibit A

Subbasin : PRE-SUB

Input Data

Area (ac)	0.85
Weighted Curve Number	91.43
Rain Gage ID	NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.62	-	98.00
-	0.23	-	74.00
Composite Area & Weighted CN	0.85		91.43

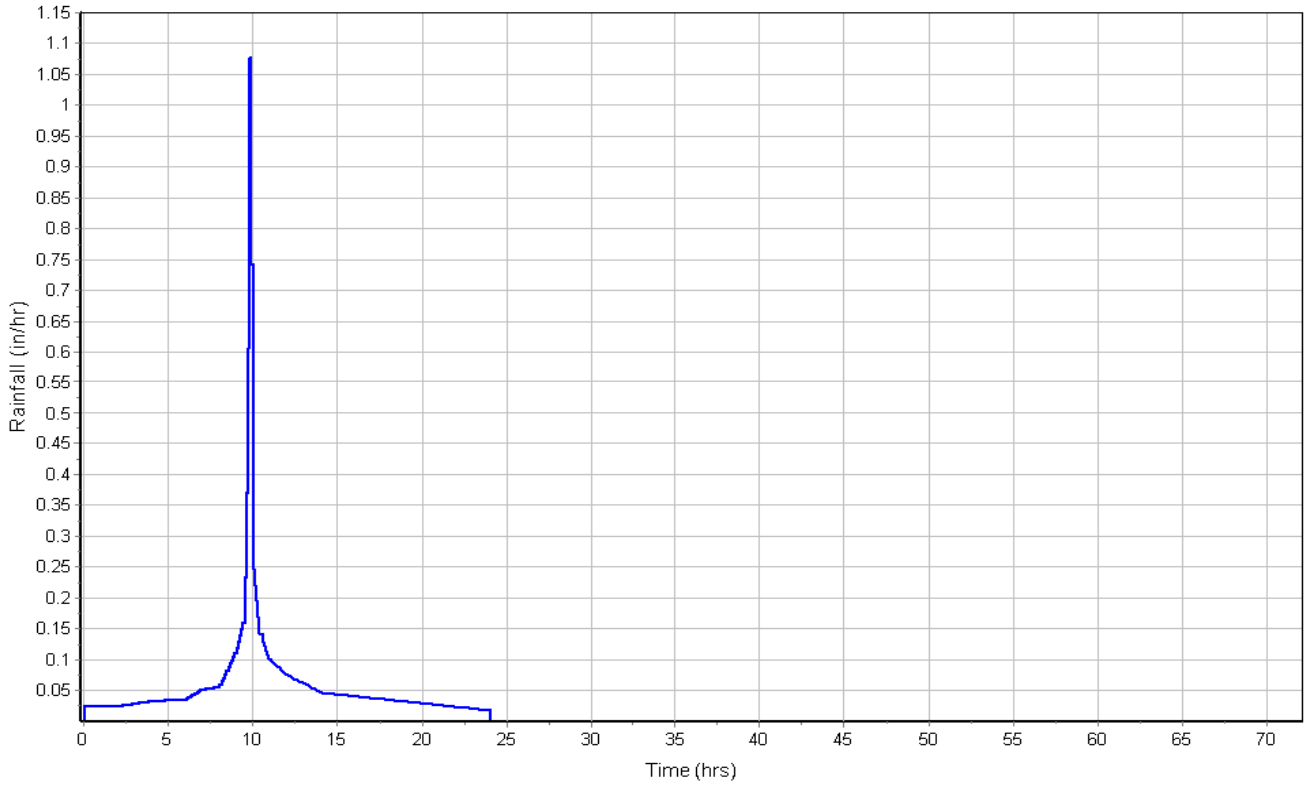
Subbasin Runoff Results

Total Rainfall (in)	1.43
Total Runoff (in)	0.71
Peak Runoff (cfs)	0.50
Weighted Curve Number	91.43
Time of Concentration (days hh:mm:ss)	0 00:02:18

Exhibit A

Subbasin : PRE-SUB

Rainfall Intensity Graph



Runoff Hydrograph

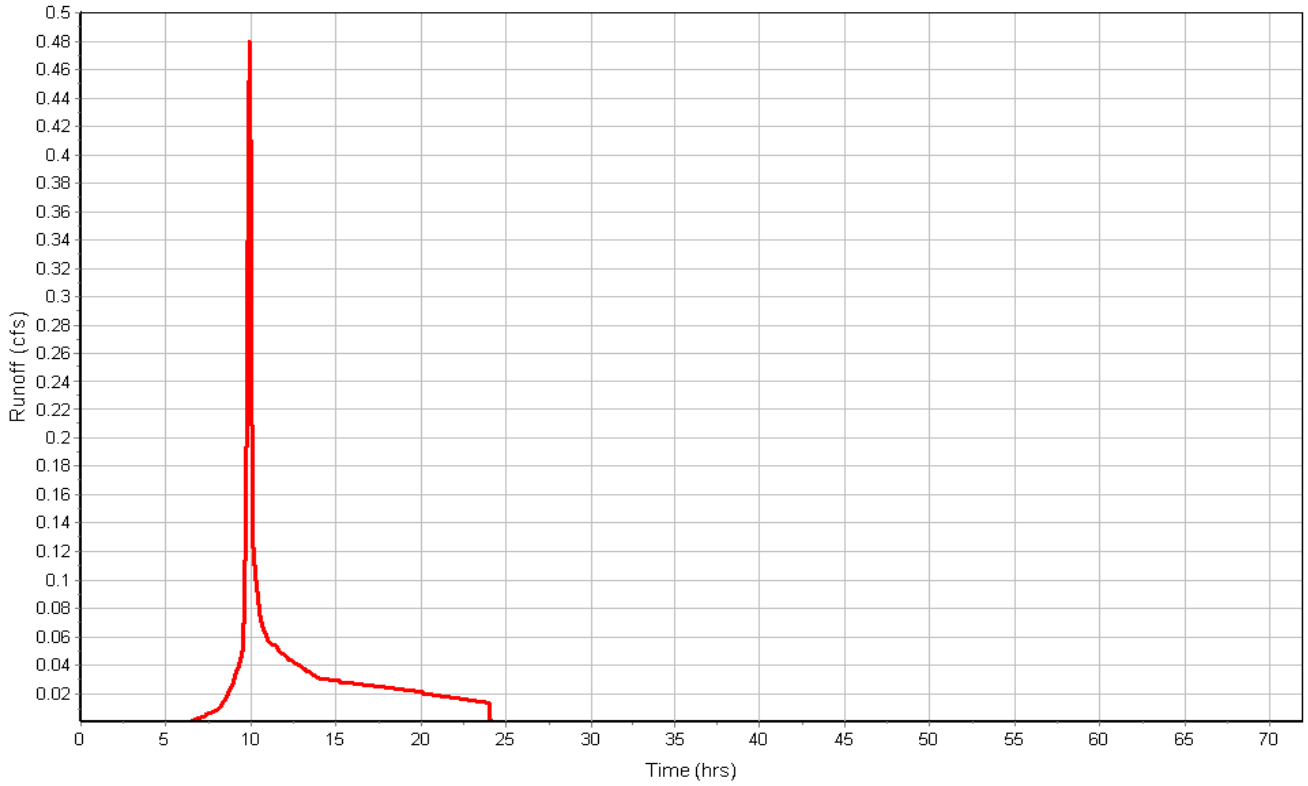


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	DI-2	789.15	794.90	5.75	789.15	0.00	0.00	-794.90	50.00	0.00
2	Jun-04	788.67	794.60	5.93	788.67	0.00	0.00	-794.60	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	0.24	0.00	793.45	4.30	0.00	1.45	791.92	2.77	0 12:53	0 00:00	0.00	0.00
2	Jun-04	0.32	0.00	793.45	4.78	0.00	1.15	791.86	3.19	0 12:53	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	Link-11	245.00	789.90	1.23	790.60	0.00	-0.70	-0.2900	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-16	85.00	789.15	0.00	788.67	0.00	0.48	0.5600	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
3	Link-17	13.00	788.60	0.50	788.67	0.00	-0.07	-0.5400	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	Link-11	0.09	0 10:04	0.56	0.17	1.26	3.24	0.67	1.00	3209.00		SURCHARGED
2	Link-16	0.23	0 10:03	0.79	0.29	0.92	1.54	0.67	1.00	3713.00		SURCHARGED
3	Link-17	0.25	0 10:03	0.77	0.32	0.91	0.24	0.67	1.00	3717.00		SURCHARGED

Exhibit A

Storage Nodes

Storage Node : DI-4*

Input Data

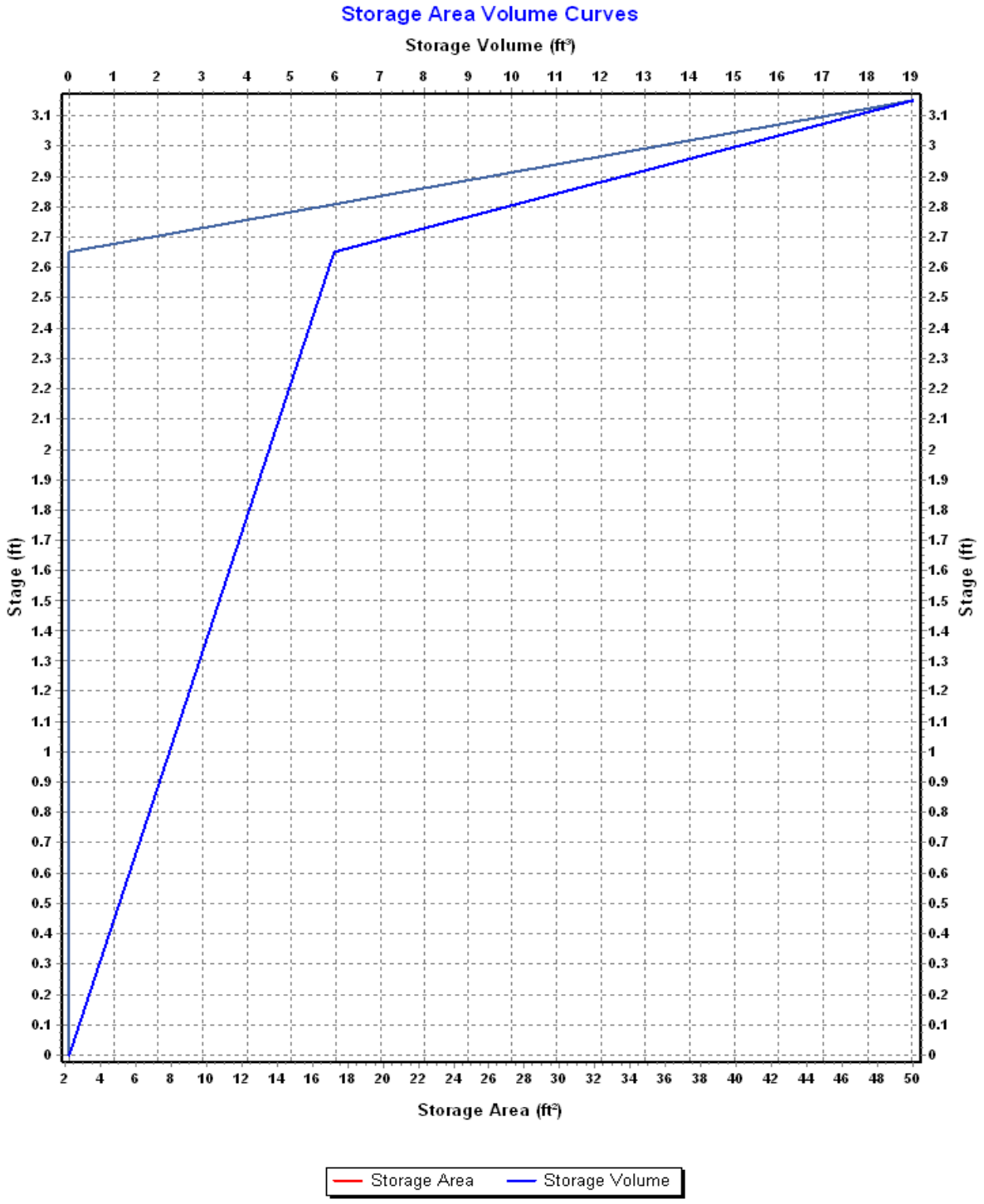
Invert Elevation (ft)	790.60
Max (Rim) Elevation (ft)	794.10
Max (Rim) Offset (ft)	3.50
Initial Water Elevation (ft)	790.60
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : DI-4

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	2.25	0.000
1.33	2.25	2.99
2.65	2.25	5.96
3.15	50	19.02

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19
Stage (ft)
3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : DI-4* (continued)

Outflow Weirs

SN	Element ID	Weir Type	Flap Gate	Crest Elevation (ft)	Crest Offset (ft)	Length (ft)	Weir Total Height (ft)	Discharge Coefficient
1	UNDR	Rectangular	No	793.40	2.80	1.00	0.50	3.33

Output Summary Results

Peak Inflow (cfs)	0.10
Peak Lateral Inflow (cfs)	0.10
Peak Outflow (cfs)	0.09
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	793.45
Max HGL Depth Attained (ft)	2.85
Average HGL Elevation Attained (ft)	792.13
Average HGL Depth Attained (ft)	1.53
Time of Max HGL Occurrence (days hh:mm)	0 12:53
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SUB-1

Input Data

Invert Elevation (ft)	788.10
Max (Rim) Elevation (ft)	795.10
Max (Rim) Offset (ft)	7.00
Initial Water Elevation (ft)	788.10
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : UNDERGROUND

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	74.7600	0.000
0.083	74.7600	6.21
0.167	74.7600	12.49
0.250	74.7600	18.70
0.333	74.7600	24.91
0.417	74.7600	31.19
0.500	74.760	37.40
0.501	165.7740	37.52
0.667	157.1640	64.32
0.750	156.6600	77.34
0.833	156.4080	90.33
0.917	156.1560	103.46
1.000	155.9040	116.41
1.083	155.6520	129.34
1.167	153.6360	142.33
1.250	151.8720	155.01
1.333	151.3680	167.59
1.417	151.1160	180.29
1.500	150.6120	192.81
1.583	150.1080	205.29
1.667	149.3520	217.87
1.750	148.8480	230.25
1.833	146.0760	242.49
1.917	143.0520	254.63
2.000	141.2880	266.43
2.083	138.7680	278.05
2.167	136.2480	289.60
2.250	133.4760	300.79
2.333	130.4520	311.74
2.417	126.9240	322.55
2.500	122.8920	332.92
2.583	118.3560	342.93
2.667	112.5600	352.63
2.750	106.0080	361.70
2.833	95.9280	370.08
2.917	87.6120	377.79
3.000	79.5480	384.73
3.083	74.7600	391.13
3.167	74.7600	397.41
3.250	74.7600	403.62
3.333	74.7600	409.83
3.417	74.7600	416.11
3.500	74.7600	422.32
3.501	84	422.40
5.5	84	590.32
5.501	876	590.80
6	1207	1110.51
6.5	1538	1796.76

Exhibit A

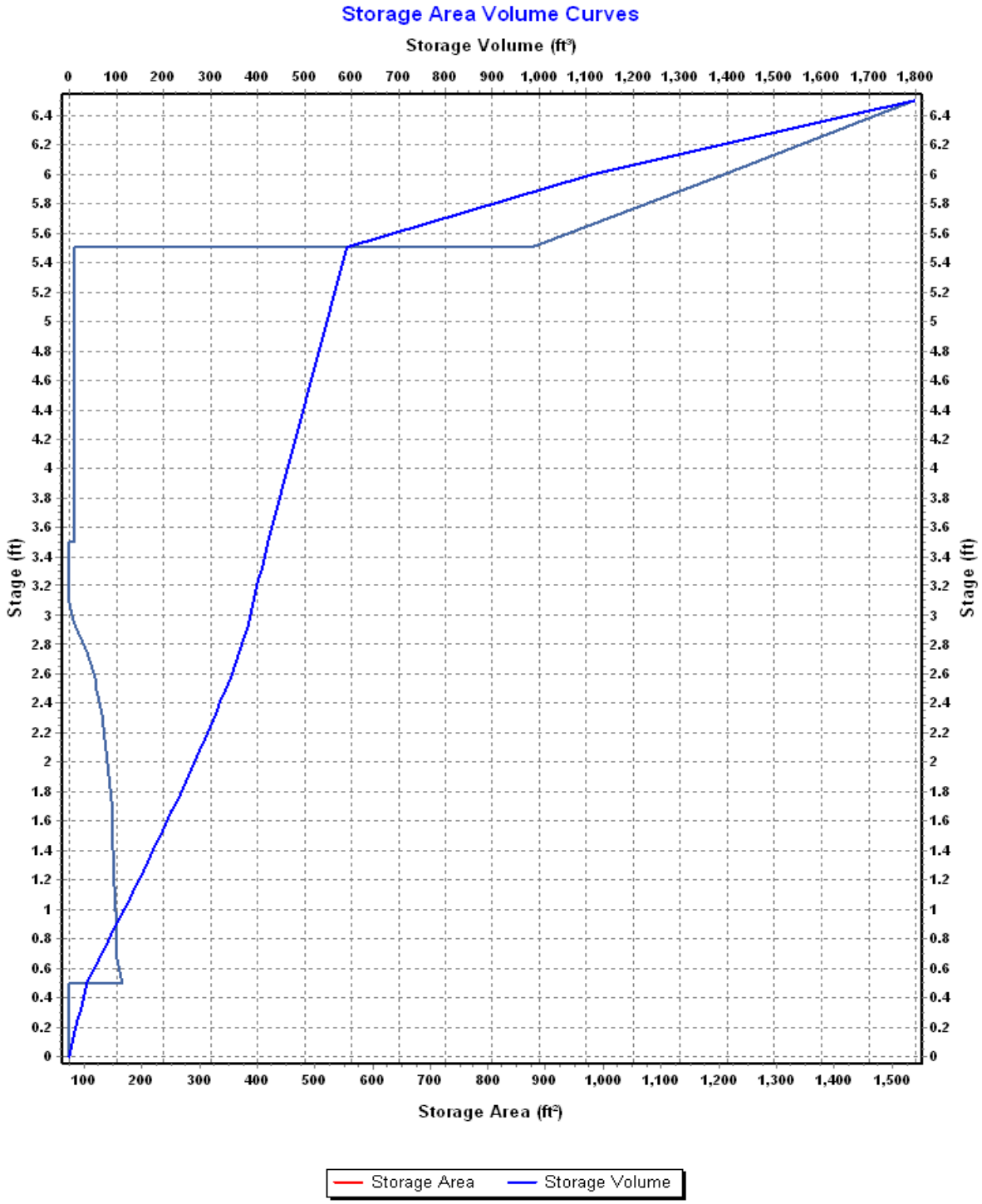

Storage Area Volume Curves
Storage Volume (ft³)
0 100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500 1,600 1,700 1,800
Stage (ft)
6.4 6.2 6 5.8 5.6 5.4 5.2 5 4.8 4.6 4.4 4.2 4 3.8 3.6 3.4 3.2 3 2.8 2.6 2.4 2.2 2 1.8 1.6 1.4 1.2 1 0.8 0.6 0.4 0.2 0
Storage Area (ft²)
100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500
Storage Area
Storage Volume

Exhibit A

Storage Node : SUB-1 (continued)

Output Summary Results

Peak Inflow (cfs)	0.33
Peak Lateral Inflow (cfs)	0.08
Peak Outflow (cfs)	0.01
Peak Exfiltration Flow Rate (cfm)	0.12
Max HGL Elevation Attained (ft)	793.45
Max HGL Depth Attained (ft)	5.35
Average HGL Elevation Attained (ft)	791.78
Average HGL Depth Attained (ft)	3.68
Time of Max HGL Occurrence (days hh:mm)	0 12:53
Total Exfiltration Volume (1000-ft³)	0.439
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-WST

Input Data

Invert Elevation (ft)	793.90
Max (Rim) Elevation (ft)	794.90
Max (Rim) Offset (ft)	1.00
Initial Water Elevation (ft)	793.90
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

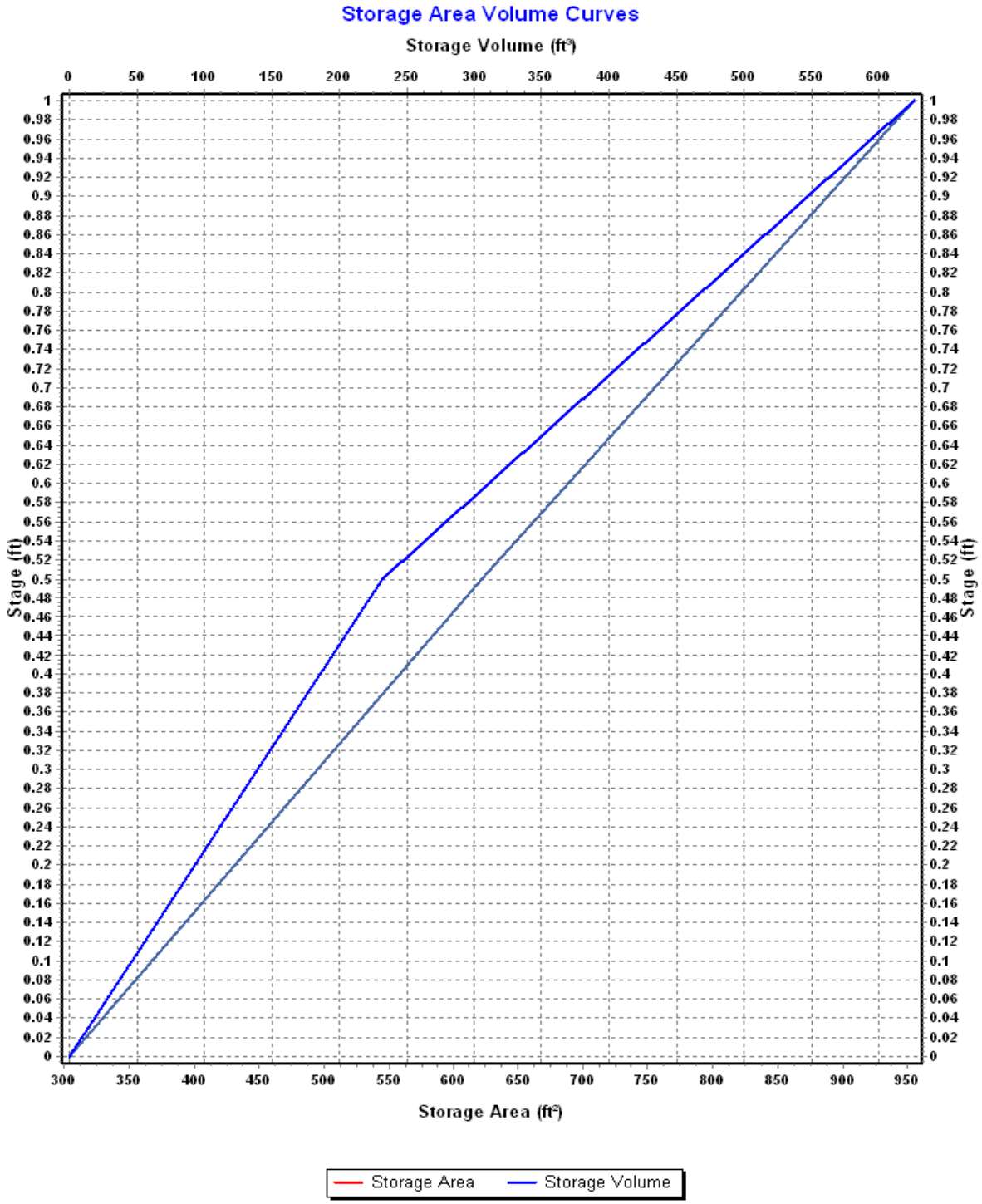
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-1

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	304	0.000
.5	622	231.50
1	955	625.75

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
Stage (ft)
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : SURF-WST (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orfice-01	Bottom	Rectangular	No		18.00	18.00	794.40	0.63

Output Summary Results

Peak Inflow (cfs)	0.29
Peak Lateral Inflow (cfs)	0.29
Peak Outflow (cfs)	0.24
Peak Exfiltration Flow Rate (cfm)	0.46
Max HGL Elevation Attained (ft)	794.45
Max HGL Depth Attained (ft)	0.55
Average HGL Elevation Attained (ft)	794.04
Average HGL Depth Attained (ft)	0.14
Time of Max HGL Occurrence (days hh:mm)	0 10:04
Total Exfiltration Volume (1000-ft³)	0.680
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Project Description

File Name ..... 2017-02-21_00136 SWCP.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	SCS TR-55
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	Jan 01, 2020	00:00:00
End Analysis On	Jan 04, 2020	00:00:00
Start Reporting On	Jan 01, 2020	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	1	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	5
Nodes	7
Junctions	2
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	3
Links	5
Channels	0
Pipes	3
Pumps	0
Orifices	1
Weirs	1
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	2-YEAR	Cumulative	inches	California	San Luis Obispo (Paso Robles)	2	2.11	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	DMA-1	0.19	87.43	2.11	1.02	0.19	0.15	0 00:04:25
2	DMA-2	0.07	85.34	2.11	0.89	0.06	0.05	0 00:04:08
3	DMA-3	0.43	93.85	2.11	1.49	0.65	0.51	0 00:05:46
4	DMA-4	0.16	93.39	2.11	1.45	0.24	0.19	0 00:06:16
5	PRE-SUB	0.85	91.43	2.11	1.29	1.10	0.95	0 00:02:18

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	Junction	789.15	794.90	789.15	0.00	50.00	0.48	793.77	0.00	1.13	0 00:00	0.00	0.00
2	Jun-04	Junction	788.67	794.60	788.67	0.00	0.00	0.45	793.68	0.00	0.92	0 00:00	0.00	0.00
3	OUT-POST	Outfall	793.30					0.27	793.30					
4	OUT-PRE	Outfall	0.00					0.90	0.00					
5	DI-4*	Storage Node	790.60	794.10	790.60		0.00	0.47	793.59				0.00	0.00
6	SUB-1	Storage Node	788.10	795.10	788.10		0.00	0.56	793.68				0.00	0.00
7	SURF-WST	Storage Node	793.90	794.90	793.90		0.00	0.51	794.48				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	Link-11	Pipe	Jun-04	DI-4*	245.00	789.90	790.60	-0.2900	8.000	0.0150	0.29	0.56	0.52	1.18	0.67	1.00	1281.00	SURCHARGED
2	Link-16	Pipe	DI-2	Jun-04	85.00	789.15	788.67	0.5600	8.000	0.0150	0.45	0.79	0.58	1.30	0.67	1.00	1795.00	SURCHARGED
3	Link-17	Pipe	SUB-1	Jun-04	13.00	788.60	788.67	-0.5400	8.000	0.0150	0.38	0.77	0.49	1.08	0.67	1.00	1964.00	SURCHARGED
4	Orfice-01	Orifice	SURF-WST	DI-2		793.90	789.15		18.000		0.48							
5	UNDR	Weir	DI-4*	OUT-POST		790.60	793.30				0.27							

Exhibit A

Subbasin Hydrology

Subbasin : DMA-1

Input Data

Area (ac)	0.19
Weighted Curve Number	87.43
Rain Gage ID	NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.11	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.19		87.43

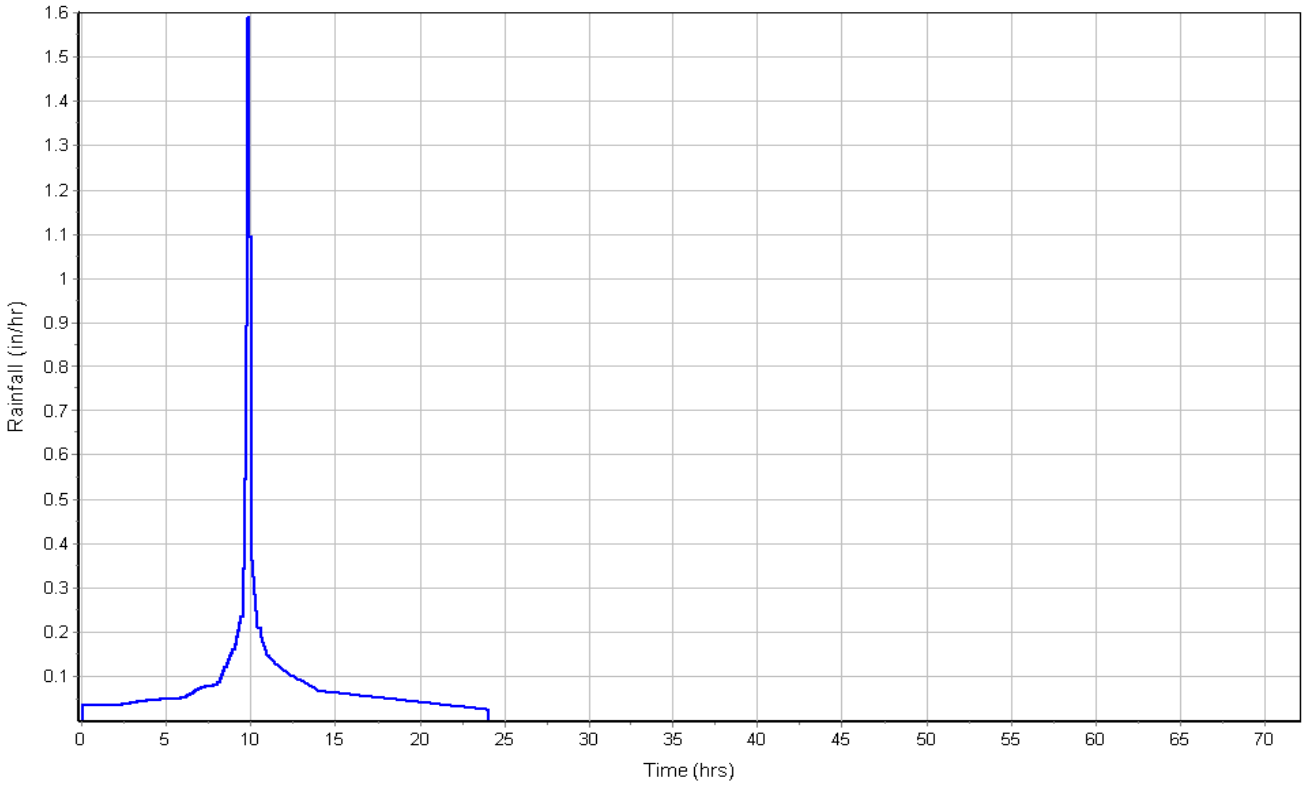
Subbasin Runoff Results

Total Rainfall (in)	2.11
Total Runoff (in)	1.02
Peak Runoff (cfs)	0.15
Weighted Curve Number	87.43
Time of Concentration (days hh:mm:ss)	0 00:04:26

Exhibit A

Subbasin : DMA-1

Rainfall Intensity Graph



Runoff Hydrograph

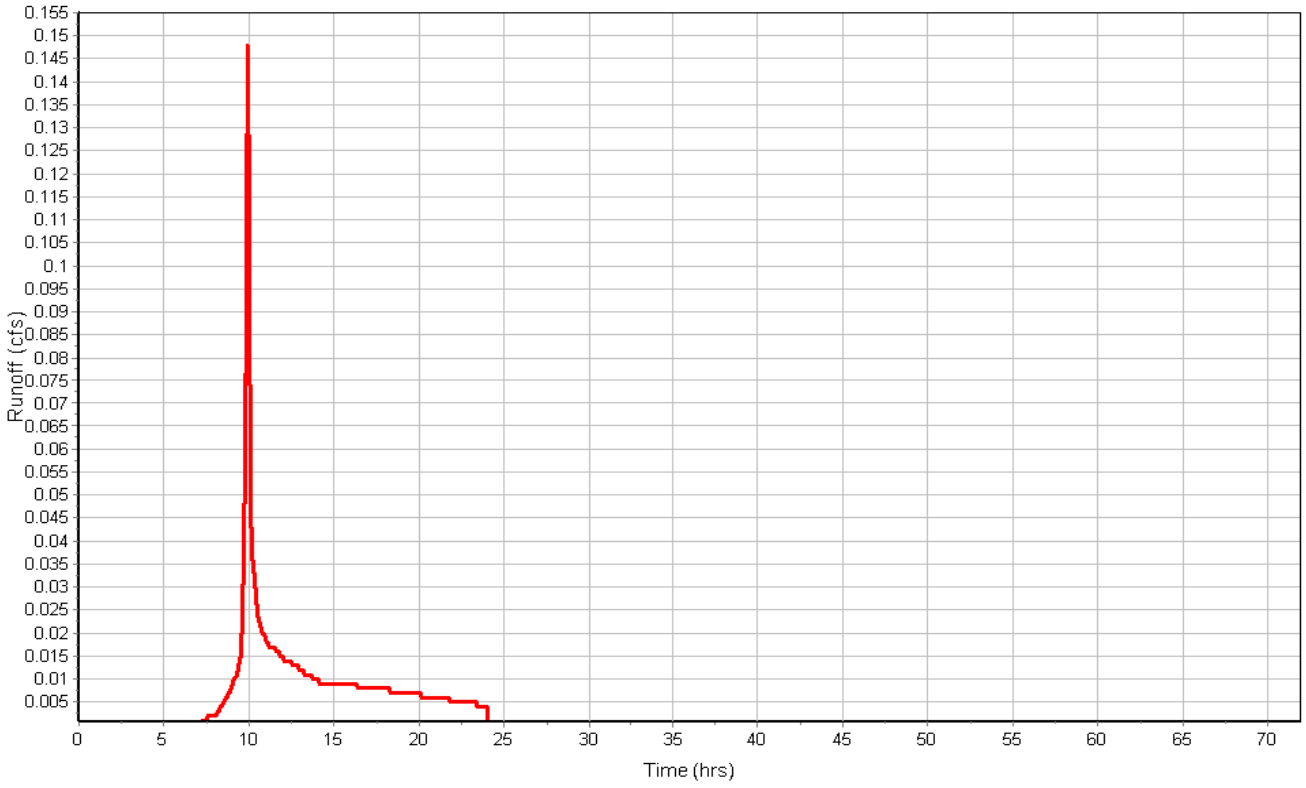


Exhibit A

Subbasin : DMA-2

Input Data

Area (ac) 0.07
Weighted Curve Number 85.34
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	98.00
-	0.04	-	74.00
Composite Area & Weighted CN	0.07		85.34

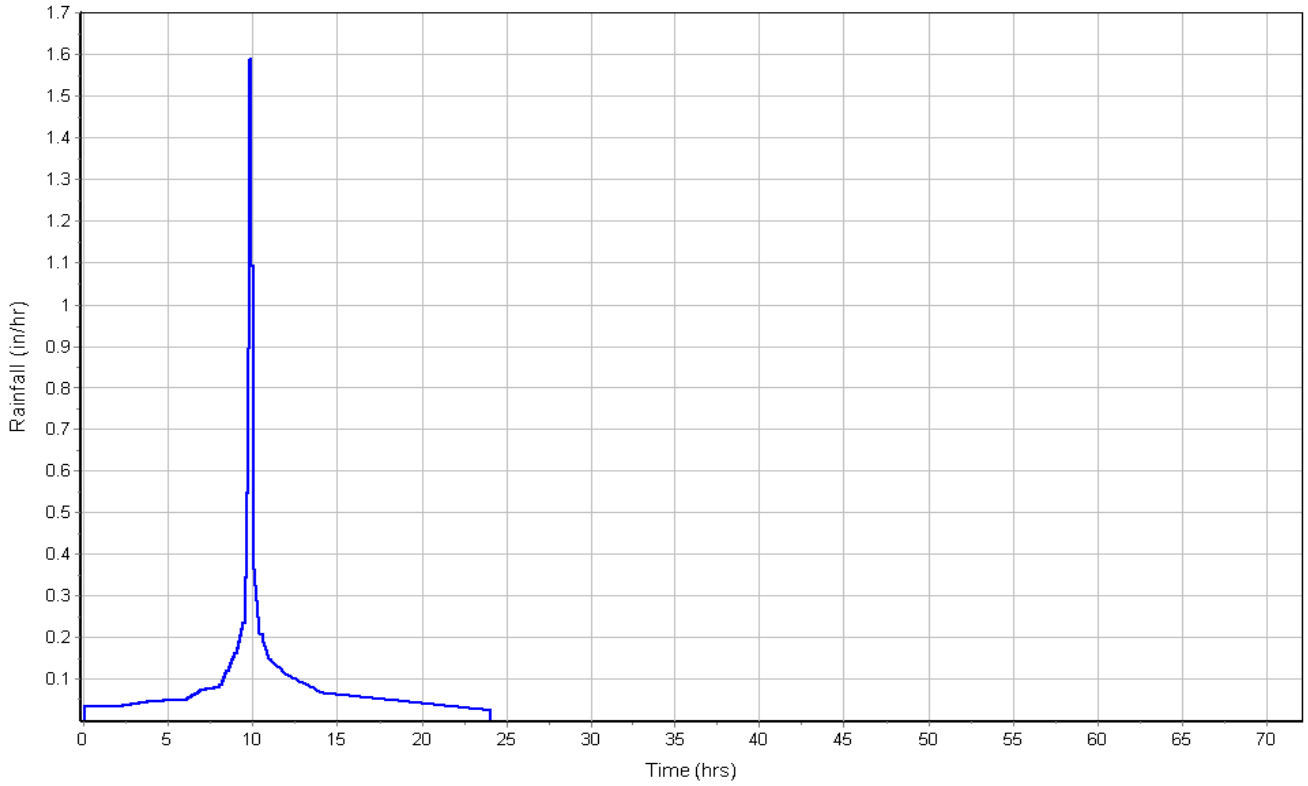
Subbasin Runoff Results

Total Rainfall (in) 2.11
Total Runoff (in) 0.89
Peak Runoff (cfs) 0.05
Weighted Curve Number 85.34
Time of Concentration (days hh:mm:ss) 0 00:04:08

Exhibit A

Subbasin : DMA-2

Rainfall Intensity Graph



Runoff Hydrograph

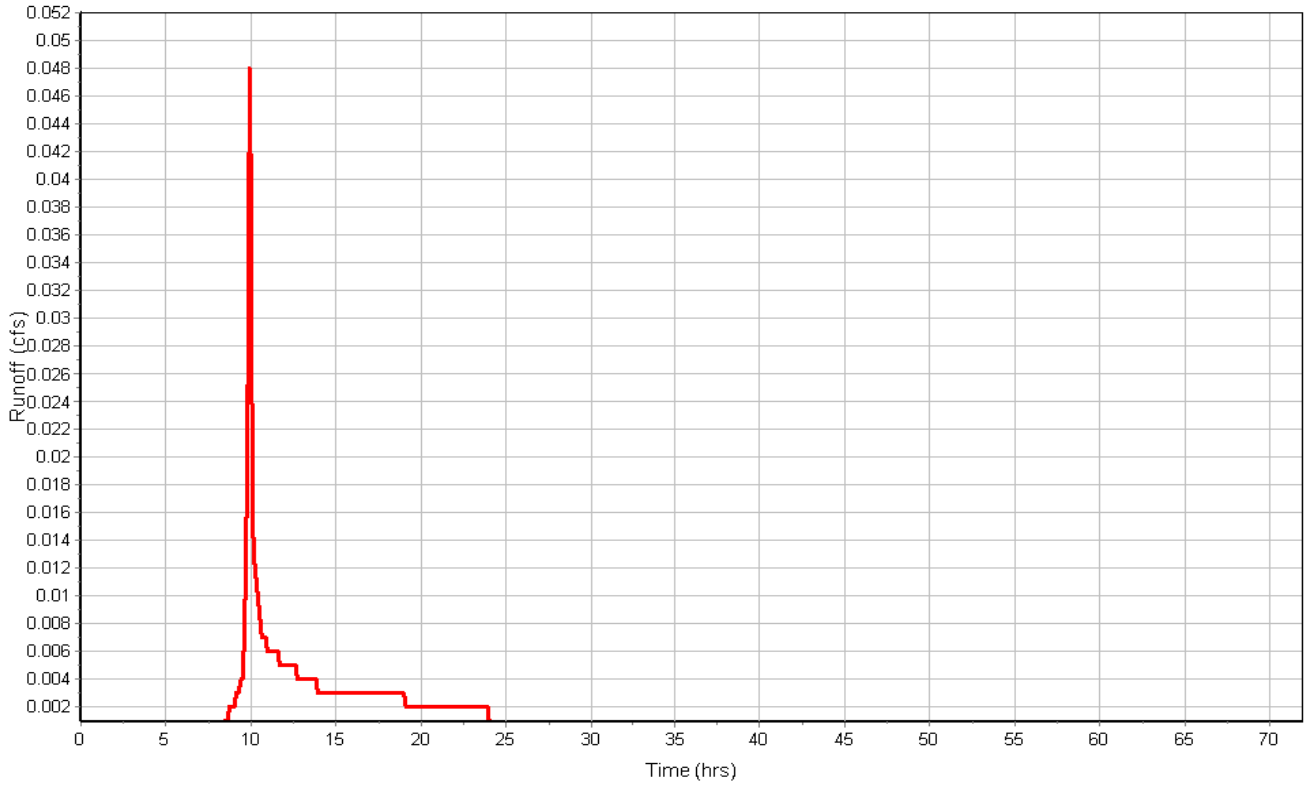


Exhibit A

Subbasin : DMA-3

Input Data

Area (ac) 0.43
Weighted Curve Number 93.85
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.36	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.44		93.85

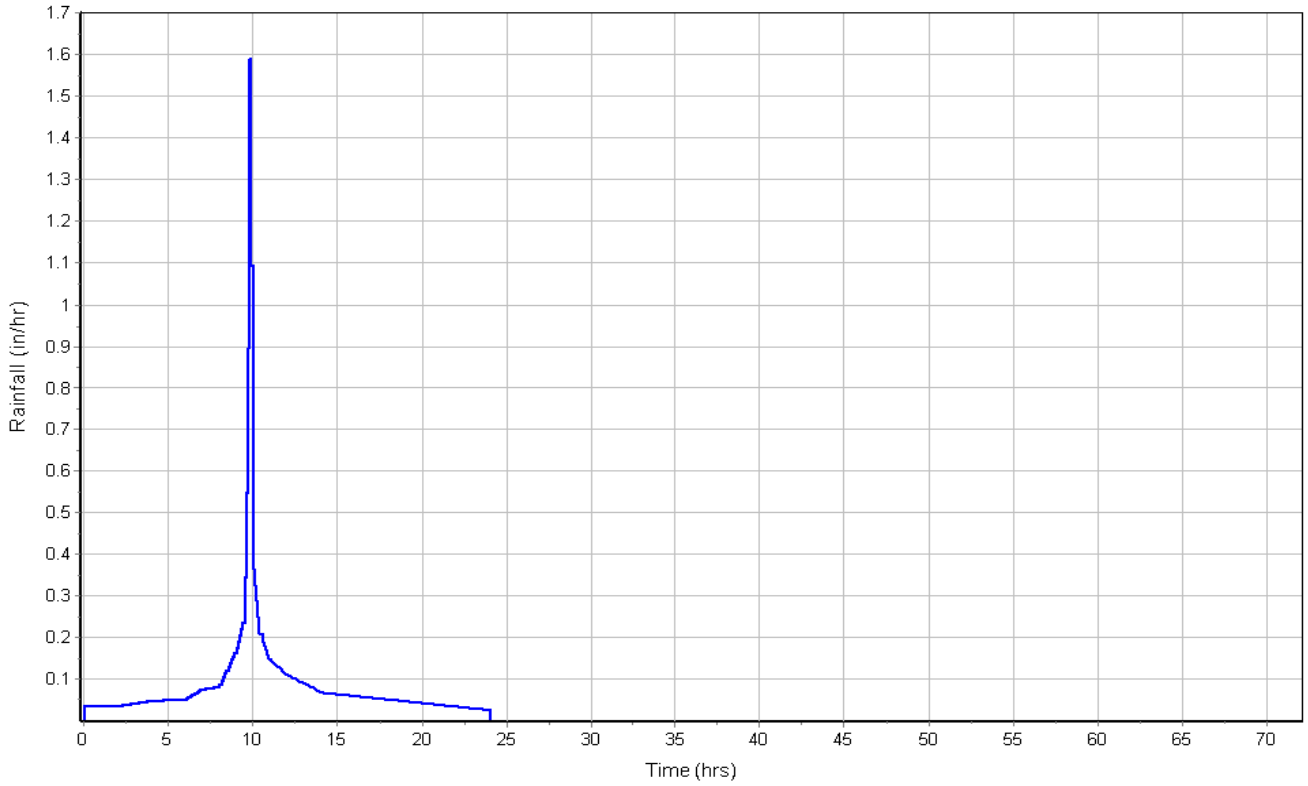
Subbasin Runoff Results

Total Rainfall (in) 2.11
Total Runoff (in) 1.49
Peak Runoff (cfs) 0.51
Weighted Curve Number 93.85
Time of Concentration (days hh:mm:ss) 0 00:05:46

Exhibit A

Subbasin : DMA-3

Rainfall Intensity Graph



Runoff Hydrograph

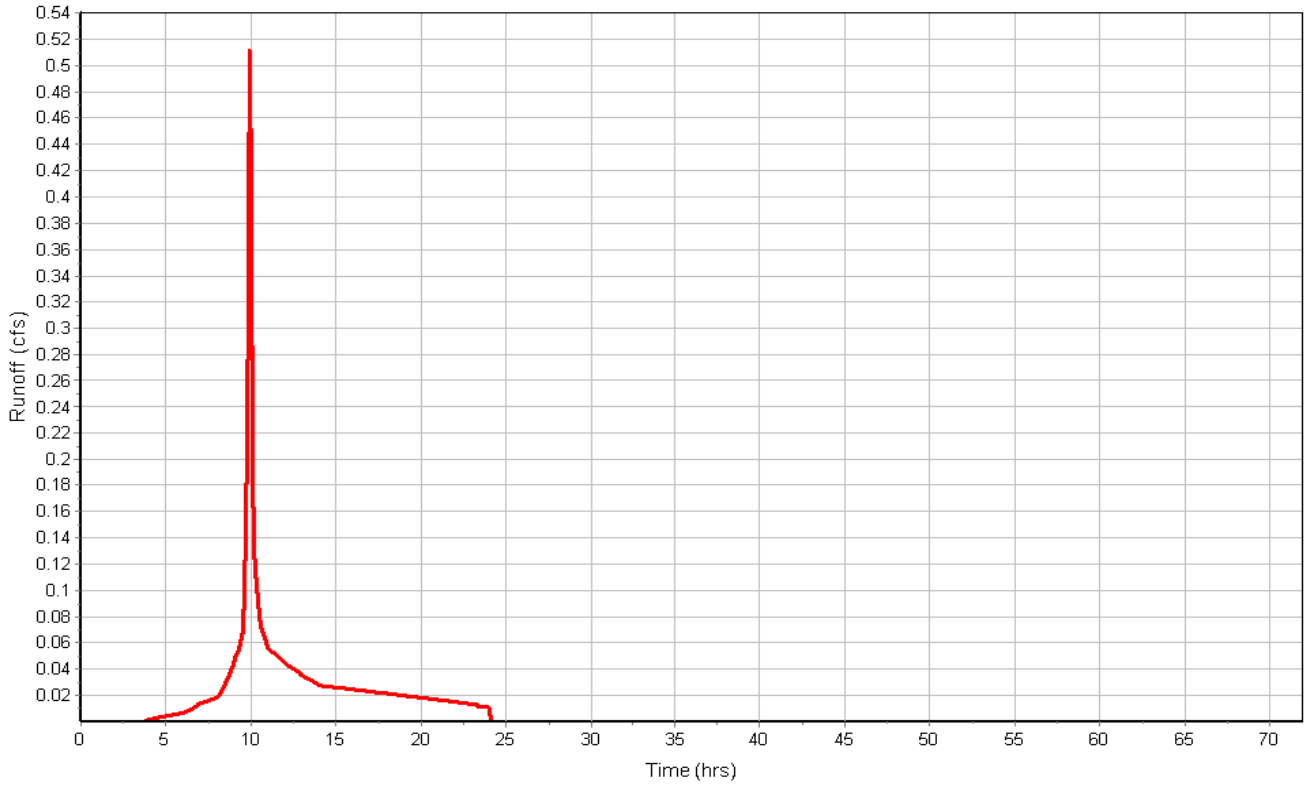


Exhibit A

Subbasin : DMA-4

Input Data

Area (ac) 0.16
Weighted Curve Number 93.39
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.13	-	98.00
-	0.03	-	74.00
Composite Area & Weighted CN	0.16		93.39

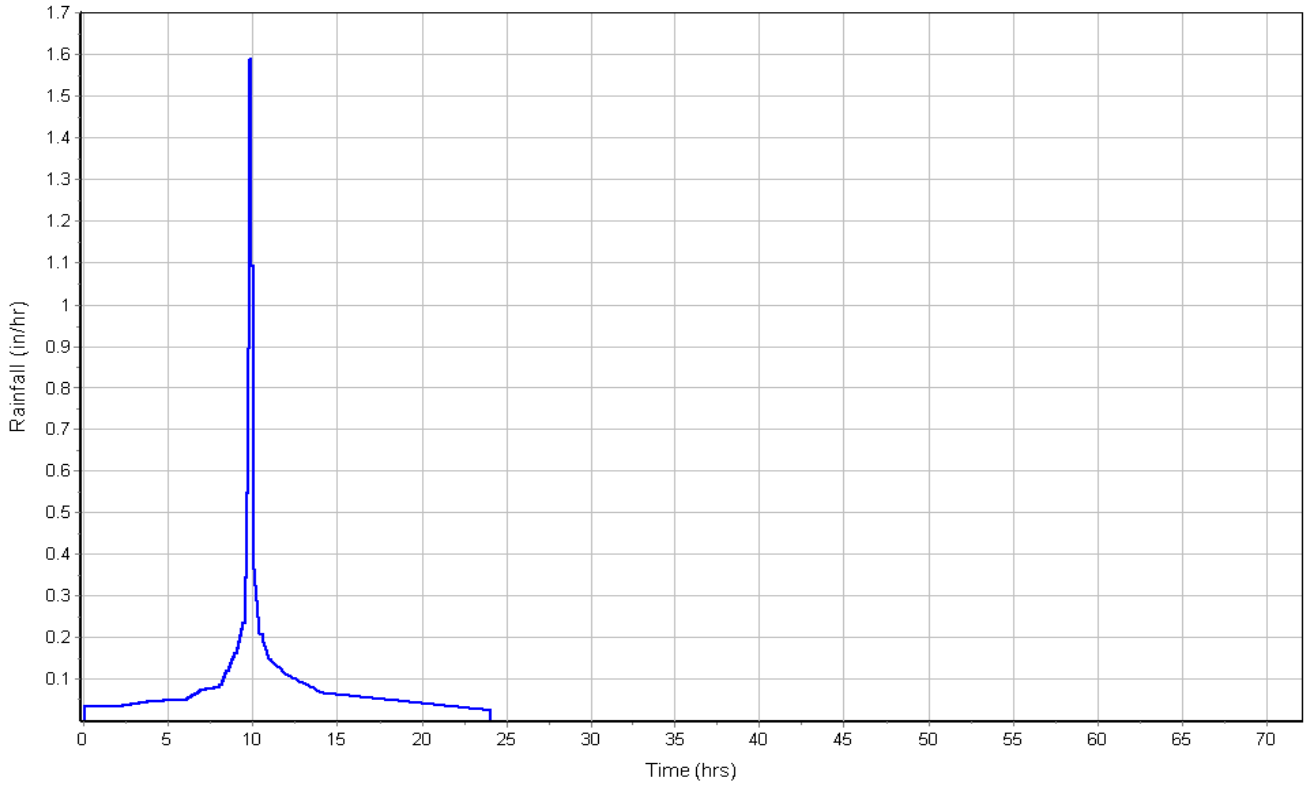
Subbasin Runoff Results

Total Rainfall (in) 2.11
Total Runoff (in) 1.45
Peak Runoff (cfs) 0.19
Weighted Curve Number 93.39
Time of Concentration (days hh:mm:ss) 0 00:06:17

Exhibit A

Subbasin : DMA-4

Rainfall Intensity Graph



Runoff Hydrograph

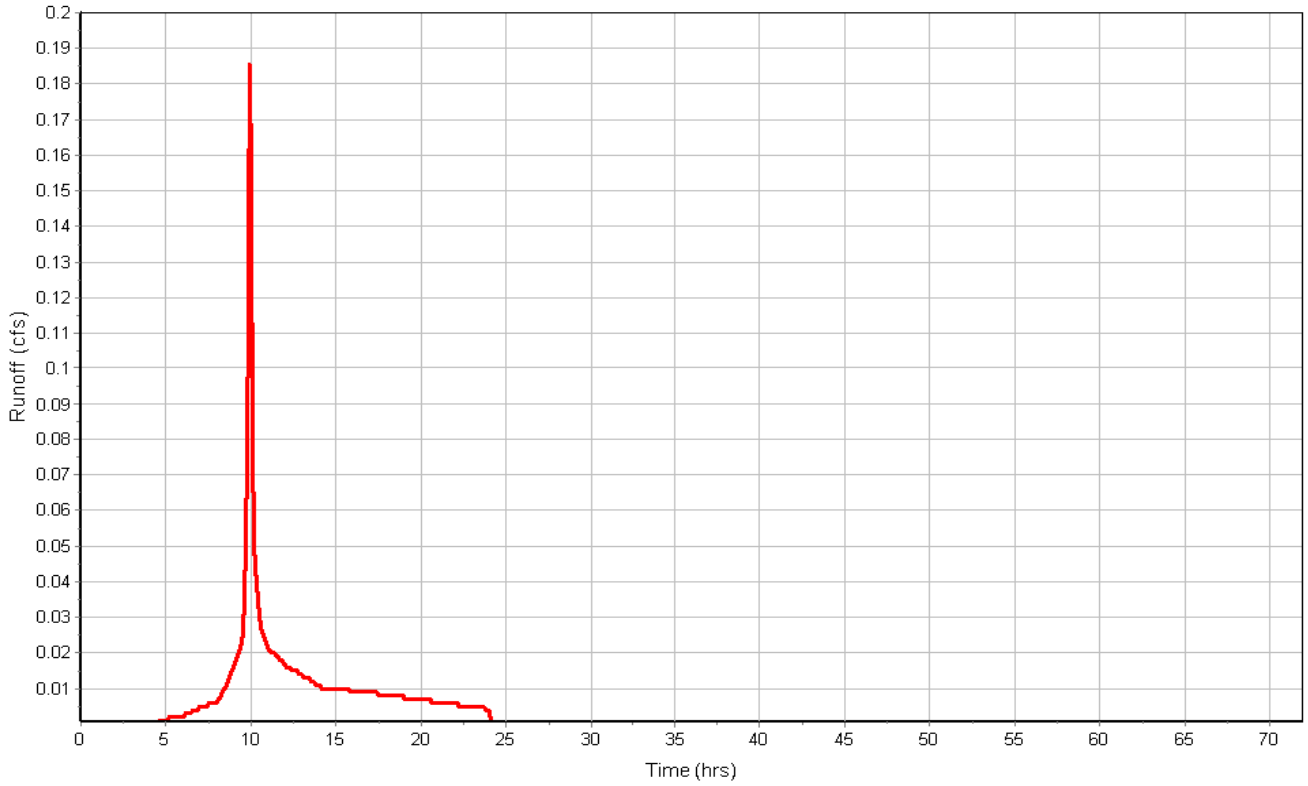


Exhibit A

Subbasin : PRE-SUB

Input Data

Area (ac) 0.85
Weighted Curve Number 91.43
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.62	-	98.00
-	0.23	-	74.00
Composite Area & Weighted CN	0.85		91.43

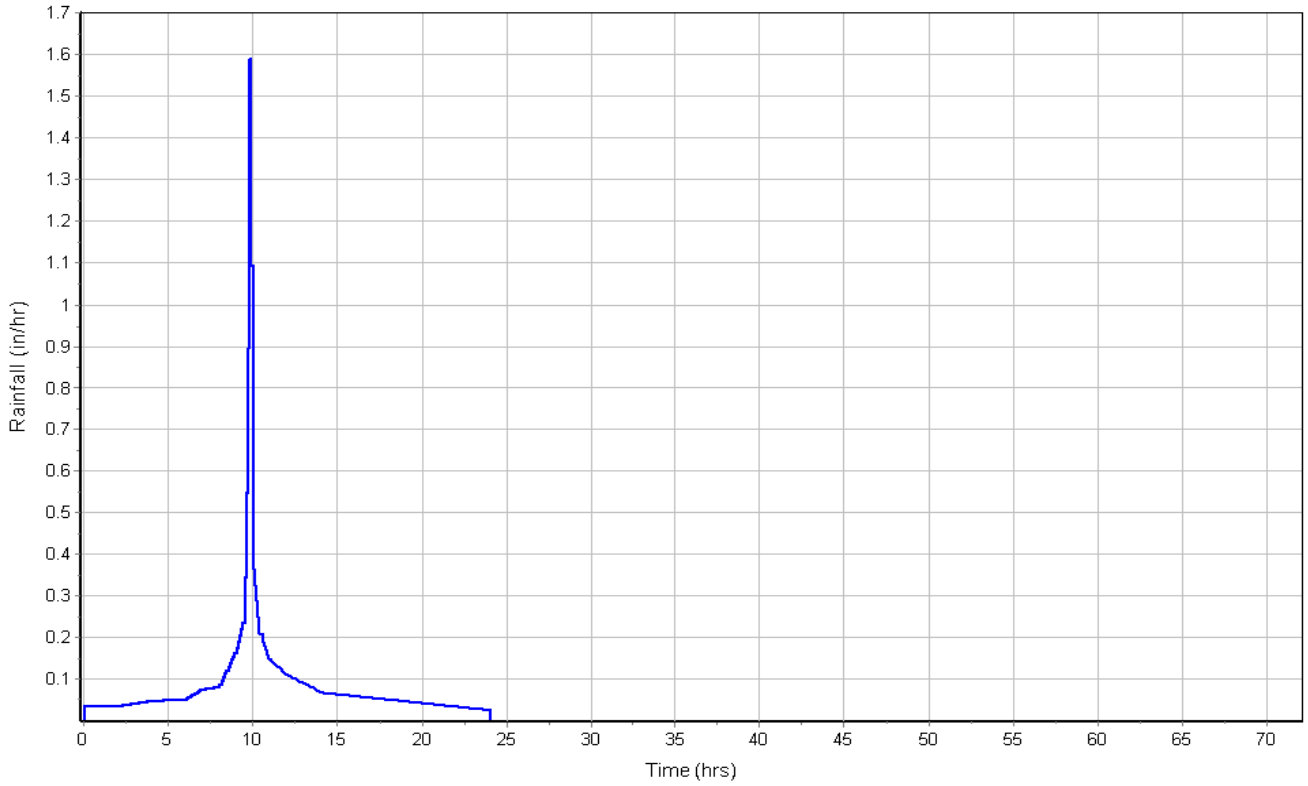
Subbasin Runoff Results

Total Rainfall (in) 2.11
Total Runoff (in) 1.29
Peak Runoff (cfs) 0.95
Weighted Curve Number 91.43
Time of Concentration (days hh:mm:ss) 0 00:02:18

Exhibit A

Subbasin : PRE-SUB

Rainfall Intensity Graph



Runoff Hydrograph

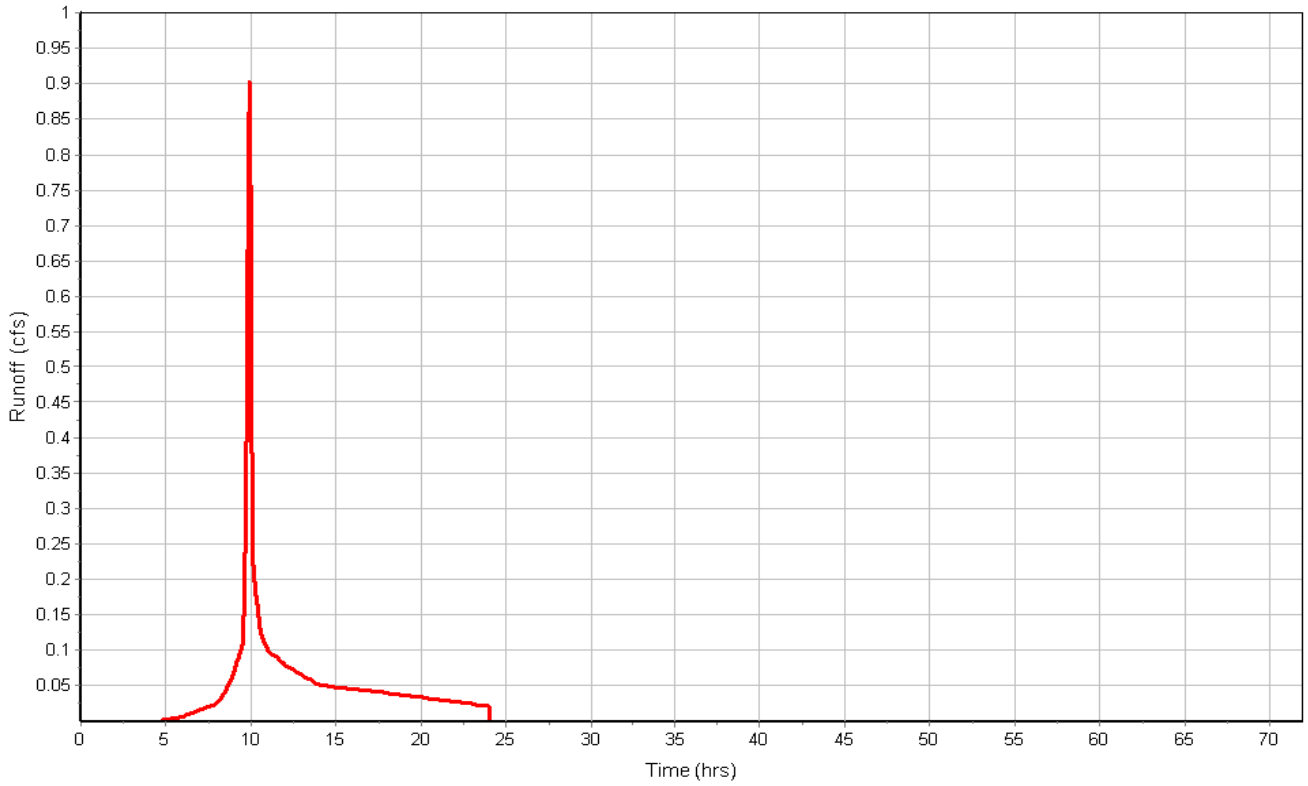


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	DI-2	789.15	794.90	5.75	789.15	0.00	0.00	-794.90	50.00	0.00
2	Jun-04	788.67	794.60	5.93	788.67	0.00	0.00	-794.60	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	0.48	0.00	793.77	4.62	0.00	1.13	790.49	1.34	0 10:06	0 00:00	0.00	0.00
2	Jun-04	0.45	0.00	793.68	5.01	0.00	0.92	790.25	1.58	0 10:14	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	Link-11	245.00	789.90	1.23	790.60	0.00	-0.70	-0.2900	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-16	85.00	789.15	0.00	788.67	0.00	0.48	0.5600	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
3	Link-17	13.00	788.60	0.50	788.67	0.00	-0.07	-0.5400	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	Link-11	0.29	0 09:59	0.56	0.52	1.18	3.46	0.67	1.00	1281.00		SURCHARGED
2	Link-16	0.45	0 09:59	0.79	0.58	1.30	1.09	0.67	1.00	1795.00		SURCHARGED
3	Link-17	0.38	0 10:01	0.77	0.49	1.08	0.20	0.67	1.00	1964.00		SURCHARGED

Exhibit A

Storage Nodes

Storage Node : DI-4*

Input Data

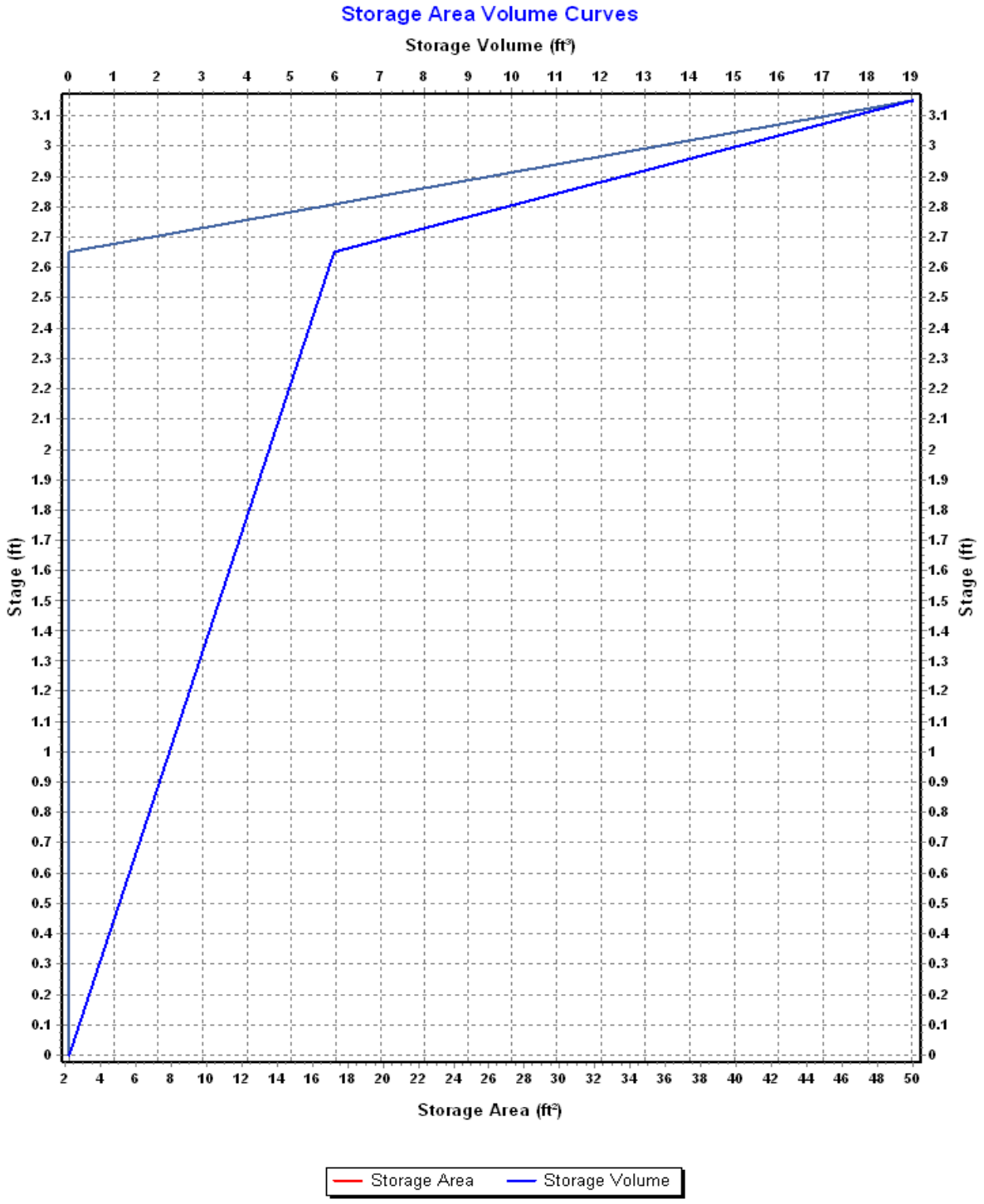
Invert Elevation (ft)	790.60
Max (Rim) Elevation (ft)	794.10
Max (Rim) Offset (ft)	3.50
Initial Water Elevation (ft)	790.60
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : DI-4

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	2.25	0.000
1.33	2.25	2.99
2.65	2.25	5.96
3.15	50	19.02

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19
Stage (ft)
3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : DI-4* (continued)

Outflow Weirs

SN	Element ID	Weir Type	Flap Gate	Crest Elevation (ft)	Crest Offset (ft)	Length (ft)	Weir Total Height (ft)	Discharge Coefficient
1	UNDR	Rectangular	No	793.40	2.80	1.00	0.50	3.33

Output Summary Results

Peak Inflow (cfs)	0.47
Peak Lateral Inflow (cfs)	0.19
Peak Outflow (cfs)	0.27
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	793.59
Max HGL Depth Attained (ft)	2.99
Average HGL Elevation Attained (ft)	791.35
Average HGL Depth Attained (ft)	0.75
Time of Max HGL Occurrence (days hh:mm)	0 10:11
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SUB-1

Input Data

Invert Elevation (ft)	788.10
Max (Rim) Elevation (ft)	795.10
Max (Rim) Offset (ft)	7.00
Initial Water Elevation (ft)	788.10
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

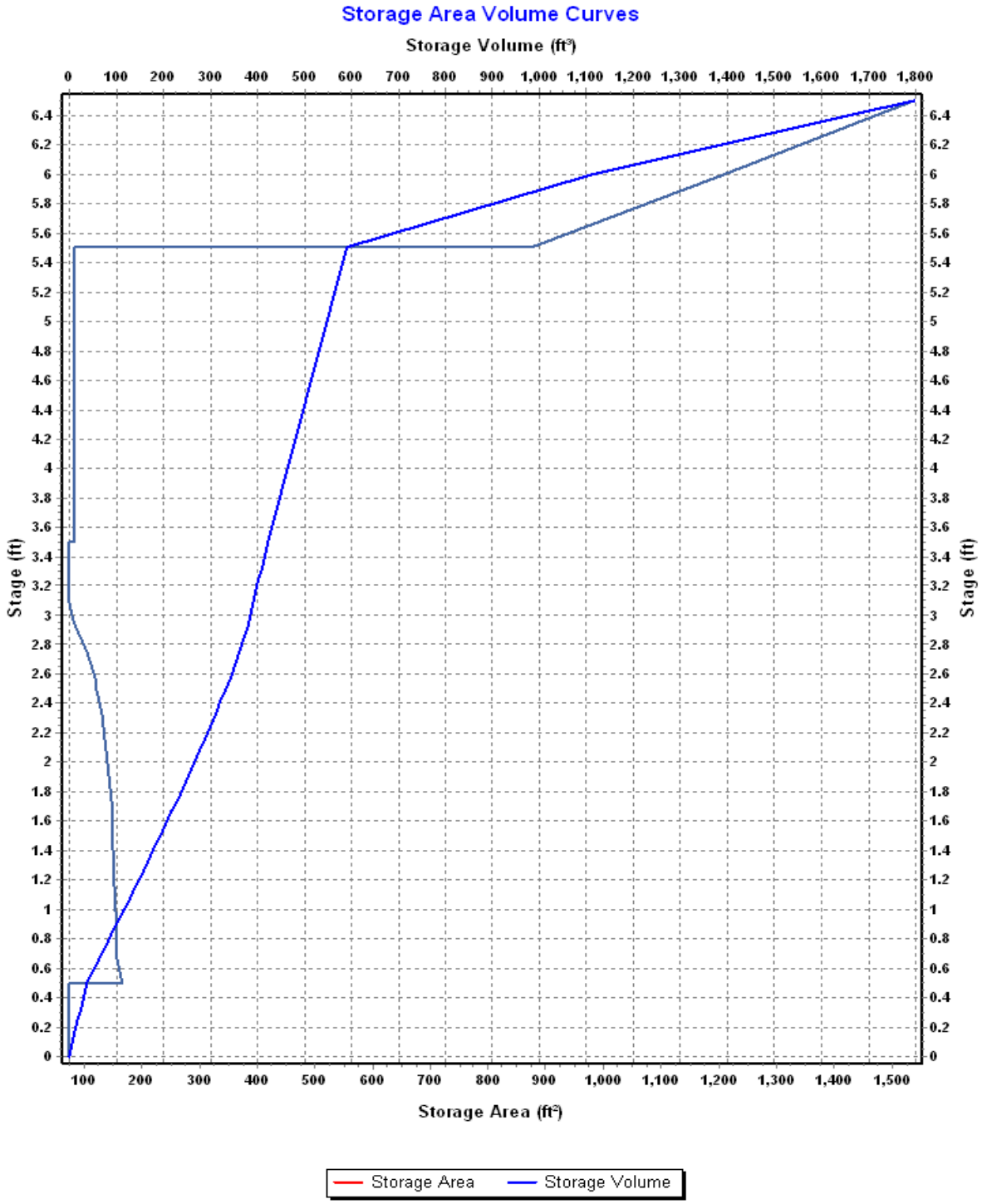
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : UNDERGROUND

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	74.7600	0.000
0.083	74.7600	6.21
0.167	74.7600	12.49
0.250	74.7600	18.70
0.333	74.7600	24.91
0.417	74.7600	31.19
0.500	74.760	37.40
0.501	165.7740	37.52
0.667	157.1640	64.32
0.750	156.6600	77.34
0.833	156.4080	90.33
0.917	156.1560	103.46
1.000	155.9040	116.41
1.083	155.6520	129.34
1.167	153.6360	142.33
1.250	151.8720	155.01
1.333	151.3680	167.59
1.417	151.1160	180.29
1.500	150.6120	192.81
1.583	150.1080	205.29
1.667	149.3520	217.87
1.750	148.8480	230.25
1.833	146.0760	242.49
1.917	143.0520	254.63
2.000	141.2880	266.43
2.083	138.7680	278.05
2.167	136.2480	289.60
2.250	133.4760	300.79
2.333	130.4520	311.74
2.417	126.9240	322.55
2.500	122.8920	332.92
2.583	118.3560	342.93
2.667	112.5600	352.63
2.750	106.0080	361.70
2.833	95.9280	370.08
2.917	87.6120	377.79
3.000	79.5480	384.73
3.083	74.7600	391.13
3.167	74.7600	397.41
3.250	74.7600	403.62
3.333	74.7600	409.83
3.417	74.7600	416.11
3.500	74.7600	422.32
3.501	84	422.40
5.5	84	590.32
5.501	876	590.80
6	1207	1110.51
6.5	1538	1796.76

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500 1,600 1,700 1,800
Stage (ft)
6.4 6.2 6 5.8 5.6 5.4 5.2 5 4.8 4.6 4.4 4.2 4 3.8 3.6 3.4 3.2 3 2.8 2.6 2.4 2.2 2 1.8 1.6 1.4 1.2 1 0.8 0.6 0.4 0.2 0
Storage Area (ft²)
100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500
Storage Area
Storage Volume

Storage Node : SUB-1 (continued)

Output Summary Results

Peak Inflow (cfs)	0.56
Peak Lateral Inflow (cfs)	0.20
Peak Outflow (cfs)	0.09
Peak Exfiltration Flow Rate (cfm)	0.65
Max HGL Elevation Attained (ft)	793.68
Max HGL Depth Attained (ft)	5.58
Average HGL Elevation Attained (ft)	789.99
Average HGL Depth Attained (ft)	1.89
Time of Max HGL Occurrence (days hh:mm)	0 10:14
Total Exfiltration Volume (1000-ft³)	1.465
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-WST

Input Data

Invert Elevation (ft)	793.90
Max (Rim) Elevation (ft)	794.90
Max (Rim) Offset (ft)	1.00
Initial Water Elevation (ft)	793.90
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

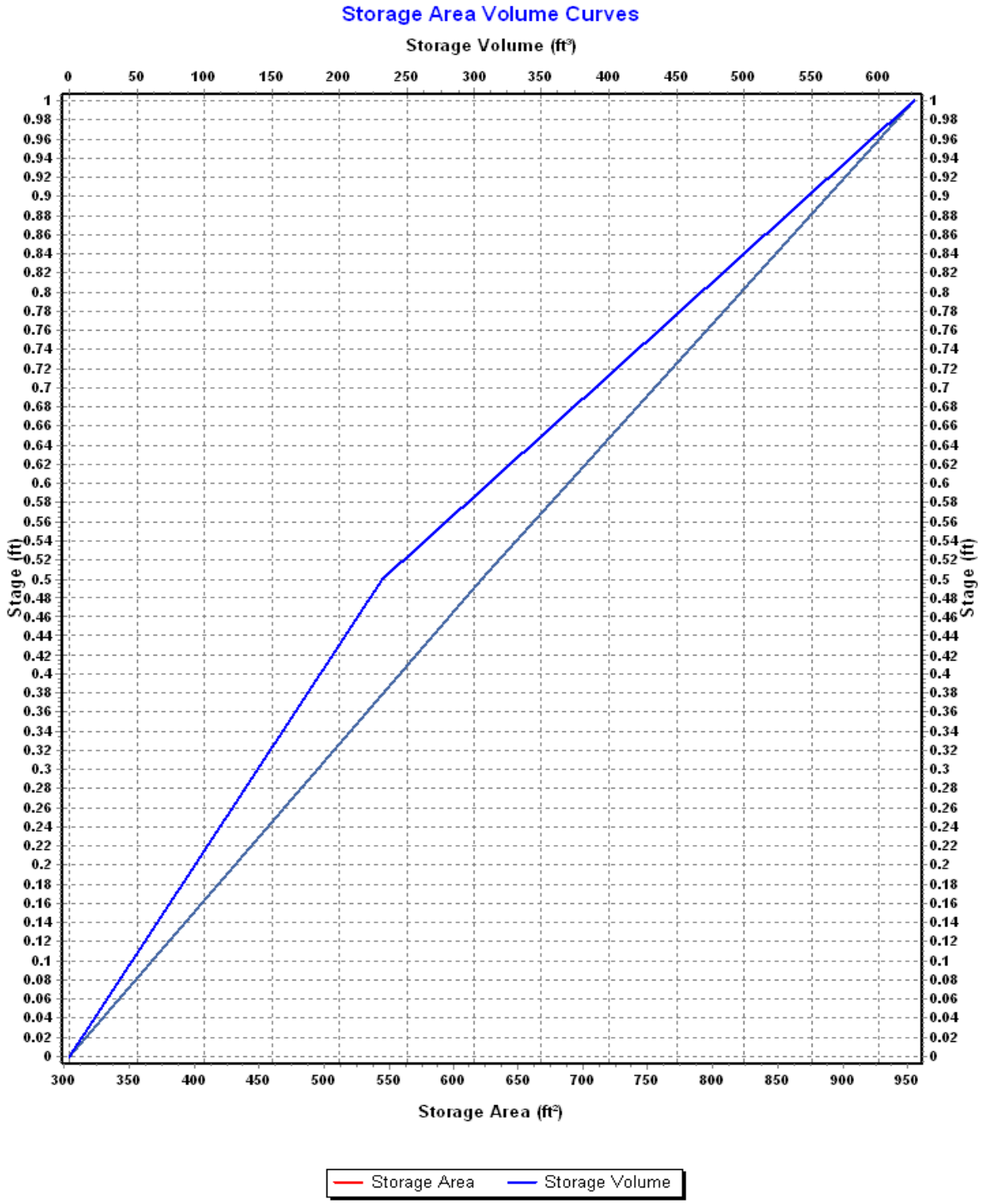
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-1

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	304	0.000
.5	622	231.50
1	955	625.75

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
Stage (ft)
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : SURF-WST (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orfice-01	Bottom	Rectangular	No		18.00	18.00	794.40	0.63

Output Summary Results

Peak Inflow (cfs)	0.51
Peak Lateral Inflow (cfs)	0.51
Peak Outflow (cfs)	0.48
Peak Exfiltration Flow Rate (cfm)	0.47
Max HGL Elevation Attained (ft)	794.48
Max HGL Depth Attained (ft)	0.58
Average HGL Elevation Attained (ft)	794.05
Average HGL Depth Attained (ft)	0.15
Time of Max HGL Occurrence (days hh:mm)	0 10:01
Total Exfiltration Volume (1000-ft³)	0.732
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Project Description

File Name ..... 2017-02-21_00136 SWCP.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	SCS TR-55
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	Jan 01, 2020	00:00:00
End Analysis On	Jan 04, 2020	00:00:00
Start Reporting On	Jan 01, 2020	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	1	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	5
Nodes	7
Junctions	2
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	3
Links	5
Channels	0
Pipes	3
Pumps	0
Orifices	1
Weirs	1
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	10-YEAR	Cumulative	inches	California	San Luis Obispo (Paso Robles)	10	3.68	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	DMA-1	0.19	87.43	3.68	2.38	0.45	0.36	0 00:04:25
2	DMA-2	0.07	85.34	3.68	2.19	0.15	0.13	0 00:04:08
3	DMA-3	0.43	93.85	3.68	3.00	1.30	1.03	0 00:05:46
4	DMA-4	0.16	93.39	3.68	2.95	0.48	0.38	0 00:06:16
5	PRE-SUB	0.85	91.43	3.68	2.75	2.35	2.02	0 00:02:18

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	Junction	789.15	794.90	789.15	0.00	50.00	0.68	794.59	0.00	0.31	0 00:00	0.00	0.00
2	Jun-04	Junction	788.67	794.60	788.67	0.00	0.00	0.68	794.34	0.00	0.26	0 00:00	0.00	0.00
3	OUT-POST	Outfall	793.30					0.76	793.30					
4	OUT-PRE	Outfall	0.00					1.89	0.00					
5	DI-4*	Storage Node	790.60	794.10	790.60		0.00	0.76	793.77				0.00	0.00
6	SUB-1	Storage Node	788.10	795.10	788.10		0.00	0.76	794.35				0.00	0.00
7	SURF-WST	Storage Node	793.90	794.90	793.90		0.00	1.02	794.70				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	Link-11	Pipe	Jun-04	DI-4*	245.00	789.90	790.60	-0.2900	8.000	0.0150	0.51	0.56	0.92	1.47	0.67	1.00	1205.00	SURCHARGED
2	Link-16	Pipe	DI-2	Jun-04	85.00	789.15	788.67	0.5600	8.000	0.0150	0.68	0.79	0.86	1.94	0.67	1.00	1596.00	SURCHARGED
3	Link-17	Pipe	SUB-1	Jun-04	13.00	788.60	788.67	-0.5400	8.000	0.0150	0.34	0.77	0.45	0.99	0.67	1.00	1733.00	SURCHARGED
4	Orfice-01	Orifice	SURF-WST	DI-2		793.90	789.15		18.000		0.68							
5	UNDR	Weir	DI-4*	OUT-POST		790.60	793.30				0.76							

Exhibit A

Subbasin Hydrology

Subbasin : DMA-1

Input Data

Area (ac) 0.19
Weighted Curve Number 87.43
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.11	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.19		87.43

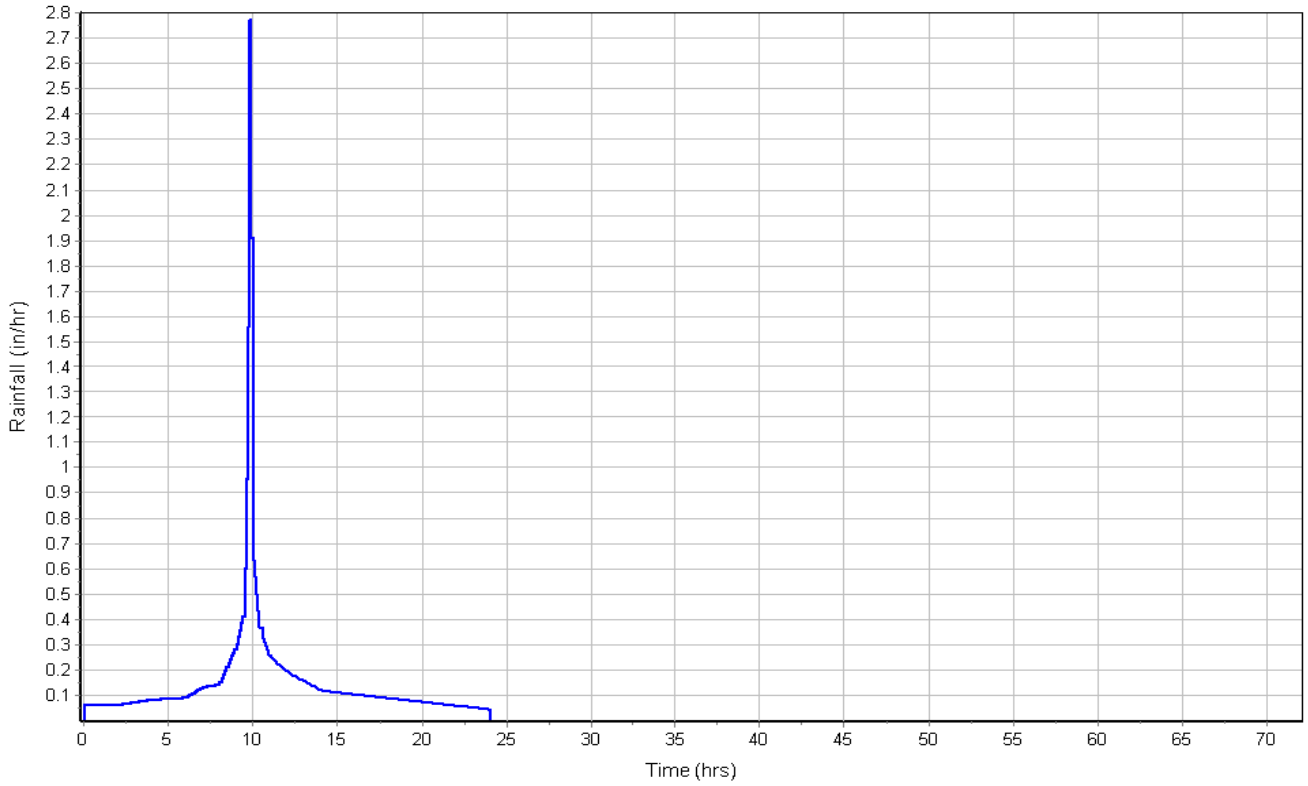
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 2.38
Peak Runoff (cfs) 0.36
Weighted Curve Number 87.43
Time of Concentration (days hh:mm:ss) 0 00:04:26

Exhibit A

Subbasin : DMA-1

Rainfall Intensity Graph



Runoff Hydrograph

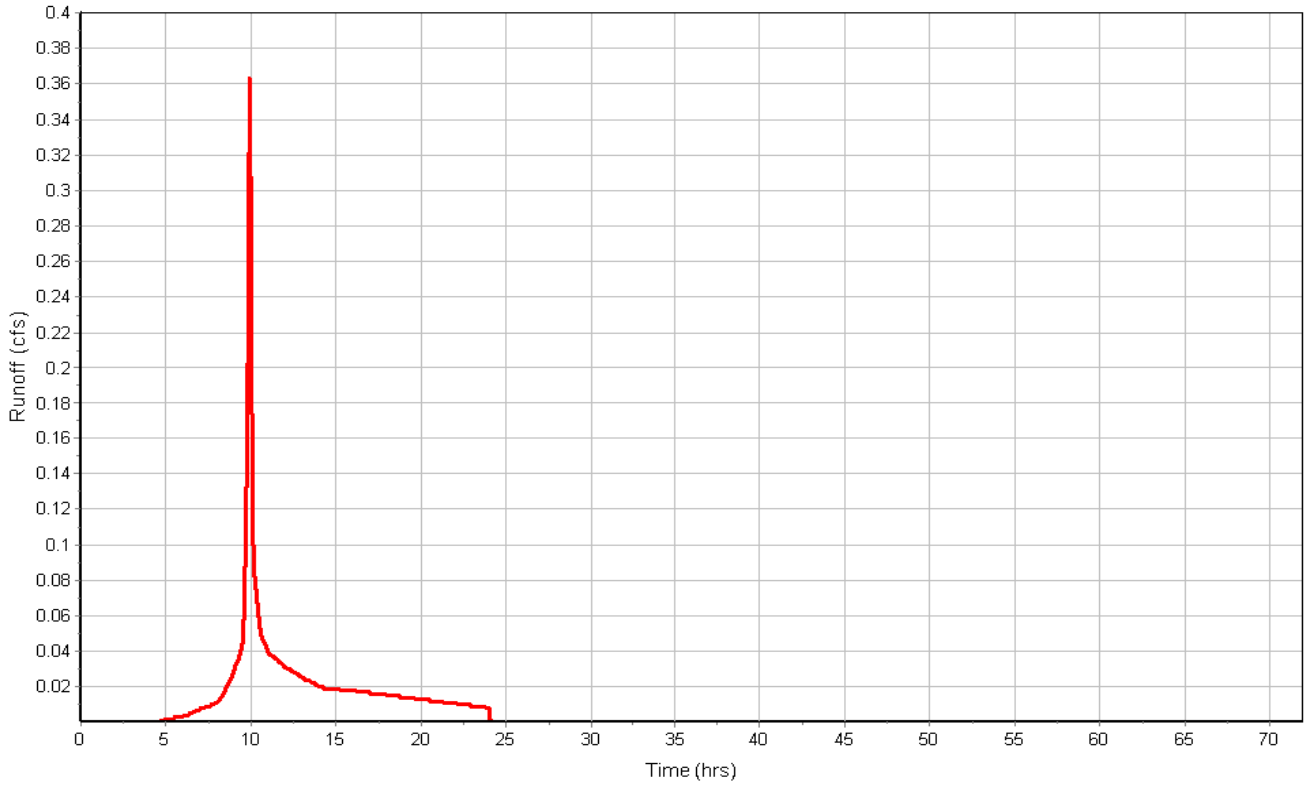


Exhibit A

Subbasin : DMA-2

Input Data

Area (ac) 0.07
Weighted Curve Number 85.34
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	98.00
-	0.04	-	74.00
Composite Area & Weighted CN	0.07		85.34

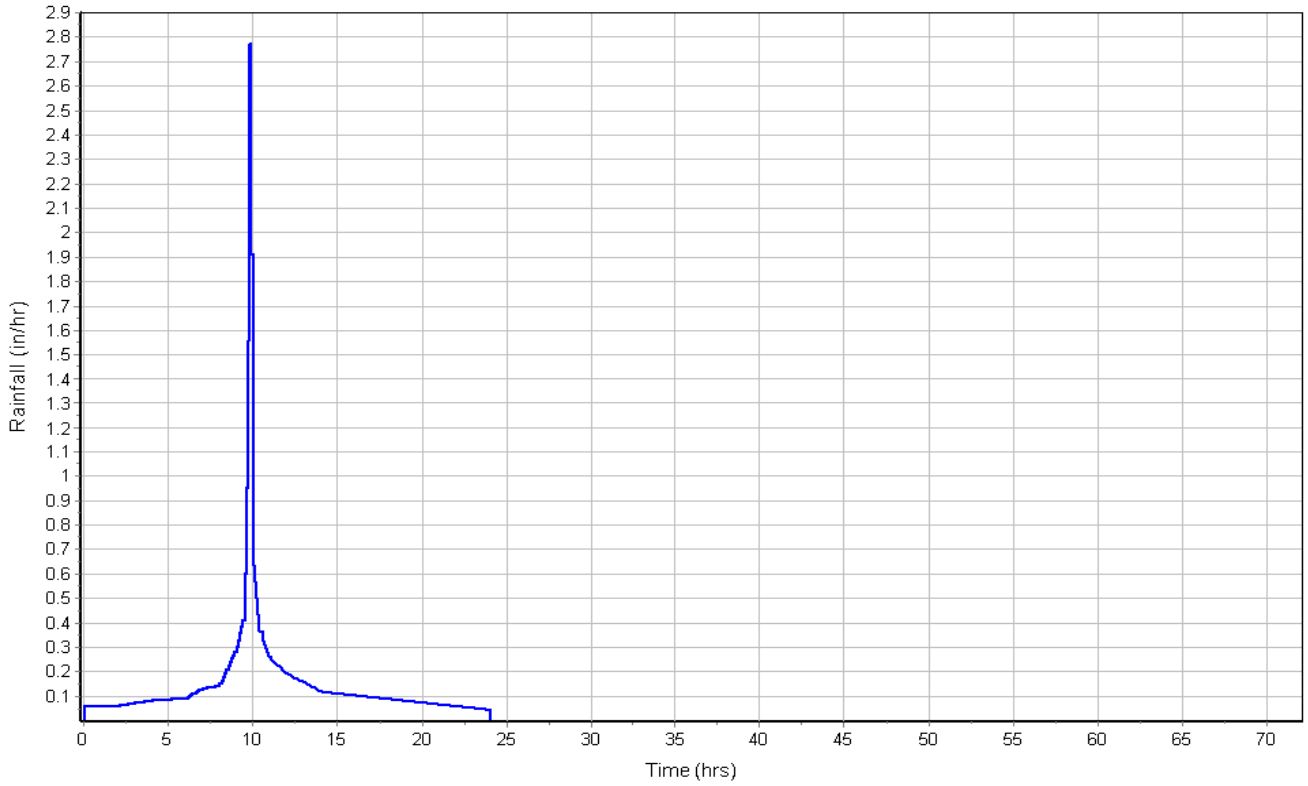
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 2.19
Peak Runoff (cfs) 0.13
Weighted Curve Number 85.34
Time of Concentration (days hh:mm:ss) 0 00:04:08

Exhibit A

Subbasin : DMA-2

Rainfall Intensity Graph



Runoff Hydrograph

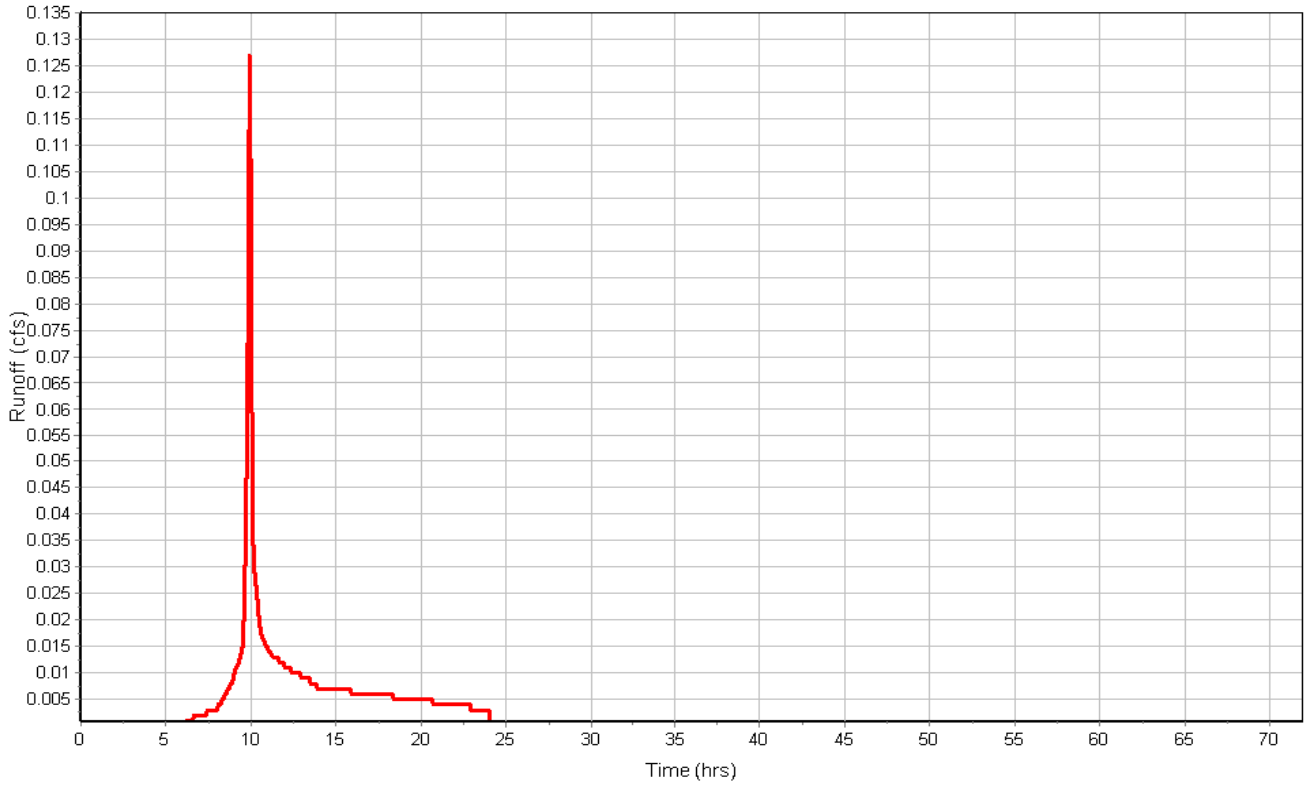


Exhibit A

Subbasin : DMA-3

Input Data

Area (ac) 0.43
Weighted Curve Number 93.85
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.36	-	98.00
-	0.08	-	74.00
Composite Area & Weighted CN	0.44		93.85

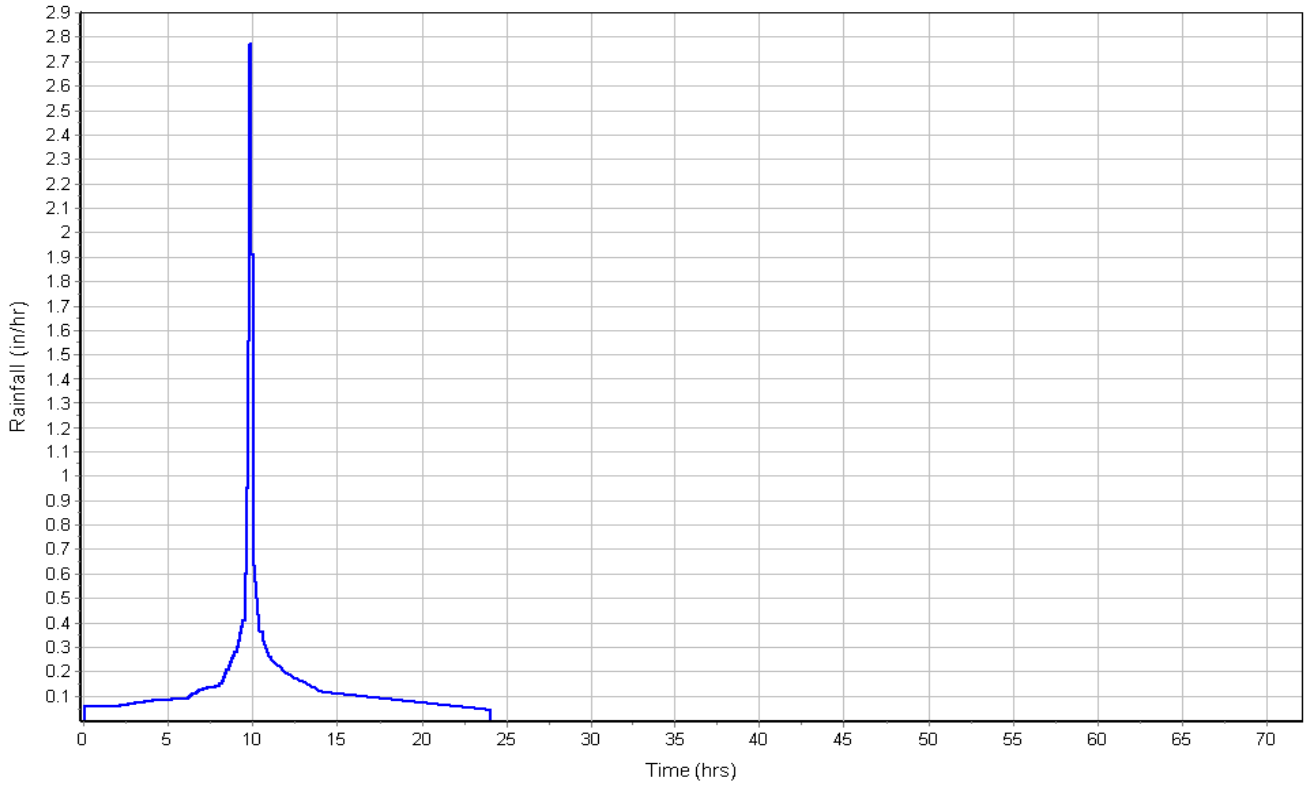
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 3.00
Peak Runoff (cfs) 1.03
Weighted Curve Number 93.85
Time of Concentration (days hh:mm:ss) 0 00:05:46

Exhibit A

Subbasin : DMA-3

Rainfall Intensity Graph



Runoff Hydrograph

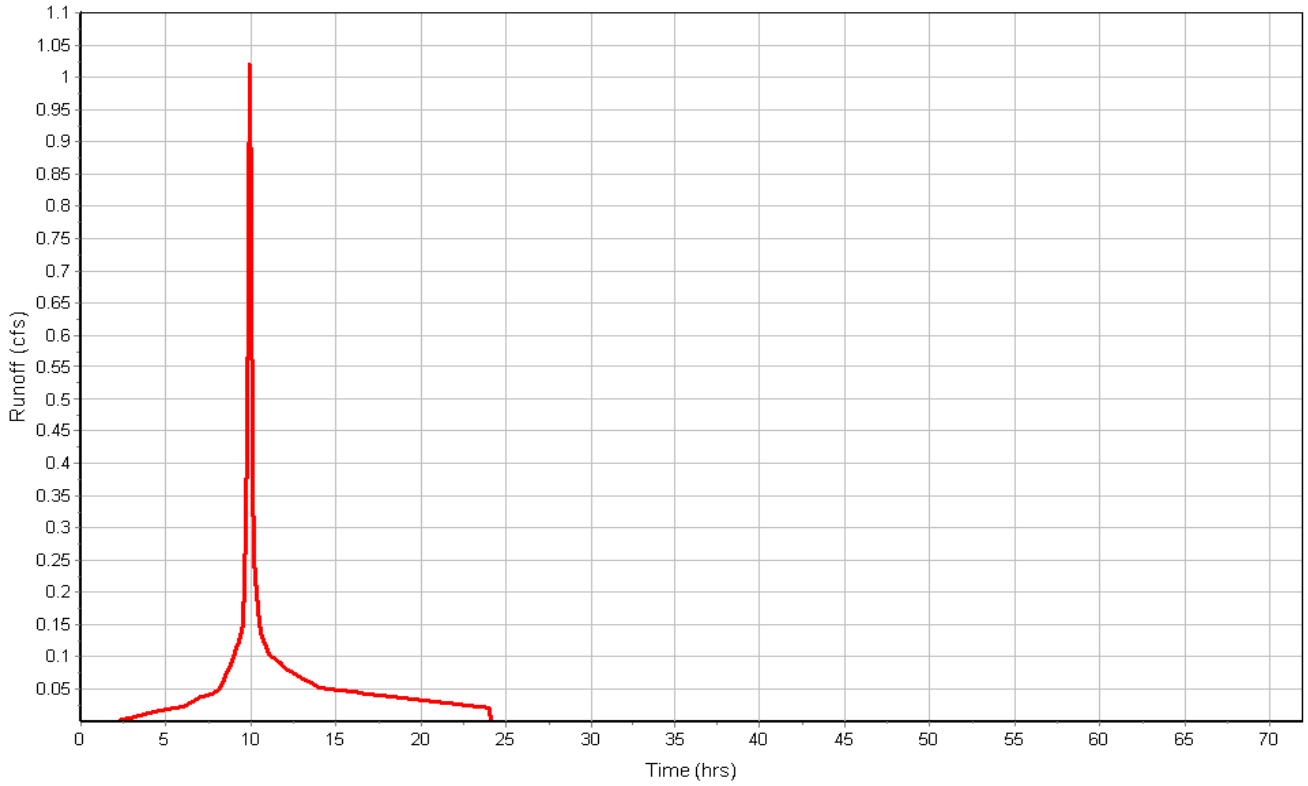


Exhibit A

Subbasin : DMA-4

Input Data

Area (ac) 0.16
Weighted Curve Number 93.39
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.13	-	98.00
-	0.03	-	74.00
Composite Area & Weighted CN	0.16		93.39

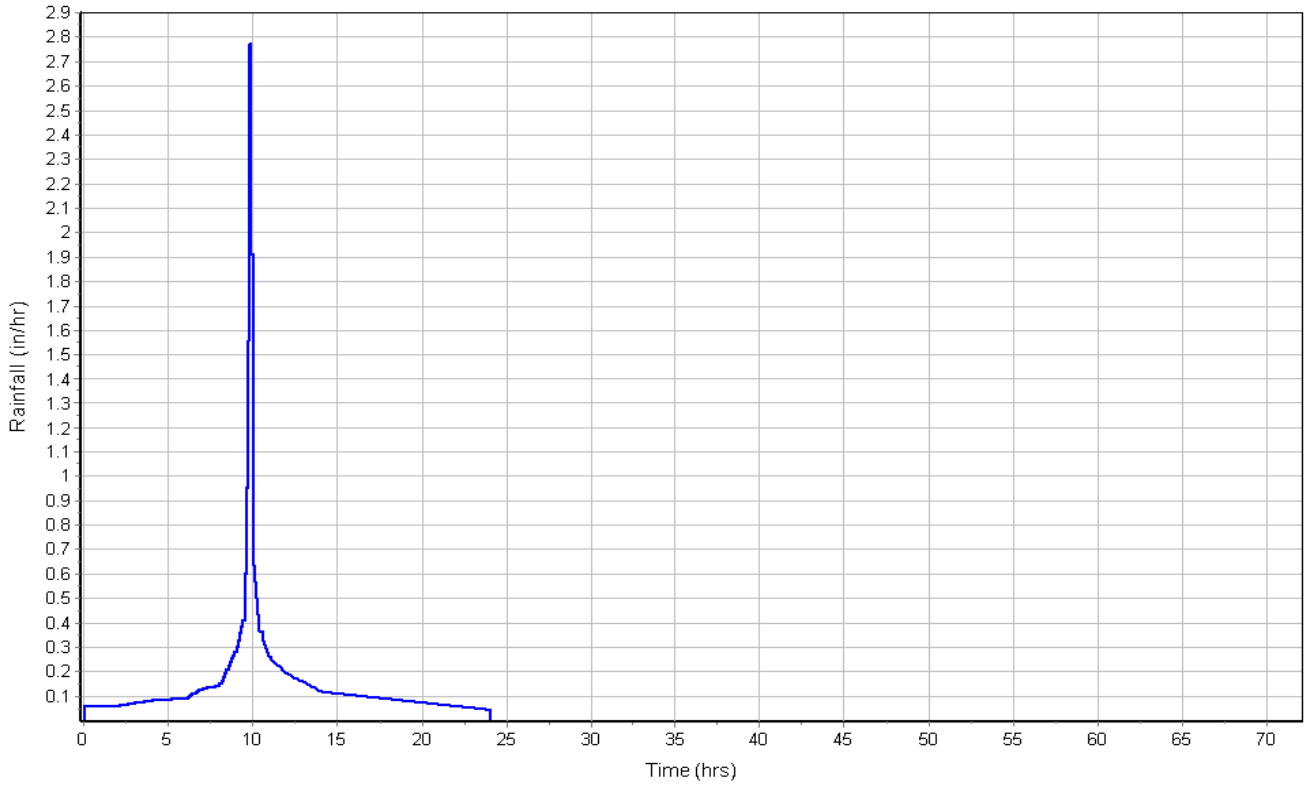
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 2.95
Peak Runoff (cfs) 0.38
Weighted Curve Number 93.39
Time of Concentration (days hh:mm:ss) 0 00:06:17

Exhibit A

Subbasin : DMA-4

Rainfall Intensity Graph



Runoff Hydrograph

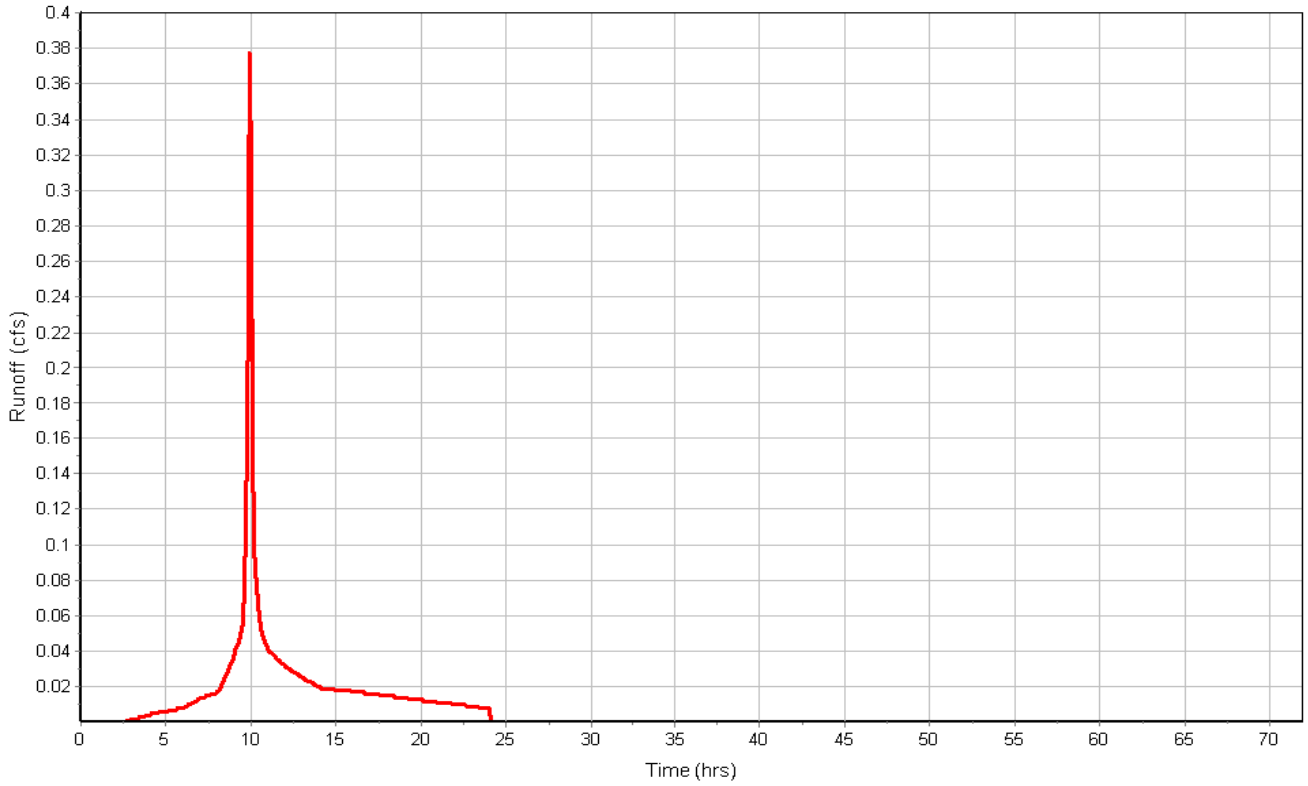


Exhibit A

Subbasin : PRE-SUB

Input Data

Area (ac) 0.85
Weighted Curve Number 91.43
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.62	-	98.00
-	0.23	-	74.00
Composite Area & Weighted CN	0.85		91.43

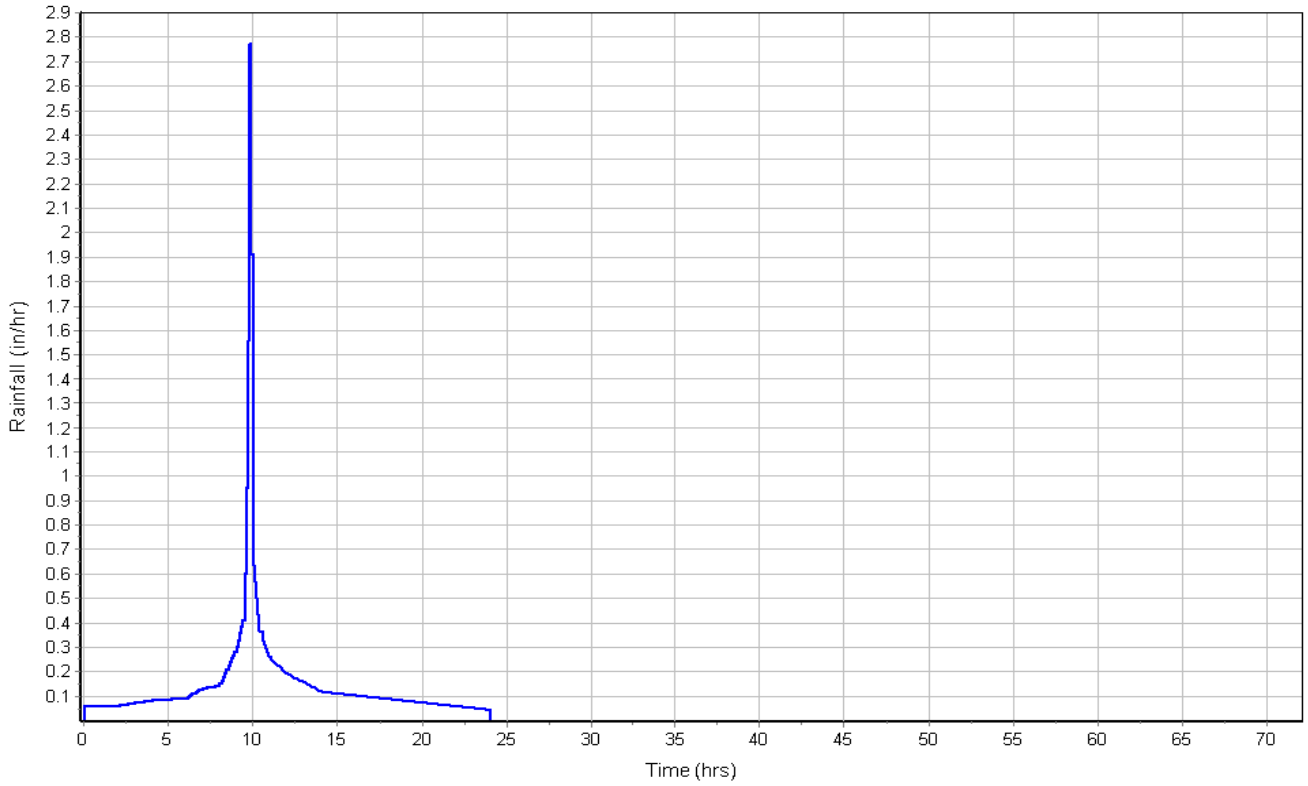
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 2.75
Peak Runoff (cfs) 2.02
Weighted Curve Number 91.43
Time of Concentration (days hh:mm:ss) 0 00:02:18

Exhibit A

Subbasin : PRE-SUB

Rainfall Intensity Graph



Runoff Hydrograph

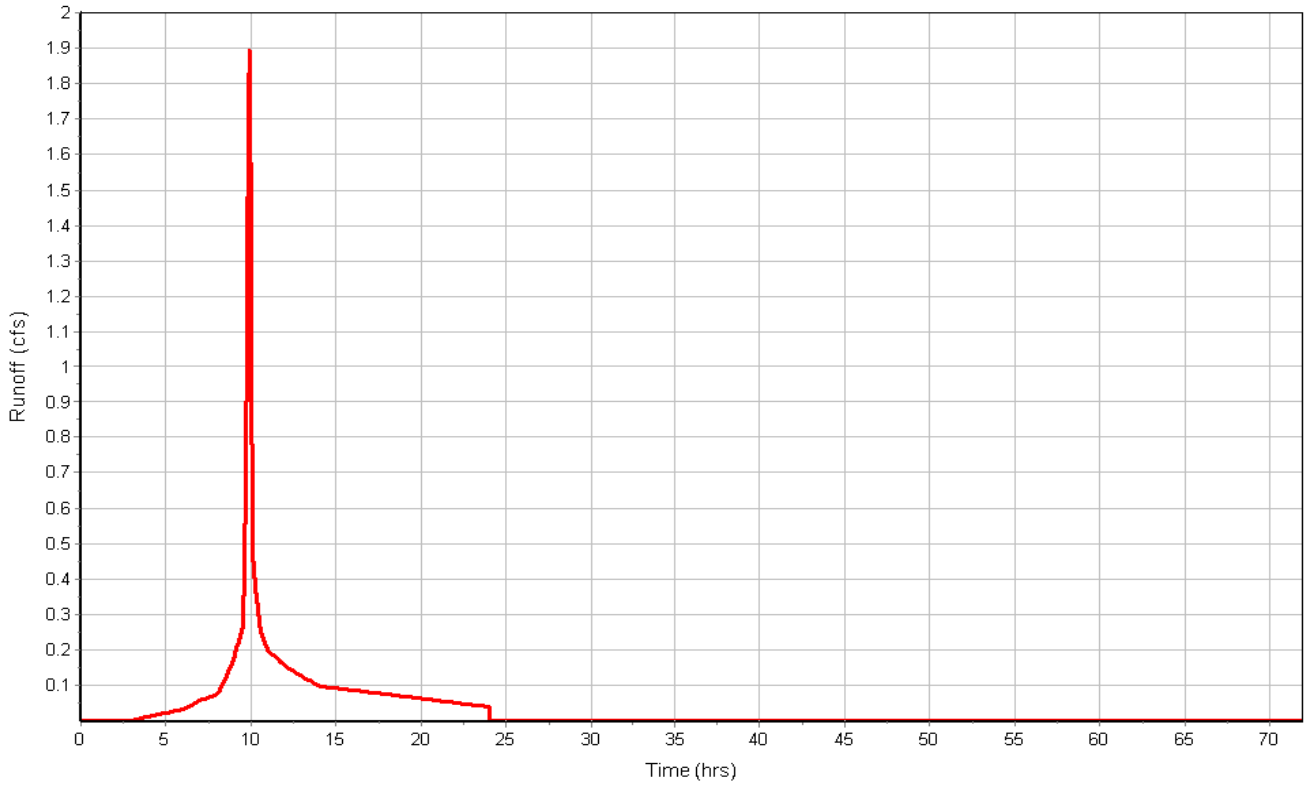


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	DI-2	789.15	794.90	5.75	789.15	0.00	0.00	-794.90	50.00	0.00
2	Jun-04	788.67	794.60	5.93	788.67	0.00	0.00	-794.60	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	DI-2	0.68	0.00	794.59	5.44	0.00	0.31	790.42	1.27	0 10:08	0 00:00	0.00	0.00
2	Jun-04	0.68	0.00	794.34	5.67	0.00	0.26	790.15	1.48	0 10:17	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	Link-11	245.00	789.90	1.23	790.60	0.00	-0.70	-0.2900	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-16	85.00	789.15	0.00	788.67	0.00	0.48	0.5600	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1
3	Link-17	13.00	788.60	0.50	788.67	0.00	-0.07	-0.5400	CIRCULAR	8.040	8.040	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	Link-11	0.51	0 10:18	0.56	0.92	1.47	2.78	0.67	1.00	1205.00		SURCHARGED
2	Link-16	0.68	0 09:57	0.79	0.86	1.94	0.73	0.67	1.00	1596.00		SURCHARGED
3	Link-17	0.34	0 09:56	0.77	0.45	0.99	0.22	0.67	1.00	1733.00		SURCHARGED

Exhibit A

Storage Nodes

Storage Node : DI-4*

Input Data

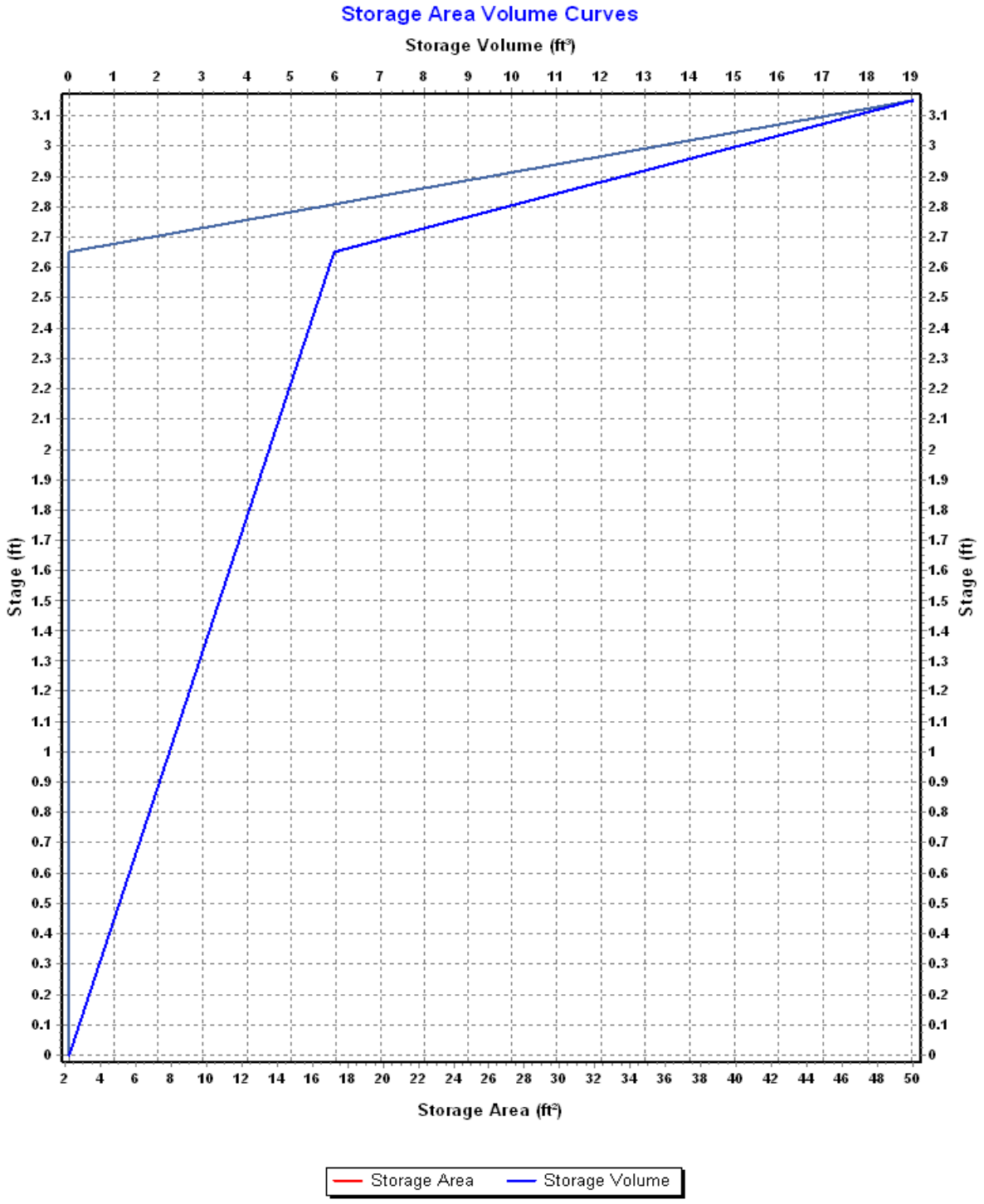
Invert Elevation (ft)	790.60
Max (Rim) Elevation (ft)	794.10
Max (Rim) Offset (ft)	3.50
Initial Water Elevation (ft)	790.60
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Storage Area Volume Curves

Storage Curve : DI-4

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	2.25	0.000
1.33	2.25	2.99
2.65	2.25	5.96
3.15	50	19.02

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19
Stage (ft)
3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : DI-4* (continued)

Outflow Weirs

SN	Element ID	Weir Type	Flap Gate	Crest Elevation (ft)	Crest Offset (ft)	Length (ft)	Weir Total Height (ft)	Discharge Coefficient
1	UNDR	Rectangular	No	793.40	2.80	1.00	0.50	3.33

Output Summary Results

Peak Inflow (cfs)	0.76
Peak Lateral Inflow (cfs)	0.38
Peak Outflow (cfs)	0.76
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	793.77
Max HGL Depth Attained (ft)	3.17
Average HGL Elevation Attained (ft)	791.34
Average HGL Depth Attained (ft)	0.74
Time of Max HGL Occurrence (days hh:mm)	0 10:05
Total Exfiltration Volume (1000-ft³)	0.000
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SUB-1

Input Data

Invert Elevation (ft)	788.10
Max (Rim) Elevation (ft)	795.10
Max (Rim) Offset (ft)	7.00
Initial Water Elevation (ft)	788.10
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : UNDERGROUND

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	74.7600	0.000
0.083	74.7600	6.21
0.167	74.7600	12.49
0.250	74.7600	18.70
0.333	74.7600	24.91
0.417	74.7600	31.19
0.500	74.760	37.40
0.501	165.7740	37.52
0.667	157.1640	64.32
0.750	156.6600	77.34
0.833	156.4080	90.33
0.917	156.1560	103.46
1.000	155.9040	116.41
1.083	155.6520	129.34
1.167	153.6360	142.33
1.250	151.8720	155.01
1.333	151.3680	167.59
1.417	151.1160	180.29
1.500	150.6120	192.81
1.583	150.1080	205.29
1.667	149.3520	217.87
1.750	148.8480	230.25
1.833	146.0760	242.49
1.917	143.0520	254.63
2.000	141.2880	266.43
2.083	138.7680	278.05
2.167	136.2480	289.60
2.250	133.4760	300.79
2.333	130.4520	311.74
2.417	126.9240	322.55
2.500	122.8920	332.92
2.583	118.3560	342.93
2.667	112.5600	352.63
2.750	106.0080	361.70
2.833	95.9280	370.08
2.917	87.6120	377.79
3.000	79.5480	384.73
3.083	74.7600	391.13
3.167	74.7600	397.41
3.250	74.7600	403.62
3.333	74.7600	409.83
3.417	74.7600	416.11
3.500	74.7600	422.32
3.501	84	422.40
5.5	84	590.32
5.501	876	590.80
6	1207	1110.51
6.5	1538	1796.76

Exhibit A

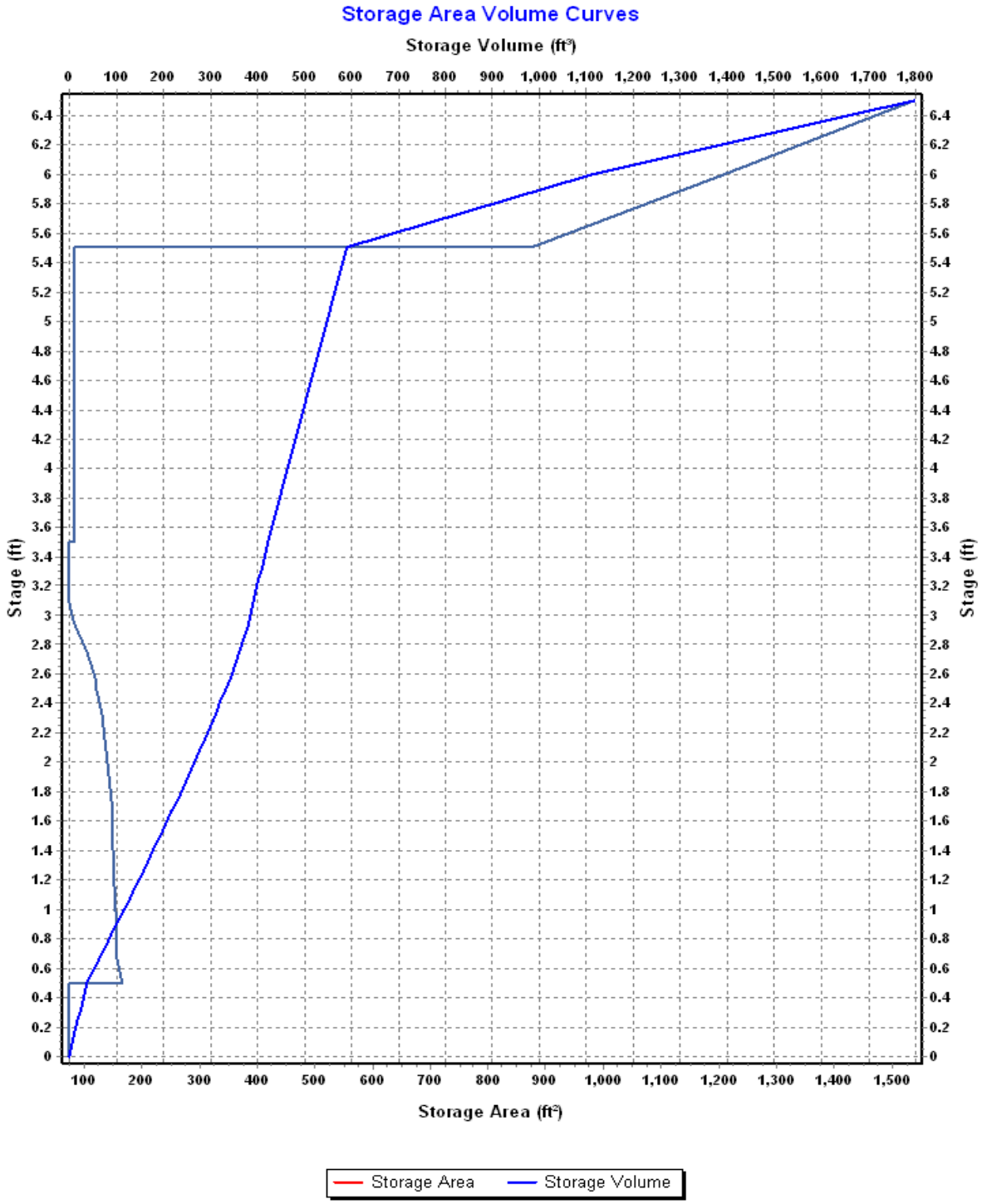

Storage Area Volume Curves
Storage Volume (ft³)
0 100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500 1,600 1,700 1,800
Stage (ft)
6.4 6.2 6 5.8 5.6 5.4 5.2 5 4.8 4.6 4.4 4.2 4 3.8 3.6 3.4 3.2 3 2.8 2.6 2.4 2.2 2 1.8 1.6 1.4 1.2 1 0.8 0.6 0.4 0.2 0
Storage Area (ft²)
100 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1,400 1,500
Storage Area
Storage Volume

Exhibit A

Storage Node : SUB-1 (continued)

Output Summary Results

Peak Inflow (cfs)	0.76
Peak Lateral Inflow (cfs)	0.49
Peak Outflow (cfs)	0.31
Peak Exfiltration Flow Rate (cfm)	0.95
Max HGL Elevation Attained (ft)	794.35
Max HGL Depth Attained (ft)	6.25
Average HGL Elevation Attained (ft)	789.85
Average HGL Depth Attained (ft)	1.75
Time of Max HGL Occurrence (days hh:mm)	0 10:18
Total Exfiltration Volume (1000-ft³)	1.760
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-WST

Input Data

Invert Elevation (ft)	793.90
Max (Rim) Elevation (ft)	794.90
Max (Rim) Offset (ft)	1.00
Initial Water Elevation (ft)	793.90
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

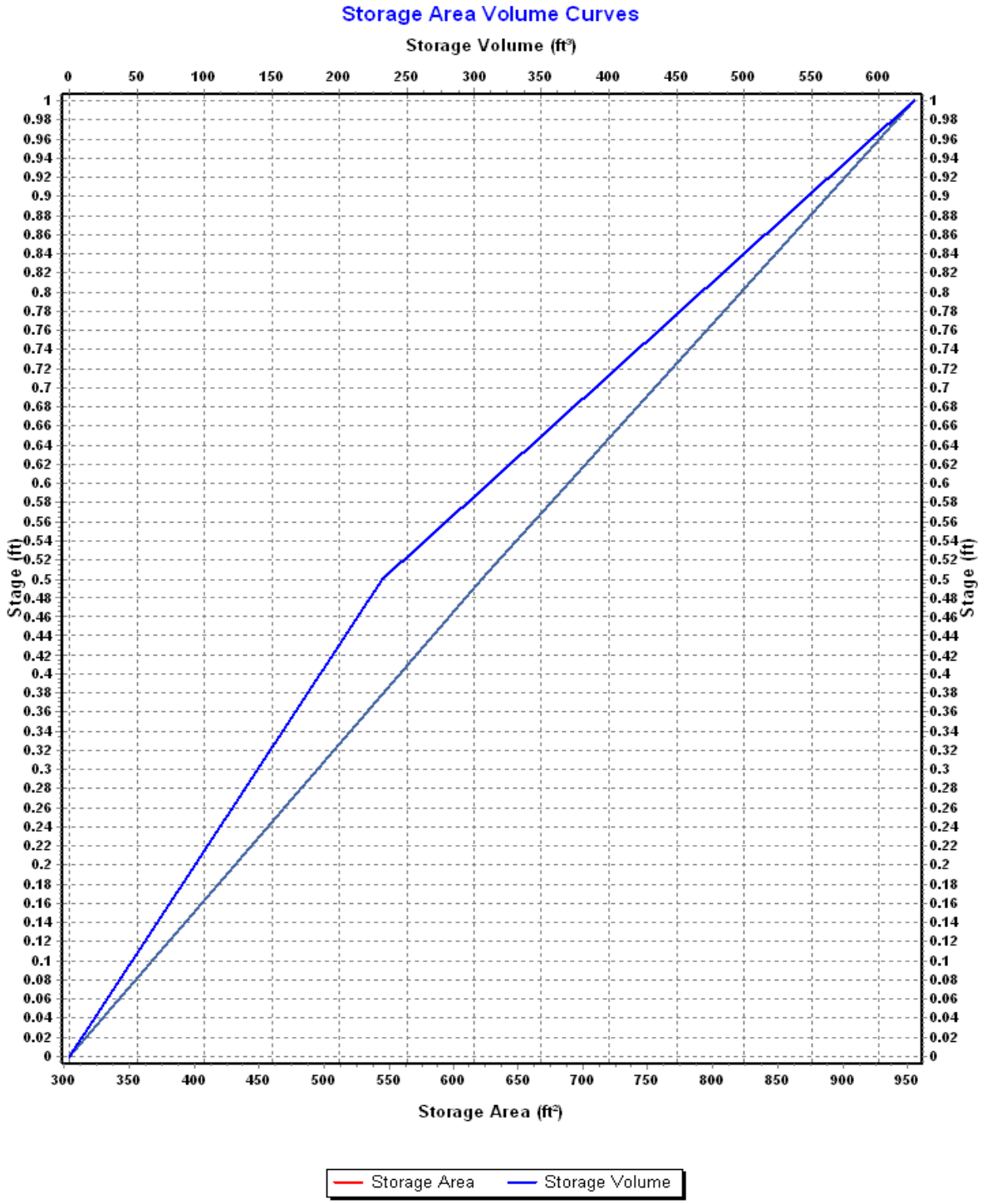
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-1

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	304	0.000
.5	622	231.50
1	955	625.75

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
Stage (ft)
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : SURF-WST (continued)

Outflow Orifices

SN	Element ID	Orifice Type	Orifice Shape	Flap Gate	Circular Orifice Diameter (in)	Rectangular Orifice Height (in)	Rectangular Orifice Width (in)	Orifice Invert Elevation (ft)	Orifice Coefficient
1	Orfice-01	Bottom	Rectangular	No		18.00	18.00	794.40	0.63

Output Summary Results

Peak Inflow (cfs)	1.02
Peak Lateral Inflow (cfs)	1.02
Peak Outflow (cfs)	0.68
Peak Exfiltration Flow Rate (cfm)	0.57
Max HGL Elevation Attained (ft)	794.70
Max HGL Depth Attained (ft)	0.8
Average HGL Elevation Attained (ft)	794.06
Average HGL Depth Attained (ft)	0.16
Time of Max HGL Occurrence (days hh:mm)	0 10:08
Total Exfiltration Volume (1000-ft³)	0.874
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Project Description

File Name ..... 2017-03-23_00136 SWCP - Southern Lot.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	SCS TR-55
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	Jan 01, 2020	00:00:00
End Analysis On	Jan 04, 2020	00:00:00
Start Reporting On	Jan 01, 2020	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	1	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	2
Nodes	5
Junctions	1
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	2
Links	3
Channels	0
Pipes	2
Pumps	0
Orifices	1
Weirs	0
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	95TH	Cumulative	inches	California	San Luis Obispo (Atascadero)	2	1.43	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	POST-SOUTH	0.26	95.28	1.43	0.97	0.25	0.16	0 00:15:09
2	PRE-SOUTH	0.26	94.98	1.43	0.94	0.25	0.16	0 00:15:00

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	Junction	790.16	794.00	790.16	6.00	0.00	0.16	790.31	0.00	3.69	0 00:00	0.00	0.00
2	POST-DEVEL	Outfall	790.10					0.08	790.10					
3	PRE-DEVEL	Outfall	0.00					0.15	0.00					
4	CULTEC	Storage Node	788.46	792.00	788.46		0.00	0.08	789.85				0.00	0.00
5	SURF-SOUTHERN	Storage Node	790.20	794.20	790.20		20.00	0.16	790.46				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	DI	Pipe	SURF-SOUTHERN	BOX	4.00	790.20	790.16	1.0000	6.000	0.0150	0.16	0.49	0.32	2.08	0.20	0.41	0.00	Calculated
2	Link-UG	Pipe	CULTEC	BOX	14.00	790.02	790.16	-1.0000	6.000	0.0150	0.08	0.49	0.16	1.68	0.14	0.28	0.00	Calculated
3	OUT	Orifice	BOX	POST-DEVEL		790.16	790.10		6.000		0.08							

Exhibit A

Subbasin Hydrology

Subbasin : POST-SOUTH

Input Data

Area (ac) 0.26
Weighted Curve Number 95.28
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		95.28

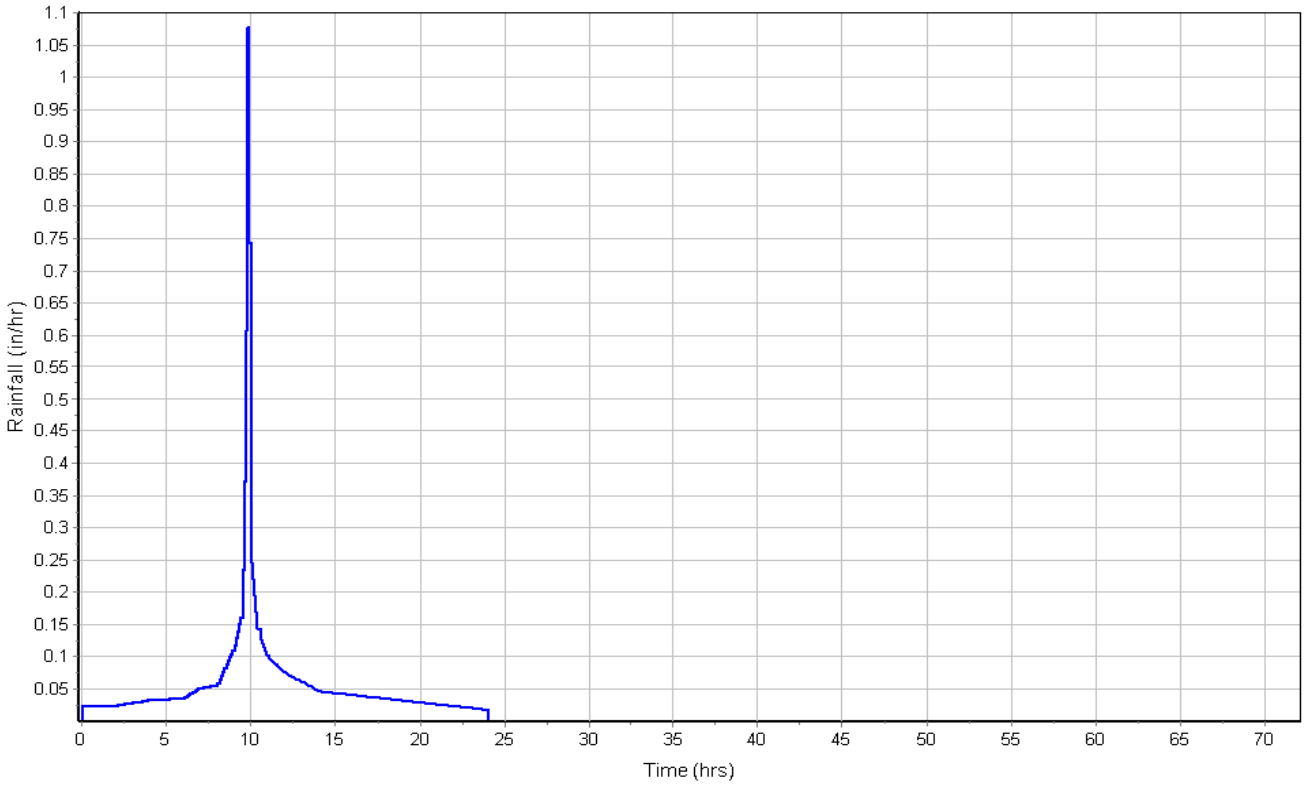
Subbasin Runoff Results

Total Rainfall (in) 1.43
Total Runoff (in) 0.97
Peak Runoff (cfs) 0.16
Weighted Curve Number 95.28
Time of Concentration (days hh:mm:ss) 0 00:15:09

Exhibit A

Subbasin : POST-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

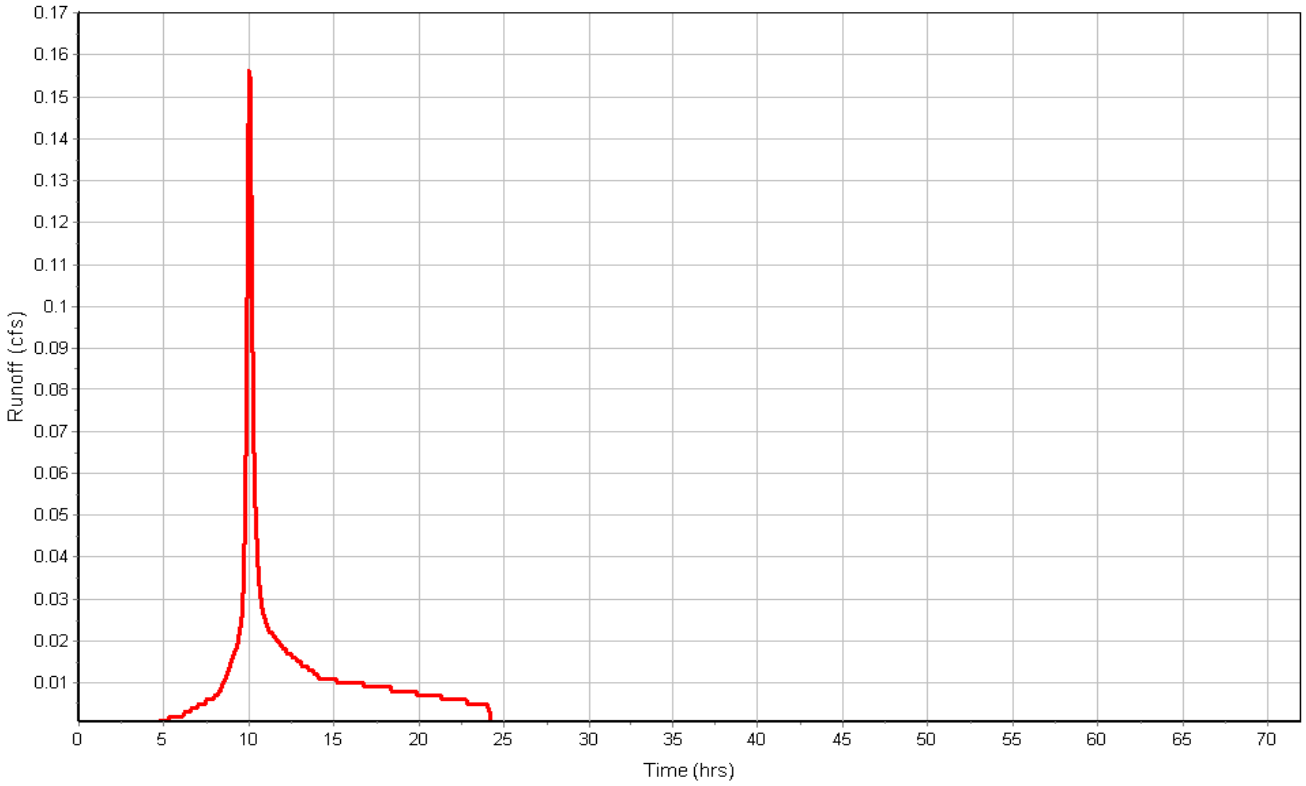


Exhibit A

Subbasin : PRE-SOUTH

Input Data

Area (ac) 0.26
Weighted Curve Number 94.98
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		94.98

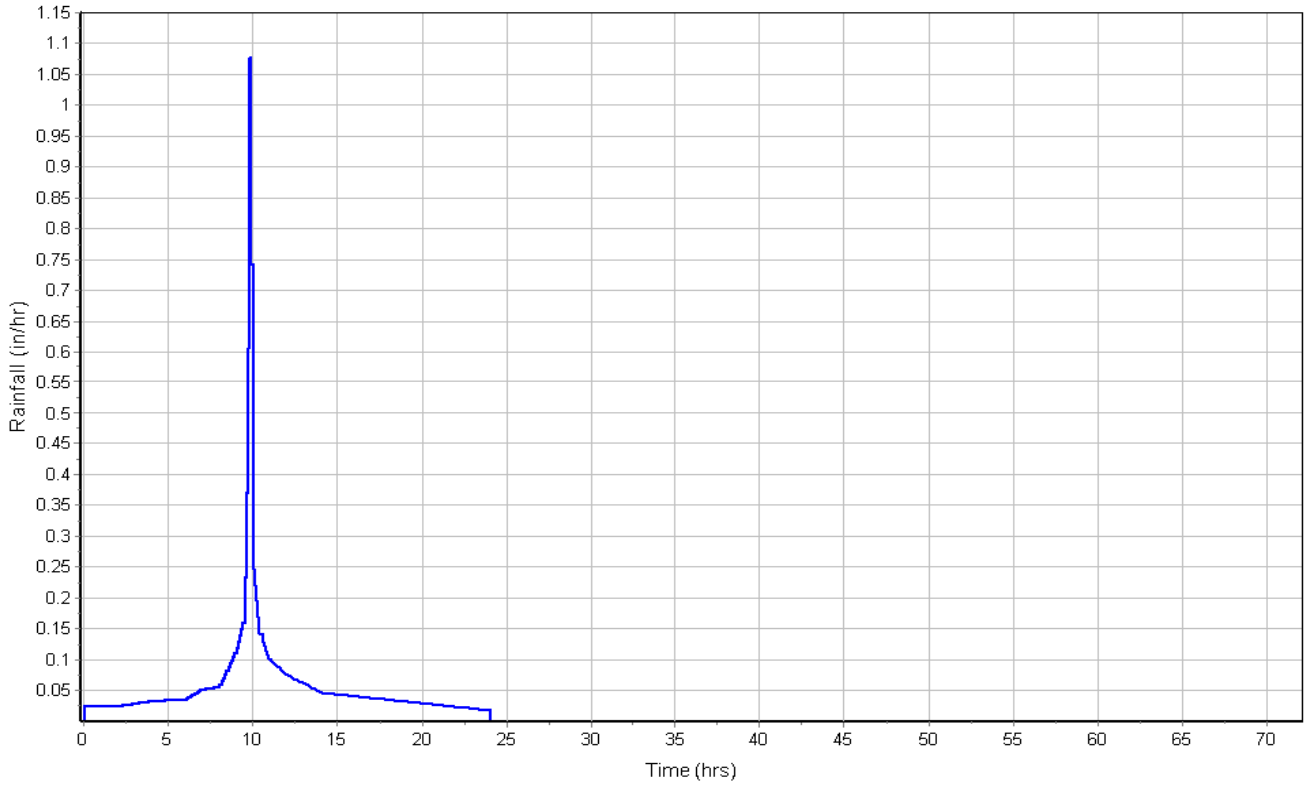
Subbasin Runoff Results

Total Rainfall (in) 1.43
Total Runoff (in) 0.94
Peak Runoff (cfs) 0.16
Weighted Curve Number 94.98
Time of Concentration (days hh:mm:ss) 0 00:15:00

Exhibit A

Subbasin : PRE-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

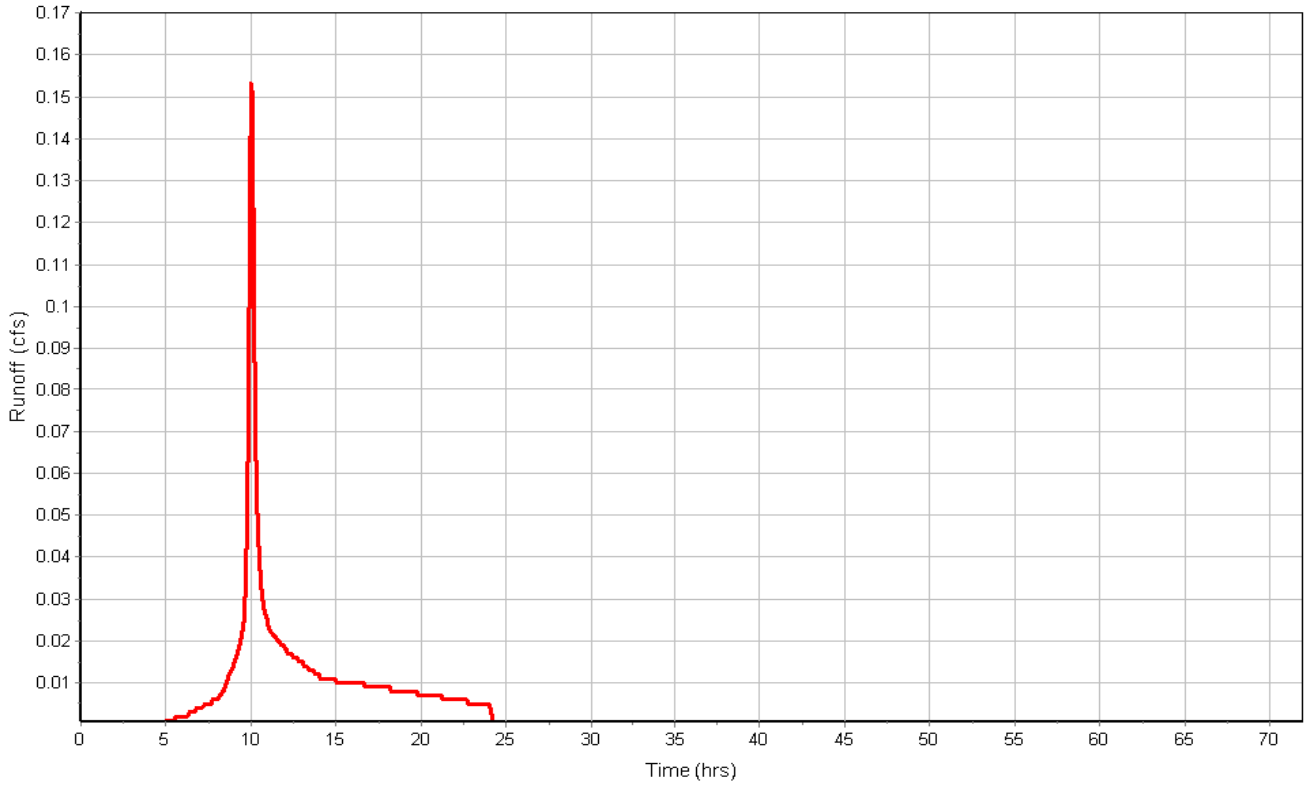


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	BOX	790.16	794.00	3.84	790.16	0.00	6.00	-788.00	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	0.16	0.00	790.31	0.15	0.00	3.69	790.17	0.01	0 10:07	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	DI	4.00	790.20	0.00	790.16	0.00	0.04	1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-UG	14.00	790.02	1.56	790.16	0.00	-0.14	-1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	DI	0.16	0 10:05	0.49	0.32	2.08	0.03	0.20	0.41	0.00		Calculated
2	Link-UG	0.08	0 10:07	0.49	0.16	1.68	0.14	0.14	0.28	0.00		Calculated

Exhibit A

Storage Nodes

Storage Node : CULTEC

Input Data

Invert Elevation (ft)	788.46
Max (Rim) Elevation (ft)	792.00
Max (Rim) Offset (ft)	3.54
Initial Water Elevation (ft)	788.46
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SOUTH-UG

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	89.7120	0.000
0.083	89.7120	7.45
0.167	89.7120	14.99
0.250	89.7120	22.44
0.333	89.7120	29.89
0.417	89.7120	37.43
0.500	89.712	44.88
.501	198.9288	45.02
0.667	188.5968	77.18
0.750	187.9920	92.81
0.833	187.6896	108.40
0.917	187.3872	124.15
1.000	187.0848	139.69
1.083	186.7824	155.21
1.167	184.3632	170.80
1.250	182.2464	186.01
1.333	181.6416	201.11
1.417	181.3392	216.36
1.500	180.7344	231.39
1.583	180.1296	246.37
1.667	179.2224	261.46
1.750	178.6176	276.31
1.833	175.2912	291.00
1.917	171.6624	305.57
2.000	169.5456	319.73
2.083	166.5216	333.68
2.167	163.4976	347.54
2.250	160.1712	360.97
2.333	156.5424	374.11
2.417	152.3088	387.08
2.500	147.4704	399.52
2.583	142.0272	411.53
2.667	135.0720	423.17
2.750	127.2096	434.05
2.833	115.1136	444.11
2.917	105.1344	453.36
3.000	95.4576	461.68
3.083	89.7120	469.36
3.167	89.7120	476.90
3.250	89.7120	484.35
3.333	89.7120	491.80
3.417	89.7120	499.34
3.500	89.7120	506.79

Exhibit A

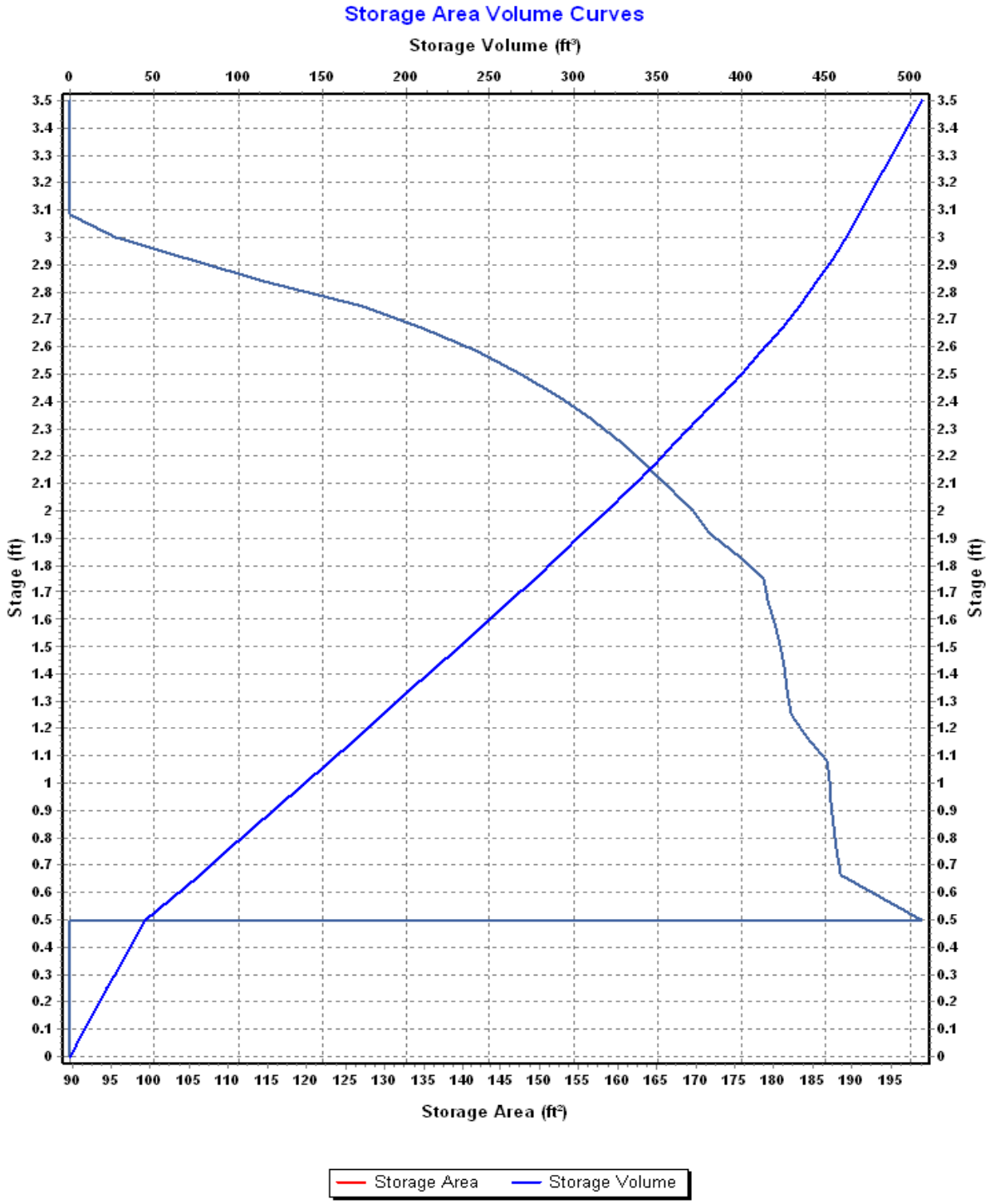

Storage Area Volume Curves
Storage Volume (ft³)
0 50 100 150 200 250 300 350 400 450 500
Stage (ft)
3.5 3.4 3.3 3.2 3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
Stage (ft)
Storage Area (ft²)
90 95 100 105 110 115 120 125 130 135 140 145 150 155 160 165 170 175 180 185 190 195
Storage Area
Storage Volume

Exhibit A

Storage Node : CULTEC (continued)

Output Summary Results

Peak Inflow (cfs)	0.08
Peak Lateral Inflow (cfs)	0.00
Peak Outflow (cfs)	0.00
Peak Exfiltration Flow Rate (cfm)	0.14
Max HGL Elevation Attained (ft)	789.85
Max HGL Depth Attained (ft)	1.39
Average HGL Elevation Attained (ft)	788.98
Average HGL Depth Attained (ft)	0.52
Time of Max HGL Occurrence (days hh:mm)	0 18:34
Total Exfiltration Volume (1000-ft³)	0.333
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-SOUTHERN

Input Data

Invert Elevation (ft)	790.20
Max (Rim) Elevation (ft)	794.20
Max (Rim) Offset (ft)	4.00
Initial Water Elevation (ft)	790.20
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	20.00
Evaporation Loss	0.00

Infiltration/Exfiltration

Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-SOUTH

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	4	0.000
4.2	4	16.80
4.201	65	16.83
4.45	174	46.59
4.7	298	105.59

Exhibit A

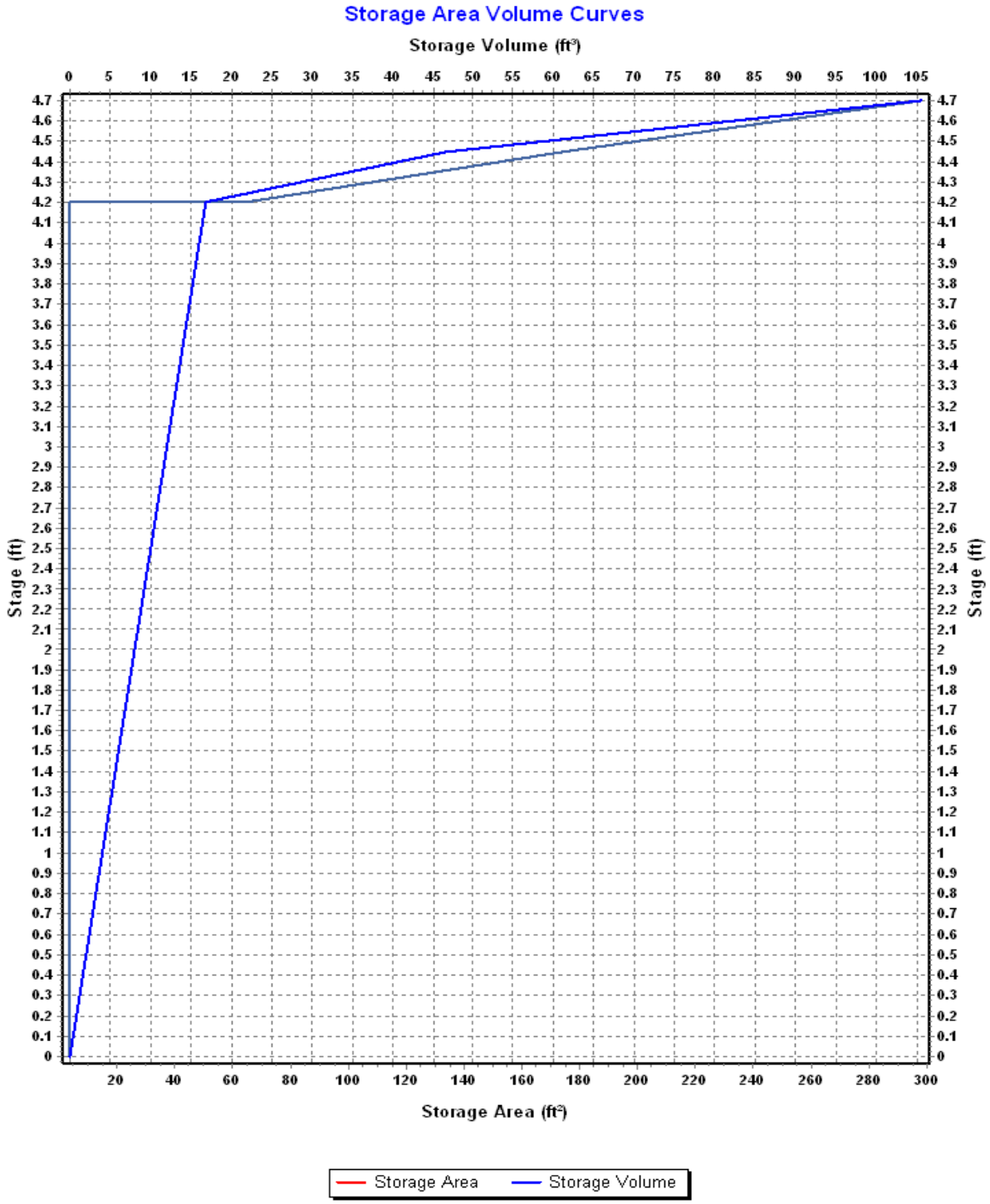

Storage Area Volume Curves
Storage Volume (ft³)
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105
Stage (ft)
0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1 1.1 1.2 1.3 1.4 1.5 1.6 1.7 1.8 1.9 2 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4 4.1 4.2 4.3 4.4 4.5 4.6 4.7
20 40 60 80 100 120 140 160 180 200 220 240 260 280 300
Storage Area (ft²)
Storage Area
Storage Volume

Exhibit A

Storage Node : SURF-SOUTHERN (continued)

Output Summary Results

Peak Inflow (cfs)	0.16
Peak Lateral Inflow (cfs)	0.16
Peak Outflow (cfs)	0.16
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	790.46
Max HGL Depth Attained (ft)	0.26
Average HGL Elevation Attained (ft)	790.22
Average HGL Depth Attained (ft)	0.02
Time of Max HGL Occurrence (days hh:mm)	0 10:05
Total Exfiltration Volume (1000-ft³)	0.003
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Project Description

File Name ..... 2017-03-23_00136 SWCP - Southern Lot.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	SCS TR-55
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	Jan 01, 2020	00:00:00
End Analysis On	Jan 04, 2020	00:00:00
Start Reporting On	Jan 01, 2020	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	1	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	2
Nodes	5
Junctions	1
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	2
Links	3
Channels	0
Pipes	2
Pumps	0
Orifices	1
Weirs	0
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	2-YEAR	Cumulative	inches	California	San Luis Obispo (Paso Robles)	2	2.11	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	POST-SOUTH	0.26	95.28	2.11	1.61	0.42	0.35	0 00:03:54
2	PRE-SOUTH	0.26	94.98	2.11	1.59	0.41	0.35	0 00:03:54

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	Junction	790.16	794.00	790.16	6.00	0.00	0.35	790.40	0.00	3.60	0 00:00	0.00	0.00
2	POST-DEVEL	Outfall	790.10					0.17	790.10					
3	PRE-DEVEL	Outfall	0.00					0.34	0.00					
4	CULTEC	Storage Node	788.46	792.00	788.46		0.00	0.18	790.27				0.00	0.00
5	SURF-SOUTHERN	Storage Node	790.20	794.20	790.20		20.00	0.35	790.62				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	DI	Pipe	SURF-SOUTHERN	BOX	4.00	790.20	790.16	1.0000	6.000	0.0150	0.35	0.49	0.72	2.51	0.33	0.67	0.00	Calculated
2	Link-UG	Pipe	CULTEC	BOX	14.00	790.02	790.16	-1.0000	6.000	0.0150	0.18	0.49	0.36	2.04	0.22	0.45	0.00	Calculated
3	OUT	Orifice	BOX	POST-DEVEL		790.16	790.10		6.000		0.17							

Exhibit A

Subbasin Hydrology

Subbasin : POST-SOUTH

Input Data

Area (ac)	0.26
Weighted Curve Number	95.28
Rain Gage ID	NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		95.28

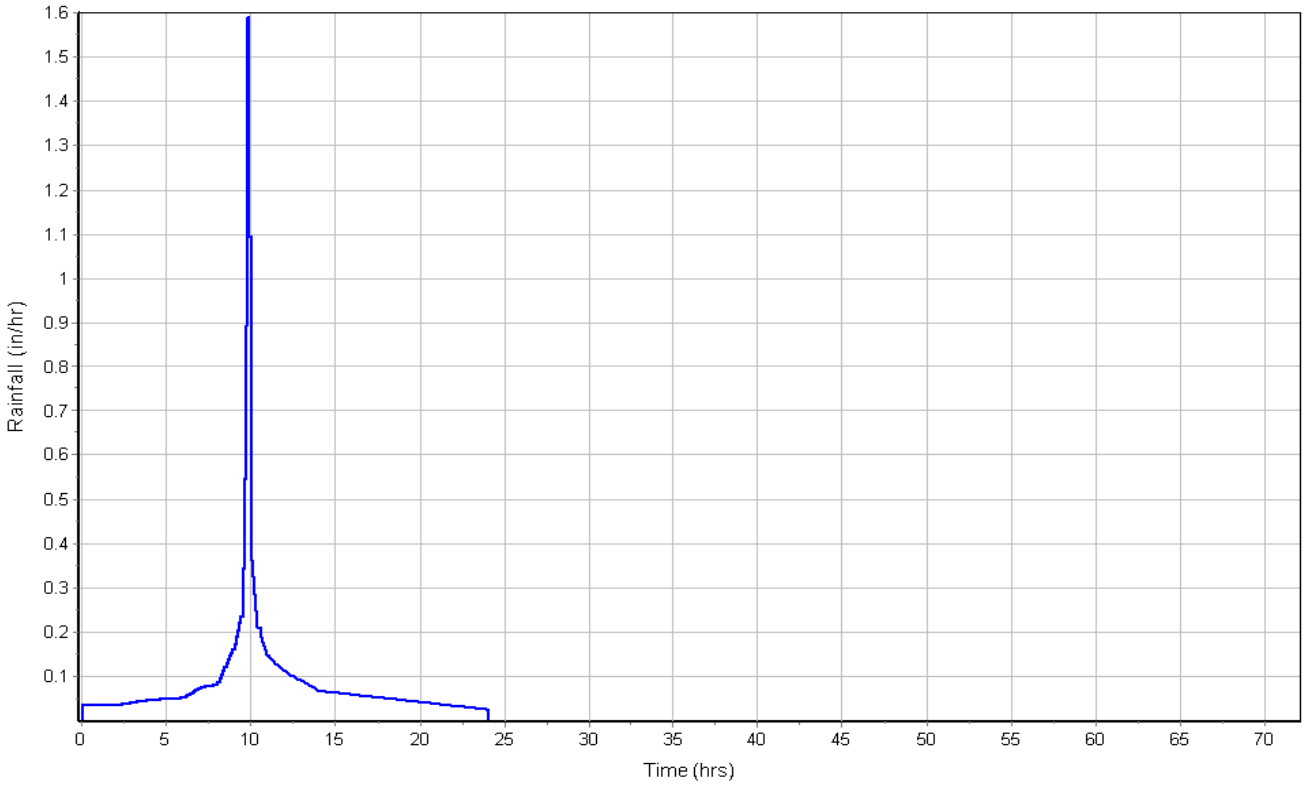
Subbasin Runoff Results

Total Rainfall (in)	2.11
Total Runoff (in)	1.61
Peak Runoff (cfs)	0.35
Weighted Curve Number	95.28
Time of Concentration (days hh:mm:ss)	0 00:03:54

Exhibit A

Subbasin : POST-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

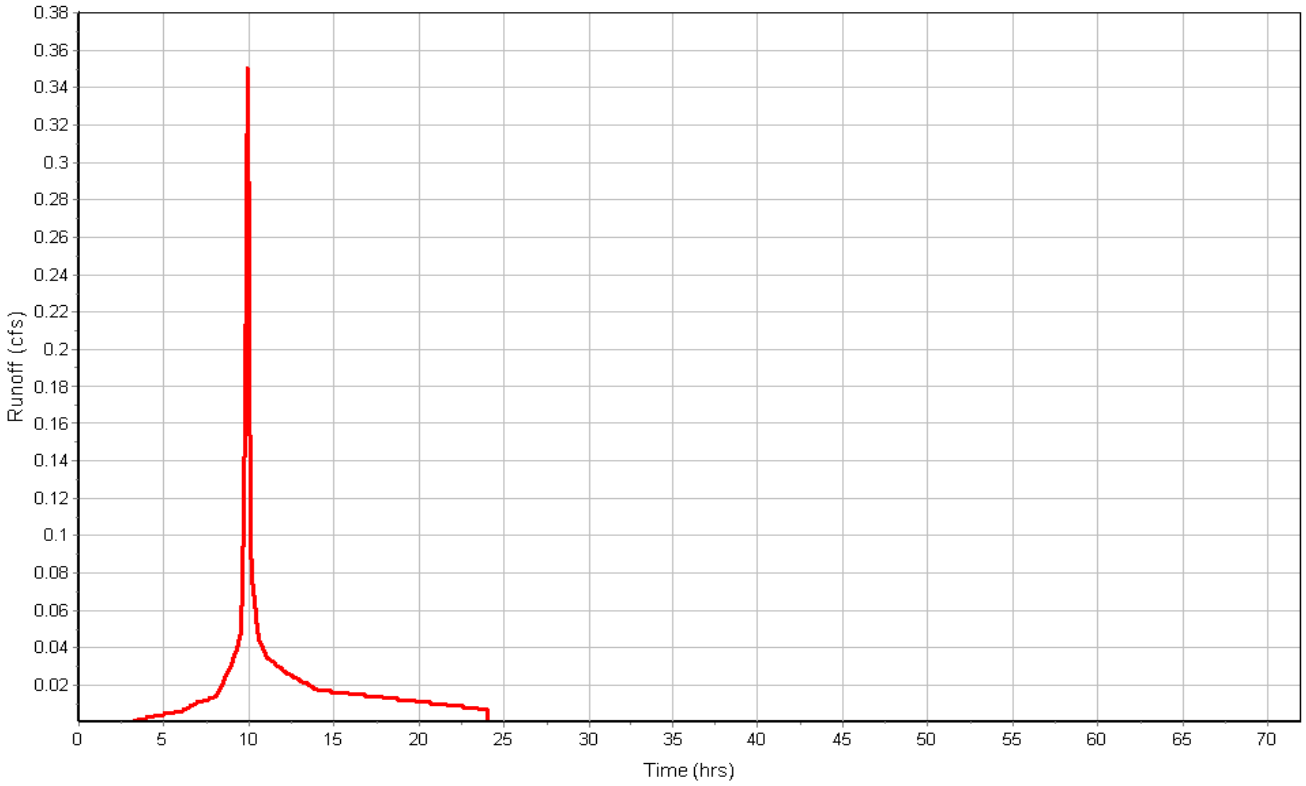


Exhibit A

Subbasin : PRE-SOUTH

Input Data

Area (ac) 0.26
Weighted Curve Number 94.98
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		94.98

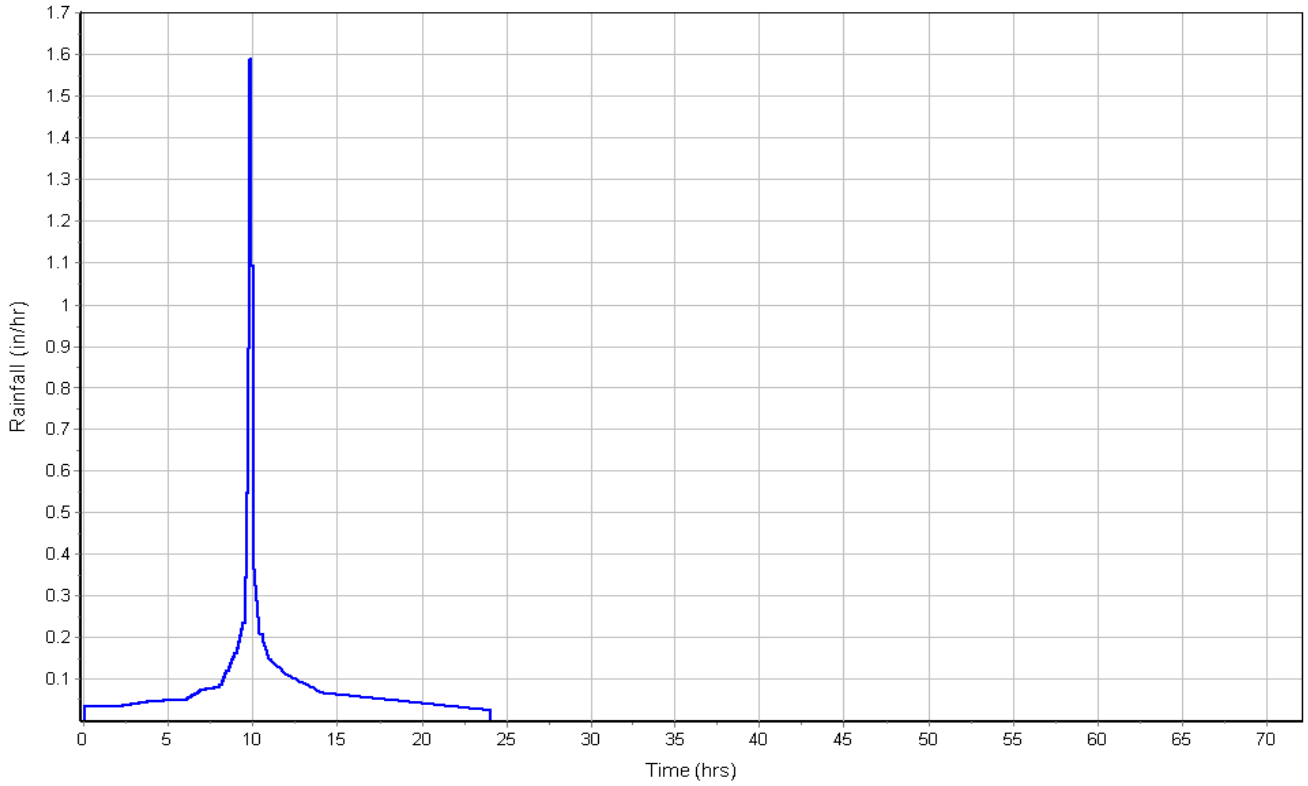
Subbasin Runoff Results

Total Rainfall (in) 2.11
Total Runoff (in) 1.59
Peak Runoff (cfs) 0.35
Weighted Curve Number 94.98
Time of Concentration (days hh:mm:ss) 0 00:03:54

Exhibit A

Subbasin : PRE-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

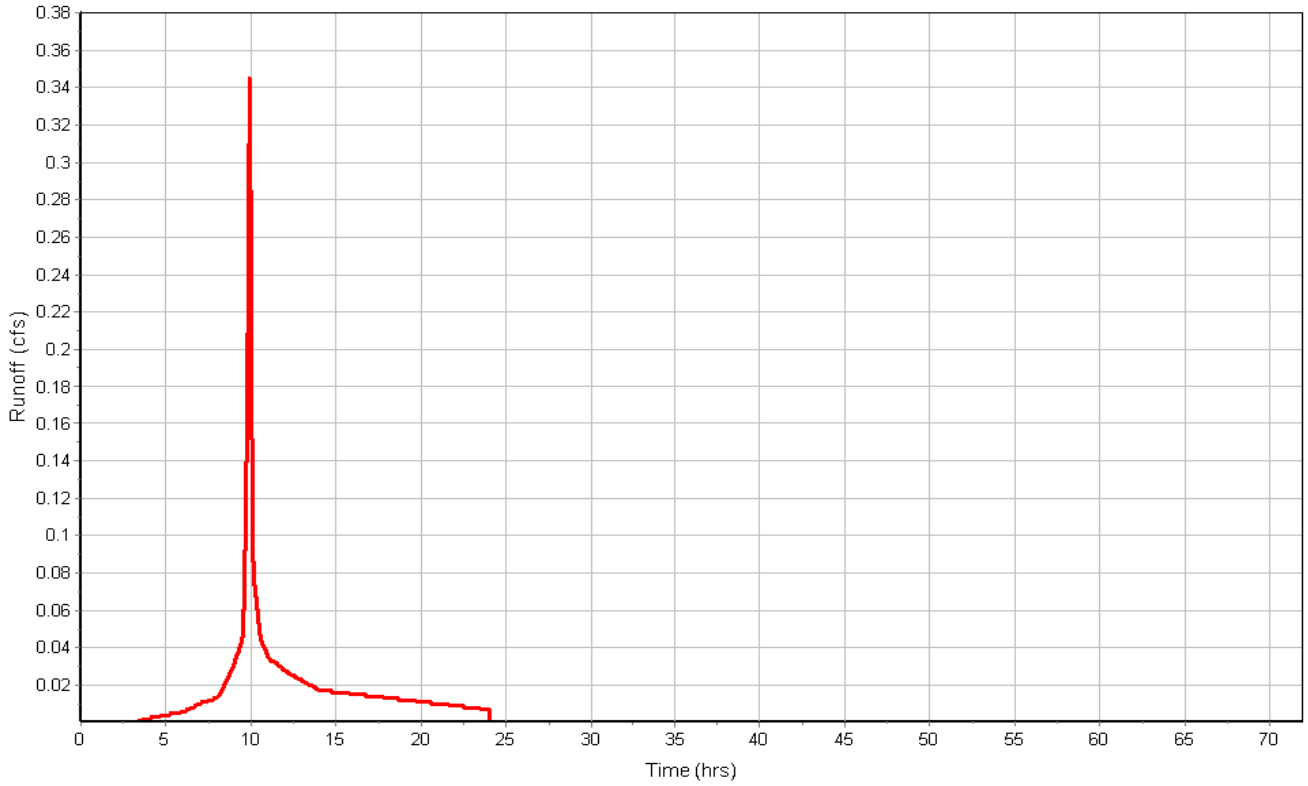


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	BOX	790.16	794.00	3.84	790.16	0.00	6.00	-788.00	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	0.35	0.00	790.40	0.24	0.00	3.60	790.17	0.01	0 10:00	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	DI	4.00	790.20	0.00	790.16	0.00	0.04	1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-UG	14.00	790.02	1.56	790.16	0.00	-0.14	-1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	DI	0.35	0 10:00	0.49	0.72	2.51	0.03	0.33	0.67	0.00		Calculated
2	Link-UG	0.18	0 10:00	0.49	0.36	2.04	0.11	0.22	0.45	0.00		Calculated

Exhibit A

Storage Nodes

Storage Node : CULTEC

Input Data

Invert Elevation (ft)	788.46
Max (Rim) Elevation (ft)	792.00
Max (Rim) Offset (ft)	3.54
Initial Water Elevation (ft)	788.46
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

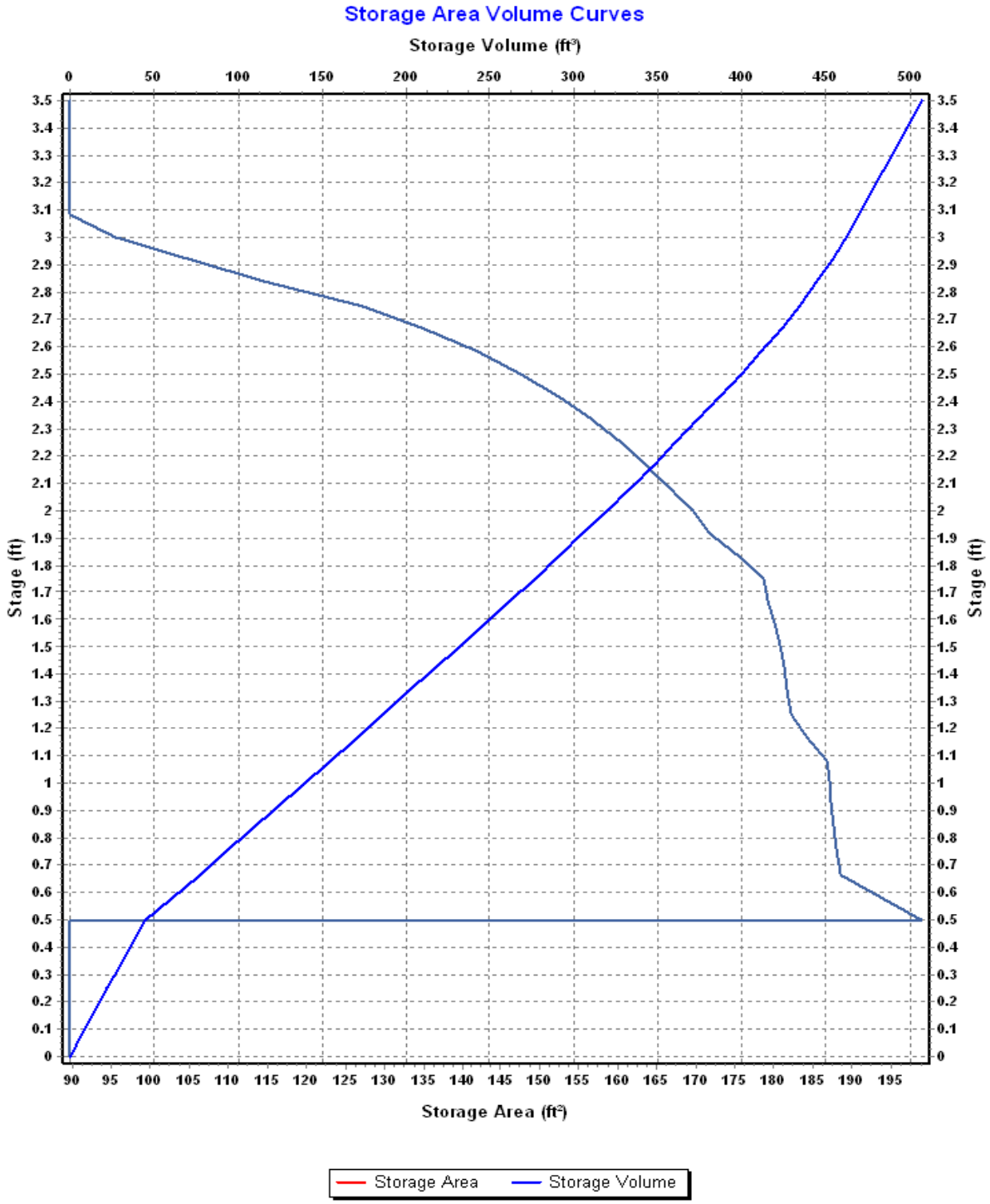
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SOUTH-UG

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	89.7120	0.000
0.083	89.7120	7.45
0.167	89.7120	14.99
0.250	89.7120	22.44
0.333	89.7120	29.89
0.417	89.7120	37.43
0.500	89.712	44.88
.501	198.9288	45.02
0.667	188.5968	77.18
0.750	187.9920	92.81
0.833	187.6896	108.40
0.917	187.3872	124.15
1.000	187.0848	139.69
1.083	186.7824	155.21
1.167	184.3632	170.80
1.250	182.2464	186.01
1.333	181.6416	201.11
1.417	181.3392	216.36
1.500	180.7344	231.39
1.583	180.1296	246.37
1.667	179.2224	261.46
1.750	178.6176	276.31
1.833	175.2912	291.00
1.917	171.6624	305.57
2.000	169.5456	319.73
2.083	166.5216	333.68
2.167	163.4976	347.54
2.250	160.1712	360.97
2.333	156.5424	374.11
2.417	152.3088	387.08
2.500	147.4704	399.52
2.583	142.0272	411.53
2.667	135.0720	423.17
2.750	127.2096	434.05
2.833	115.1136	444.11
2.917	105.1344	453.36
3.000	95.4576	461.68
3.083	89.7120	469.36
3.167	89.7120	476.90
3.250	89.7120	484.35
3.333	89.7120	491.80
3.417	89.7120	499.34
3.500	89.7120	506.79

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 50 100 150 200 250 300 350 400 450 500
Stage (ft)
3.5 3.4 3.3 3.2 3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
Stage (ft)
Storage Area (ft²)
90 95 100 105 110 115 120 125 130 135 140 145 150 155 160 165 170 175 180 185 190 195
Storage Area
Storage Volume

Storage Node : CULTEC (continued)

Output Summary Results

Peak Inflow (cfs)	0.18
Peak Lateral Inflow (cfs)	0.00
Peak Outflow (cfs)	0.00
Peak Exfiltration Flow Rate (cfm)	0.14
Max HGL Elevation Attained (ft)	790.27
Max HGL Depth Attained (ft)	1.81
Average HGL Elevation Attained (ft)	789.25
Average HGL Depth Attained (ft)	0.79
Time of Max HGL Occurrence (days hh:mm)	0 10:35
Total Exfiltration Volume (1000-ft³)	0.413
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-SOUTHERN

Input Data

Invert Elevation (ft)	790.20
Max (Rim) Elevation (ft)	794.20
Max (Rim) Offset (ft)	4.00
Initial Water Elevation (ft)	790.20
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	20.00
Evaporation Loss	0.00

Infiltration/Exfiltration

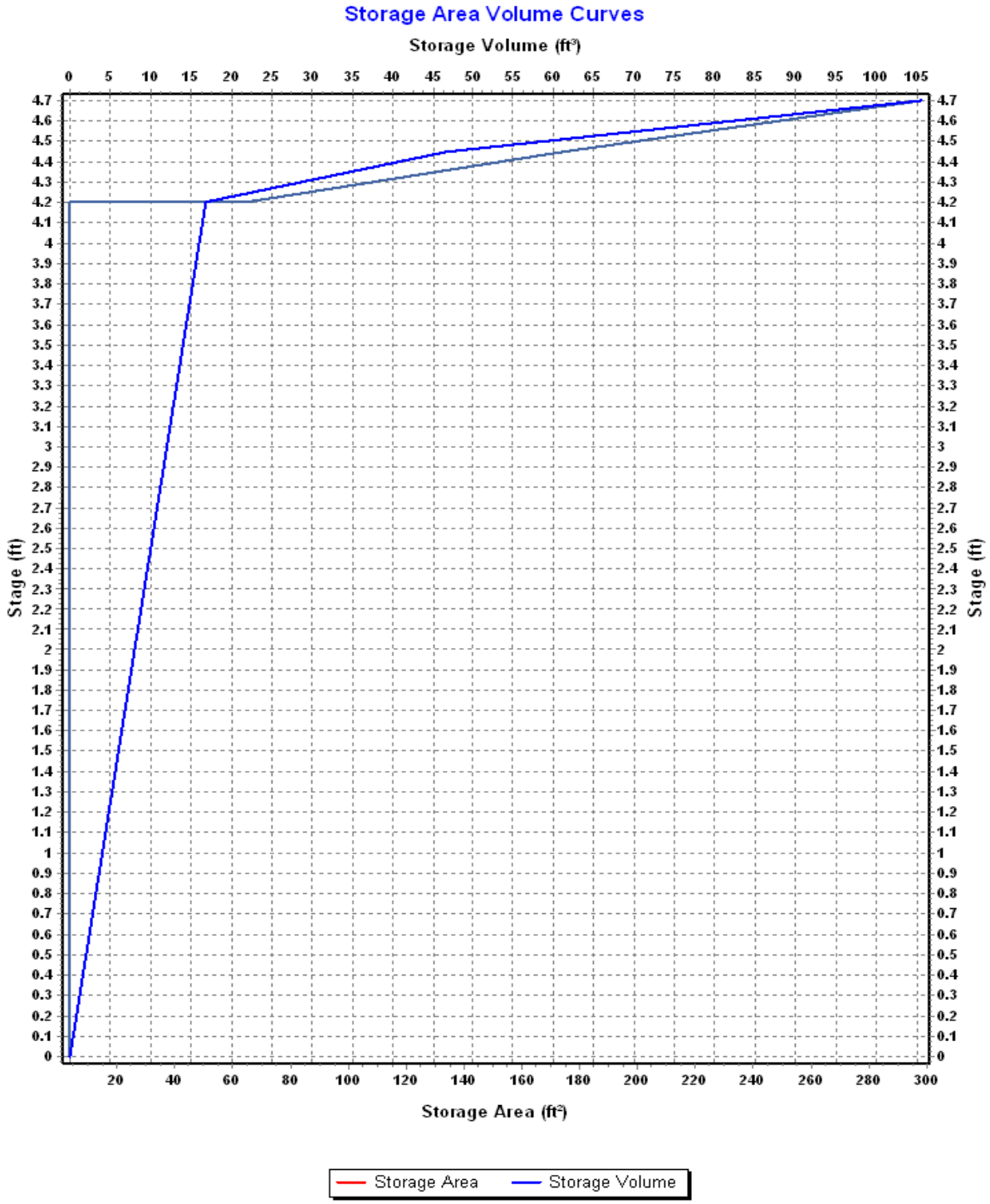
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-SOUTH

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	4	0.000
4.2	4	16.80
4.201	65	16.83
4.45	174	46.59
4.7	298	105.59

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105
Stage (ft)
0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1 1.1 1.2 1.3 1.4 1.5 1.6 1.7 1.8 1.9 2 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4 4.1 4.2 4.3 4.4 4.5 4.6 4.7
20 40 60 80 100 120 140 160 180 200 220 240 260 280 300
Storage Area (ft²)
Storage Area
Storage Volume

Storage Node : SURF-SOUTHERN (continued)

Output Summary Results

Peak Inflow (cfs)	0.35
Peak Lateral Inflow (cfs)	0.35
Peak Outflow (cfs)	0.35
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	790.62
Max HGL Depth Attained (ft)	0.42
Average HGL Elevation Attained (ft)	790.22
Average HGL Depth Attained (ft)	0.02
Time of Max HGL Occurrence (days hh:mm)	0 10:00
Total Exfiltration Volume (1000-ft³)	0.004
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Project Description

File Name ..... 2017-03-23_00136 SWCP - Southern Lot.SPF

Project Options

Flow Units ..... CFS
Elevation Type ..... Elevation
Hydrology Method ..... SCS TR-55
Time of Concentration (TOC) Method ..... User-Defined
Link Routing Method ..... Hydrodynamic
Enable Overflow Ponding at Nodes ..... YES
Skip Steady State Analysis Time Periods ... YES

Analysis Options

Start Analysis On ..... Jan 01, 2020 00:00:00
End Analysis On ..... Jan 04, 2020 00:00:00
Start Reporting On ..... Jan 01, 2020 00:00:00
Antecedent Dry Days ..... 0 days
Runoff (Dry Weather) Time Step ..... 0 01:00:00 days hh:mm:ss
Runoff (Wet Weather) Time Step ..... 0 00:05:00 days hh:mm:ss
Reporting Time Step ..... 0 00:05:00 days hh:mm:ss
Routing Time Step ..... 1 seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	2
Nodes	5
Junctions	1
Outfalls	2
Flow Diversions	0
Inlets	0
Storage Nodes	2
Links	3
Channels	0
Pipes	2
Pumps	0
Orifices	1
Weirs	0
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN	Rain Gage ID	Data Source	Data Source ID	Rainfall Type	Rain Units	State	County	Return Period (years)	Rainfall Depth (inches)	Rainfall Distribution
1		Time Series	10-YEAR	Cumulative	inches	California	San Luis Obispo (Paso Robles)	10	3.68	SCS Type I 24-hr

Exhibit A

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Curve Number	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	POST-SOUTH	0.26	95.28	3.68	3.15	0.82	0.67	0 00:03:54
2	PRE-SOUTH	0.26	94.98	3.68	3.11	0.81	0.67	0 00:03:54

Exhibit A

Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft²)	Peak Inflow (cfs)	Max HGL Elevation Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	Junction	790.16	794.00	790.16	6.00	0.00	0.66	790.76	0.00	3.24	0 00:00	0.00	0.00
2	POST-DEVEL	Outfall	790.10					0.57	790.10					
3	PRE-DEVEL	Outfall	0.00					0.66	0.00					
4	CULTEC	Storage Node	788.46	792.00	788.46		0.00	0.18	790.76				0.00	0.00
5	SURF-SOUTHERN	Storage Node	790.20	794.20	790.20		20.00	0.67	790.97				0.00	0.00

Exhibit A

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Reported Condition
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)		(min)	
1	DI	Pipe	SURF-SOUTHERN	BOX	4.00	790.20	790.16	1.0000	6.000	0.0150	0.66	0.49	1.35	3.34	0.50	1.00	9.00	SURCHARGED
2	Link-UG	Pipe	CULTEC	BOX	14.00	790.02	790.16	-1.0000	6.000	0.0150	0.18	0.49	0.36	1.95	0.50	1.00	9.00	SURCHARGED
3	OUT	Orifice	BOX	POST-DEVEL		790.16	790.10		6.000		0.57							

Exhibit A

Subbasin Hydrology

Subbasin : POST-SOUTH

Input Data

Area (ac) 0.26
Weighted Curve Number 95.28
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		95.28

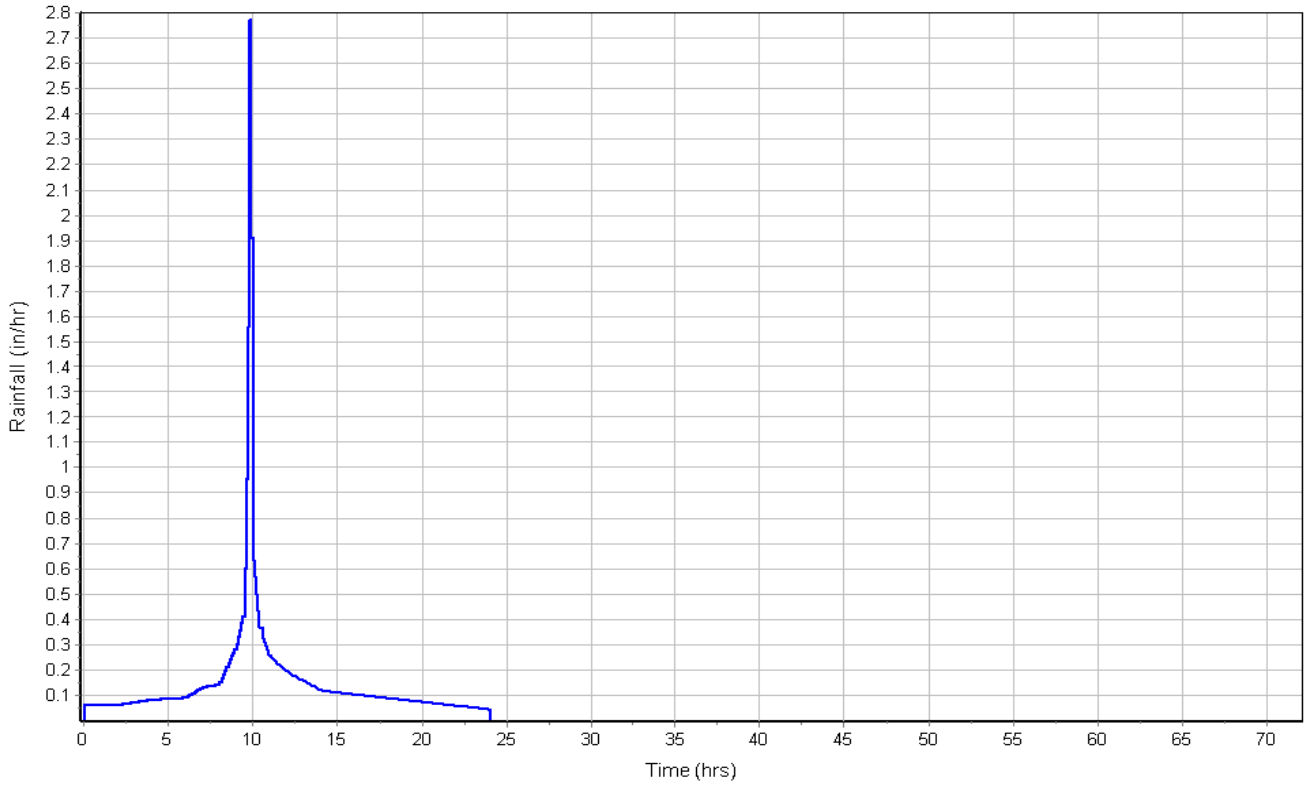
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 3.15
Peak Runoff (cfs) 0.67
Weighted Curve Number 95.28
Time of Concentration (days hh:mm:ss) 0 00:03:54

Exhibit A

Subbasin : POST-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

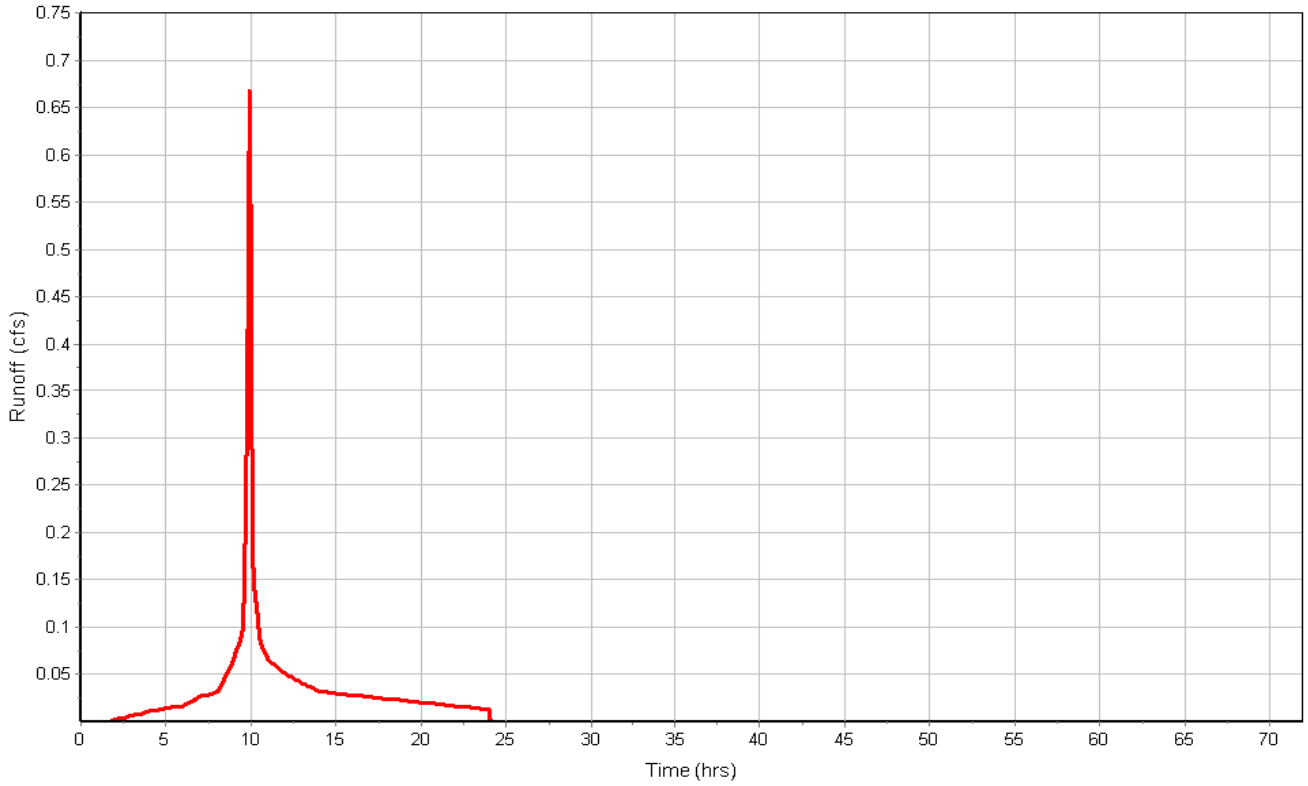


Exhibit A

Subbasin : PRE-SOUTH

Input Data

Area (ac) 0.26
Weighted Curve Number 94.98
Rain Gage ID ..... NOAA

Composite Curve Number

Soil/Surface Description	Area (acres)	Soil Group	Curve Number
-	0.03	-	72.00
-	0.23	-	98.00
Composite Area & Weighted CN	0.26		94.98

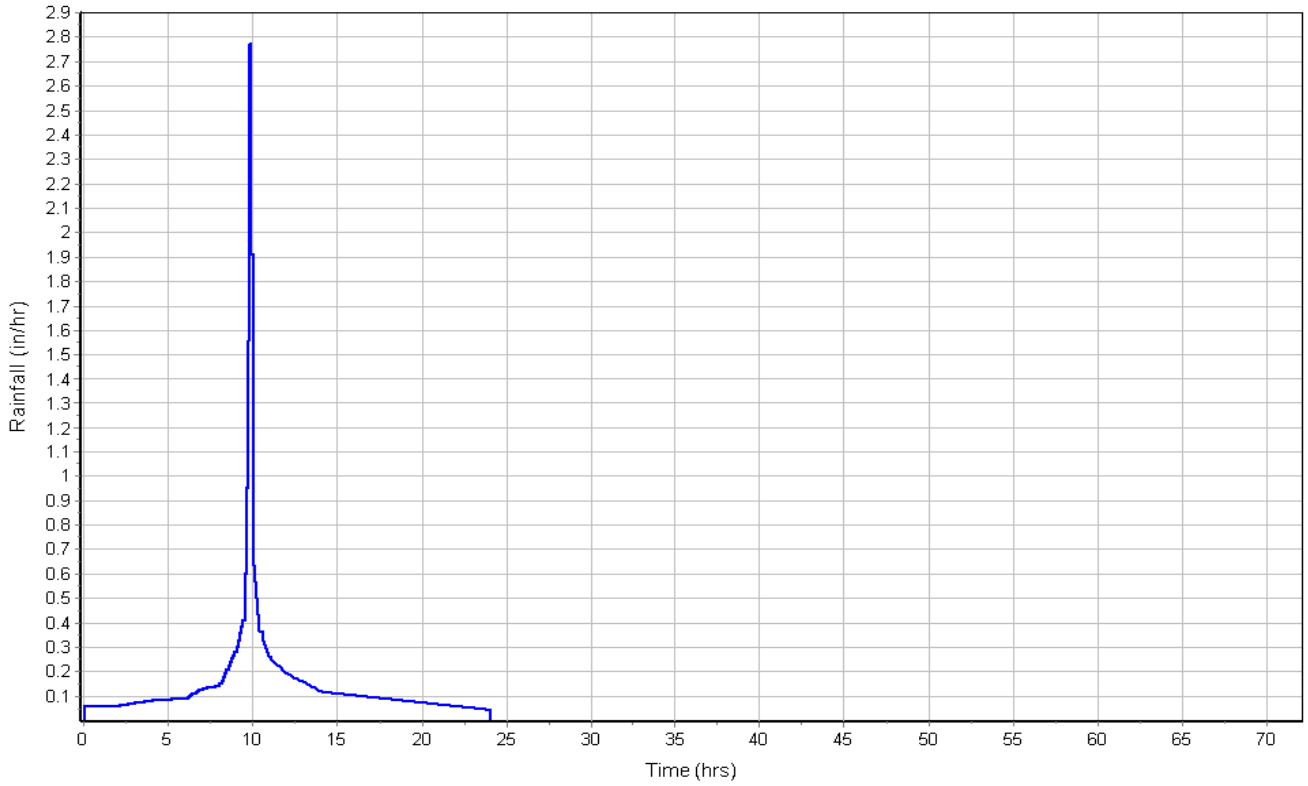
Subbasin Runoff Results

Total Rainfall (in) 3.68
Total Runoff (in) 3.11
Peak Runoff (cfs) 0.67
Weighted Curve Number 94.98
Time of Concentration (days hh:mm:ss) 0 00:03:54

Exhibit A

Subbasin : PRE-SOUTH

Rainfall Intensity Graph



Runoff Hydrograph

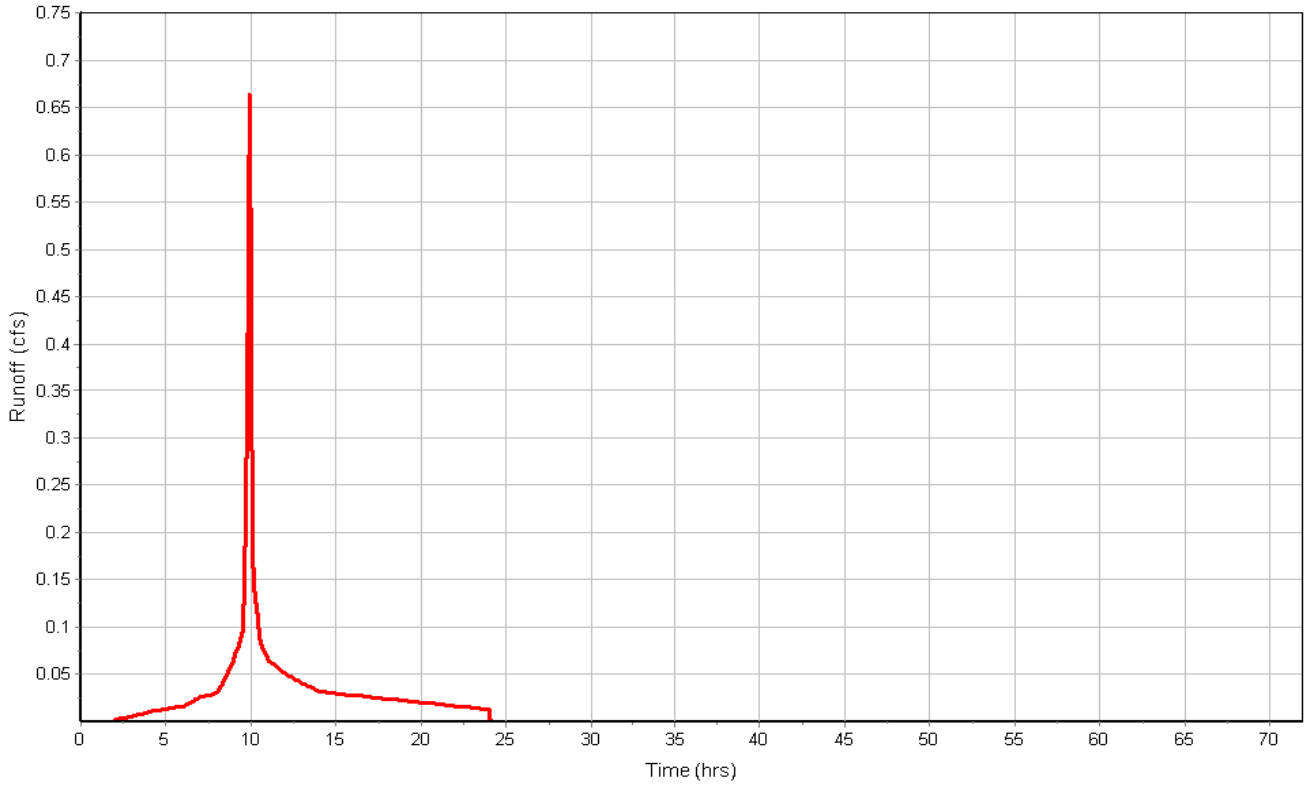


Exhibit A

Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft²)	Minimum Pipe Cover (in)
1	BOX	790.16	794.00	3.84	790.16	0.00	6.00	-788.00	0.00	0.00

Exhibit A

Junction Results

SN	Element ID	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Max HGL Elevation Attained (ft)	Max HGL Depth Attained (ft)	Max Surcharge Depth Attained (ft)	Min Freeboard Attained (ft)	Average HGL Elevation Attained (ft)	Average HGL Depth Attained (ft)	Time of Max HGL Occurrence (days hh:mm)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	BOX	0.66	0.00	790.76	0.60	0.00	3.24	790.18	0.02	0 10:02	0 00:00	0.00	0.00

Exhibit A

Pipe Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1	DI	4.00	790.20	0.00	790.16	0.00	0.04	1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1
2	Link-UG	14.00	790.02	1.56	790.16	0.00	-0.14	-1.0000	CIRCULAR	6.000	6.000	0.0150	0.5000	0.5000	0.0000	0.00	No	1

Exhibit A

Pipe Results

SN	Element ID	Peak Flow (cfs)	Time of Peak Flow Occurrence (days hh:mm)	Design Flow Capacity (cfs)	Peak Flow/ Design Flow Ratio	Peak Flow Velocity (ft/sec)	Travel Time (min)	Peak Flow Depth (ft)	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)	Froude Number	Reported Condition
1	DI	0.66	0 10:00	0.49	1.35	3.34	0.02	0.50	1.00	9.00		SURCHARGED
2	Link-UG	0.18	0 09:51	0.49	0.36	1.95	0.12	0.50	1.00	9.00		SURCHARGED

Exhibit A

Storage Nodes

Storage Node : CULTEC

Input Data

Invert Elevation (ft)	788.46
Max (Rim) Elevation (ft)	792.00
Max (Rim) Offset (ft)	3.54
Initial Water Elevation (ft)	788.46
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

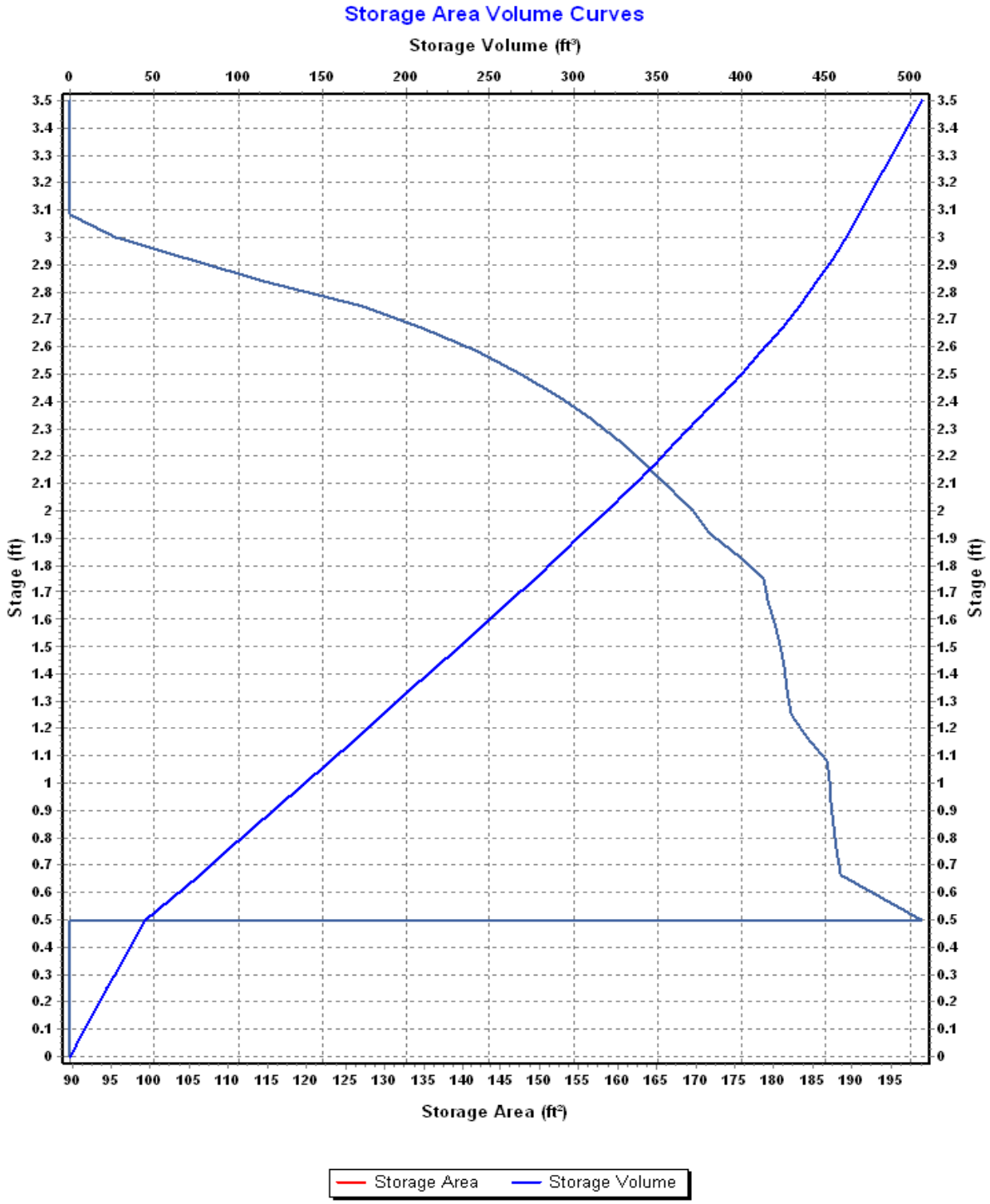
Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SOUTH-UG

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0.000	89.7120	0.000
0.083	89.7120	7.45
0.167	89.7120	14.99
0.250	89.7120	22.44
0.333	89.7120	29.89
0.417	89.7120	37.43
0.500	89.712	44.88
.501	198.9288	45.02
0.667	188.5968	77.18
0.750	187.9920	92.81
0.833	187.6896	108.40
0.917	187.3872	124.15
1.000	187.0848	139.69
1.083	186.7824	155.21
1.167	184.3632	170.80
1.250	182.2464	186.01
1.333	181.6416	201.11
1.417	181.3392	216.36
1.500	180.7344	231.39
1.583	180.1296	246.37
1.667	179.2224	261.46
1.750	178.6176	276.31
1.833	175.2912	291.00
1.917	171.6624	305.57
2.000	169.5456	319.73
2.083	166.5216	333.68
2.167	163.4976	347.54
2.250	160.1712	360.97
2.333	156.5424	374.11
2.417	152.3088	387.08
2.500	147.4704	399.52
2.583	142.0272	411.53
2.667	135.0720	423.17
2.750	127.2096	434.05
2.833	115.1136	444.11
2.917	105.1344	453.36
3.000	95.4576	461.68
3.083	89.7120	469.36
3.167	89.7120	476.90
3.250	89.7120	484.35
3.333	89.7120	491.80
3.417	89.7120	499.34
3.500	89.7120	506.79

Exhibit A


Storage Area Volume Curves
Storage Volume (ft³)
0 50 100 150 200 250 300 350 400 450 500
Stage (ft)
3.5 3.4 3.3 3.2 3.1 3 2.9 2.8 2.7 2.6 2.5 2.4 2.3 2.2 2.1 2 1.9 1.8 1.7 1.6 1.5 1.4 1.3 1.2 1.1 1 0.9 0.8 0.7 0.6 0.5 0.4 0.3 0.2 0.1 0
Stage (ft)
Storage Area (ft²)
90 95 100 105 110 115 120 125 130 135 140 145 150 155 160 165 170 175 180 185 190 195
Storage Area
Storage Volume

Storage Node : CULTEC (continued)

Output Summary Results

Peak Inflow (cfs)	0.18
Peak Lateral Inflow (cfs)	0.00
Peak Outflow (cfs)	0.10
Peak Exfiltration Flow Rate (cfm)	0.14
Max HGL Elevation Attained (ft)	790.76
Max HGL Depth Attained (ft)	2.3
Average HGL Elevation Attained (ft)	789.31
Average HGL Depth Attained (ft)	0.85
Time of Max HGL Occurrence (days hh:mm)	0 10:02
Total Exfiltration Volume (1000-ft³)	0.432
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A

Storage Node : SURF-SOUTHERN

Input Data

Invert Elevation (ft)	790.20
Max (Rim) Elevation (ft)	794.20
Max (Rim) Offset (ft)	4.00
Initial Water Elevation (ft)	790.20
Initial Water Depth (ft)	0.00
Ponded Area (ft²)	20.00
Evaporation Loss	0.00

Infiltration/Exfiltration

Exfiltration Rate (in/hr)	0.5000

Storage Area Volume Curves

Storage Curve : SURF-SOUTH

Stage (ft)	Storage Area (ft²)	Storage Volume (ft³)
0	4	0.000
4.2	4	16.80
4.201	65	16.83
4.45	174	46.59
4.7	298	105.59

Exhibit A

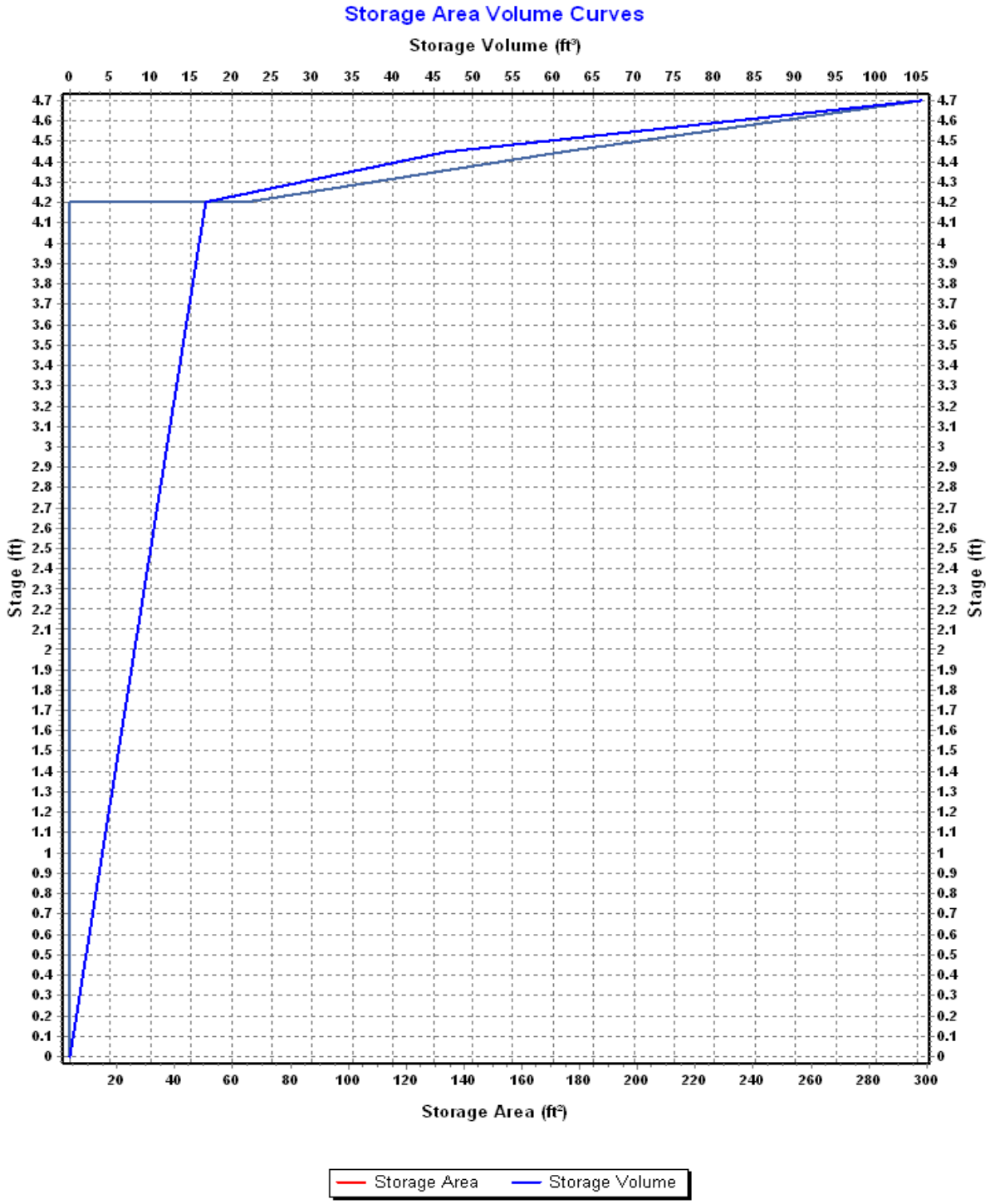

Storage Area Volume Curves
Storage Volume (ft³)
0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105
Stage (ft)
0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1 1.1 1.2 1.3 1.4 1.5 1.6 1.7 1.8 1.9 2 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4 4.1 4.2 4.3 4.4 4.5 4.6 4.7
20 40 60 80 100 120 140 160 180 200 220 240 260 280 300
Storage Area (ft²)
Storage Area
Storage Volume

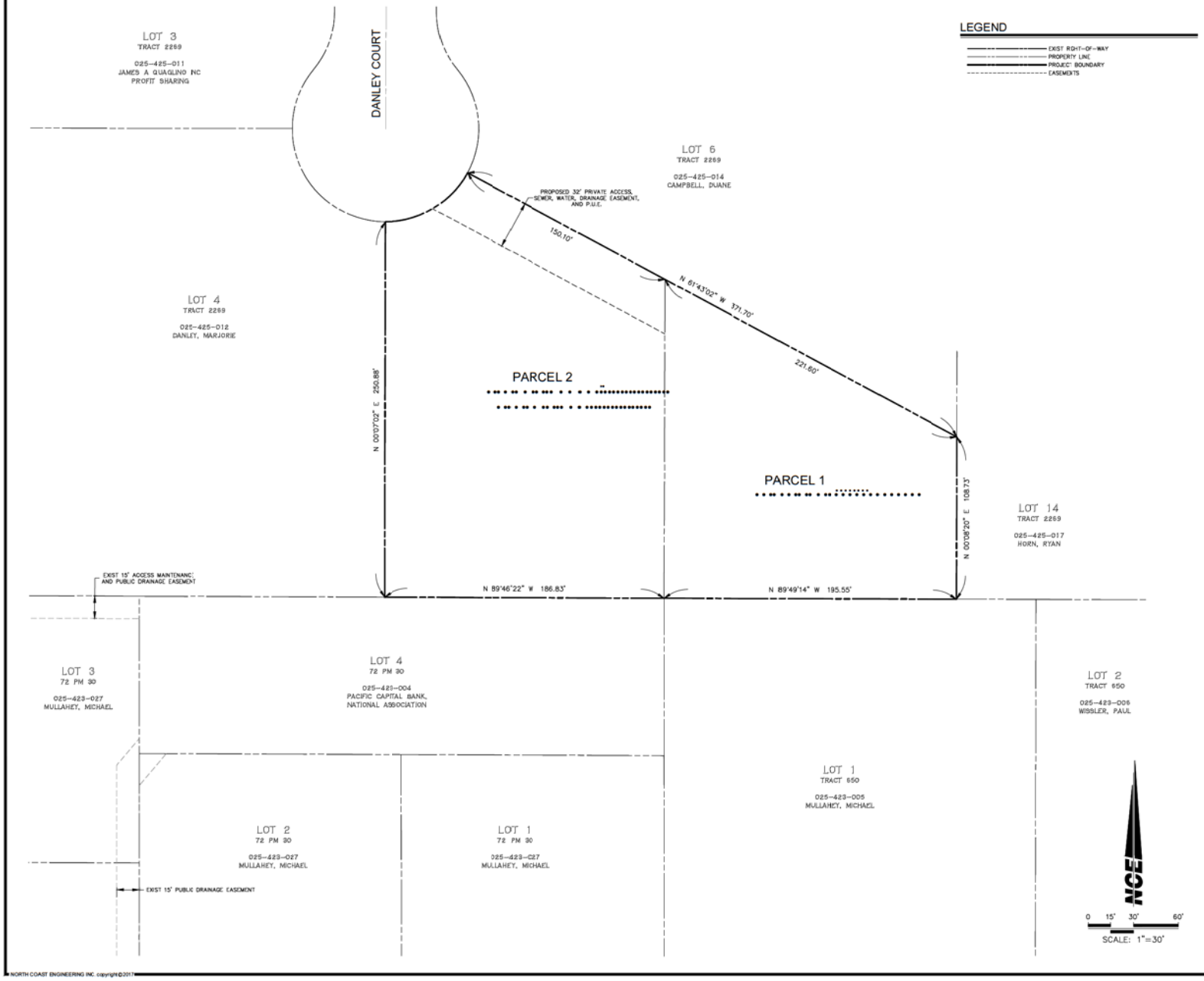
Exhibit A

Storage Node : SURF-SOUTHERN (continued)

Output Summary Results

Peak Inflow (cfs)	0.67
Peak Lateral Inflow (cfs)	0.67
Peak Outflow (cfs)	0.66
Peak Exfiltration Flow Rate (cfm)	0.00
Max HGL Elevation Attained (ft)	790.97
Max HGL Depth Attained (ft)	0.77
Average HGL Elevation Attained (ft)	790.23
Average HGL Depth Attained (ft)	0.03
Time of Max HGL Occurrence (days hh:mm)	0 10:00
Total Exfiltration Volume (1000-ft³)	0.004
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0.00

Exhibit A



LEGEND

- EXIST RIGHT-OF-WAY
- PROPERTY LINE
- PROJECT BOUNDARY
- EASEMENTS

LOCATION MAP

NO SCALE

SITE STATISTICS - OVERVIEW

TOTAL AREA	1.80 ACRES (78,556.3 SQ FT)
TOTAL LOTS	2
RECORD OWNER	MICHAEL & DIANE MULLAHEY FAMILY TRUST / 520 S. EUCLID, LLC
APPLICANT	MICHAEL MULLAHEY 2520 GOLDEN HILL ROAD PASO ROBLES, CALIFONIA 93446
SURVEYOR	NORTH COAST ENGINEERING, INC. 725 CRESTON RD., STE. B PASO ROBLES, CA 93466 JOHN SANDERS L.S. 5812 CONTACT: JOHN SANDERS (805) 239-3127
A.P.N.	025-425-013
PROPOSED USE OF PROPERTY	COMMERCIAL

LICENSED LAND SURVEYOR
JOHN R. SANDERS
5812
STATE OF CALIFORNIA

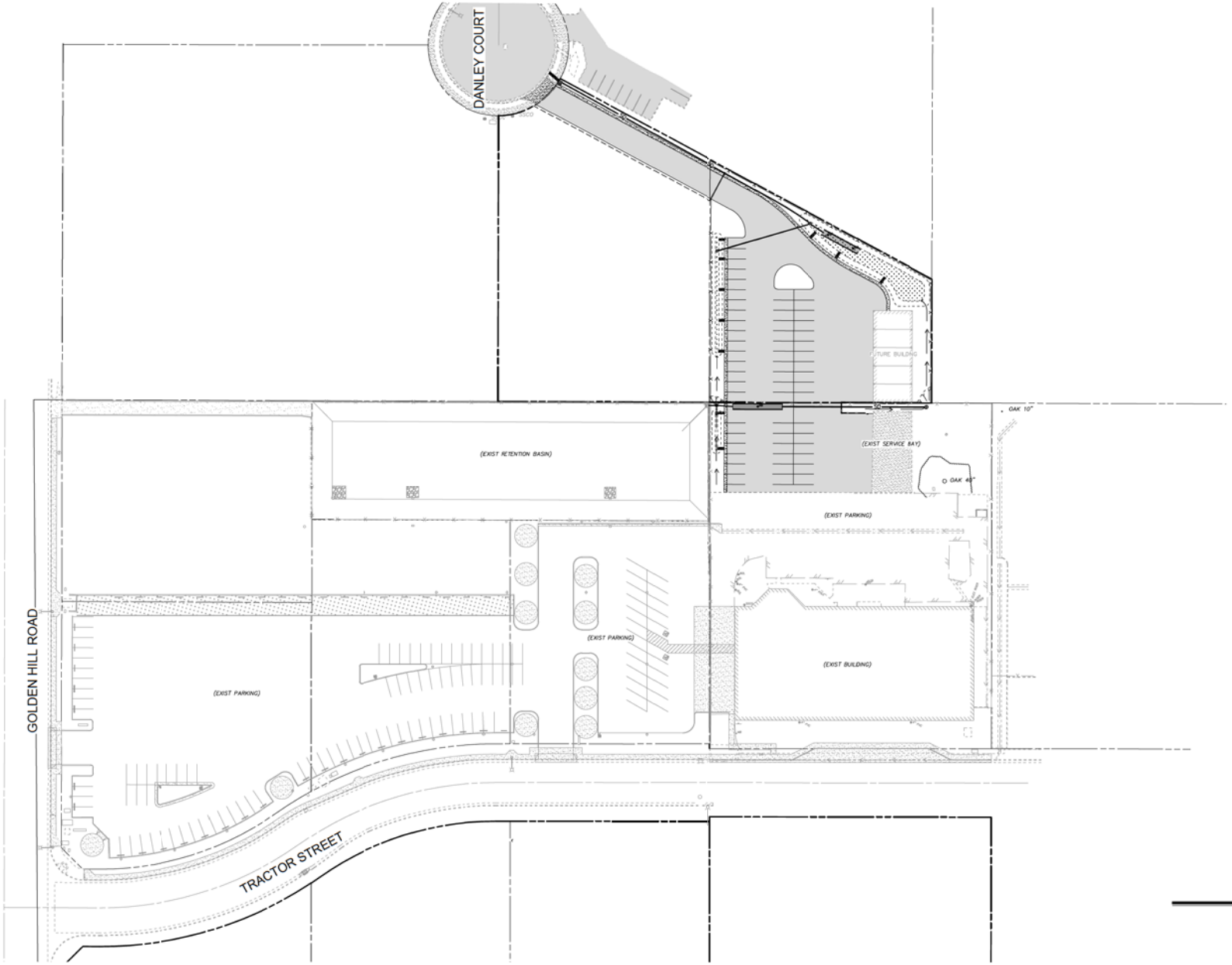
SHEET INDEX

VESTING TENTATIVE PARCEL MAP PR 16-0231

IN THE CITY OF PASO ROBLES, COUNTY OF SAN LUIS OBISPO, STATE OF CALIFORNIA
BEING A SUBDIVISION OF LOT 5 OF TRACT 2269, ACCORDING TO THE MAP RECORDED IN BOOK 19 OF TRACT MAPS, AT PAGE 86 IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY

725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.239.3127
NCE NORTH COAST ENGINEERING

R:\PRO\P0136\Design\Plan Set\Tentative Map\P0136 VTM.dwg, VTM, 4/5/2017 10:46:38 AM, EXL, 1:1

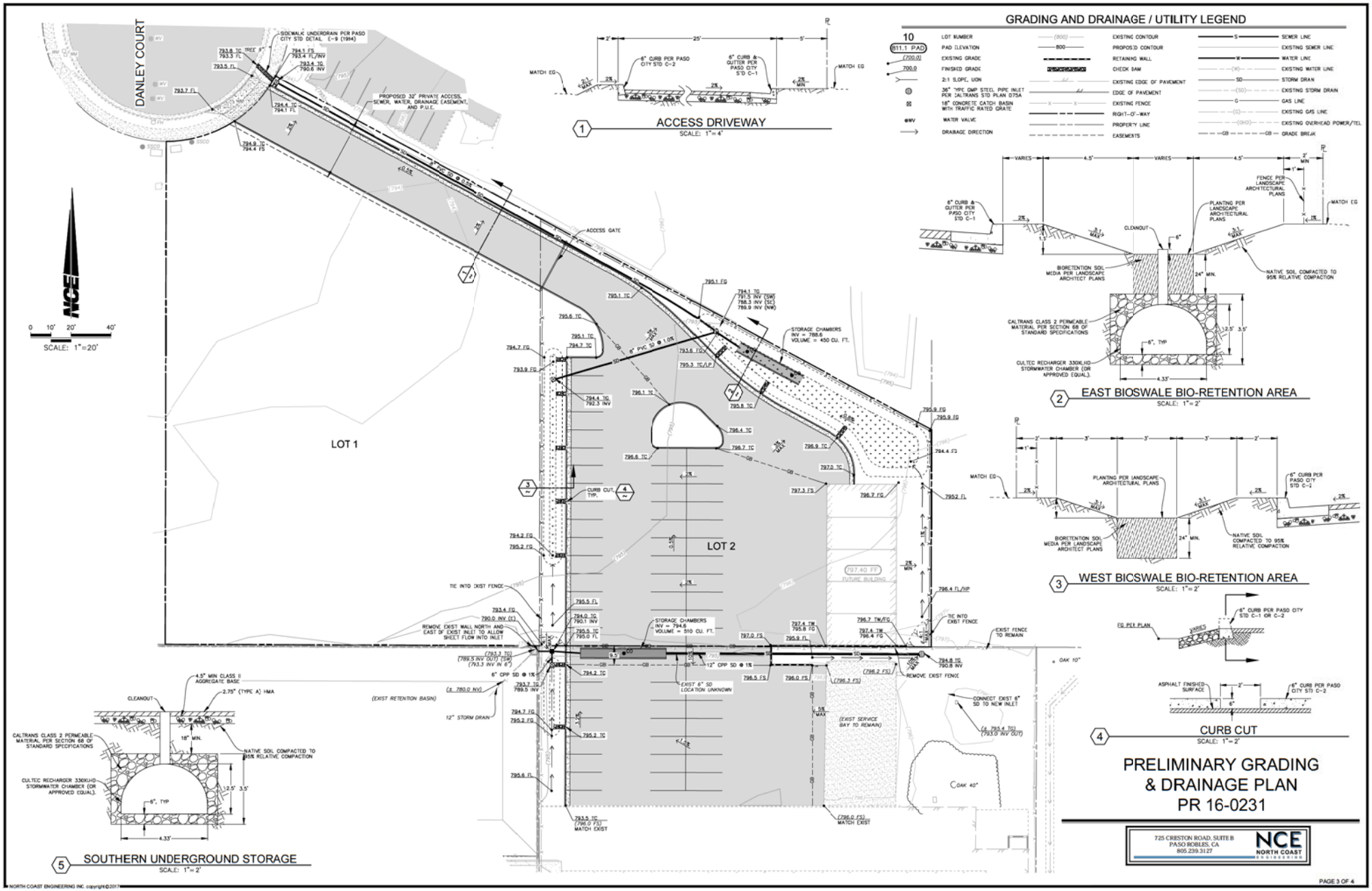


0 20' 40' 80'

SCALE: 1"=40'

OVERALL SITE PLAN
PR 16-0231

725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.239.3127
NCE NORTH COAST ENGINEERING


GRADING AND DRAINAGE / UTILITY LEGEND
LOT NUMBER
PAD ELEVATION
EXISTING GRADE
FINISHED GRADE
2:1 SLOPE, UON
36" "HPE GMP STEEL PIPE INLET PER CALTRANS STD PLAN D75A
18" CONCRETE CATCH BASIN WITH TRAFFIC RATED GRATE
WATER VALVE
DRAINAGE DIRECTION
EXISTING CONTOUR
PROPOSED CONTOUR
RETAINING WALL
CHECK DAM
EXISTING EDGE OF PAVEMENT
EDGE OF PAVEMENT
EXISTING FENCE
RIGHT-OF-WAY
PROPERTY LINE
EASEMENTS
SEWER LINE
EXISTING SEWER LINE
WATER LINE
EXISTING WATER LINE
STORM DRAIN
EXISTING STORM DRAIN
GAS LINE
EXISTING GAS LINE
EXISTING OVERHEAD POWER/TEL
GRADE BREAK
DANLEY COURT
SIDEWALK UNDERDRAIN PER PASO CITY STD DETAIL (~9 (1994)
PROPOSED 32' PRIVATE ACCESS, SEWER, WATER, DRAINAGE EASEMENT, AND P.U.E.
ACCESS GATE
LOT 1
LOT 2
STORAGE CHAMBERS INV = 788.6 VOLUME = 450 CU. FT.
STORAGE CHAMBERS INV = 794.6 VOLUME = 510 CU. FT.
CURB CUT, TYP.
TIE INTO EXIST FENCE
REMOVE EXIST WALL NORTH AND EAST OF EXIST INLET TO ALLOW SHEET FLOW INTO INLET
EXIST FENCE TO REMAIN
REMOVE EXIST FENCE
EXIST 6" SD LOCATION UNKNOWN
(EXIST RETENTION BASIN)
12" STORM DRAIN
6" CPP SD @ 1%
12" CPP SD @ 1%
CONNECT EXIST 6" SD TO NEW INLET
(EXIST SERVICE BAY TO REMAIN)
OAK 10"
OAK 40"
FUTURE BUILDING
MATCH EXIST
SCALE: 1"=20'
1 ACCESS DRIVEWAY SCALE: 1"=4'
2 EAST BIOSWALE BIO-RETENTION AREA SCALE: 1"=2'
3 WEST BICSWALE BIO-RETENTION AREA SCALE: 1"=2'
4 CURB CUT SCALE: 1"=2'
5 SOUTHERN UNDERGROUND STORAGE SCALE: 1"=2'
6" CURB PER PASO CITY STD C-2
6" CURB & GUTTER PER PASO CITY STD C-1
MATCH EG
FENCE PER LANDSCAPE ARCHITECTURAL PLANS
PLANTING PER LANDSCAPE ARCHITECTURAL PLANS
CLEANOUT
BIORETENTION SOIL MEDIA PER LANDSCAPE ARCHITECT PLANS
NATIVE SOIL COMPACTED TO 95% RELATIVE COMPACTION
CALTRANS CLASS 2 PERMEABLE MATERIAL PER SECTION 68 OF STANDARD SPECIFICATIONS
CULTEC RECHARGER 330XLHD STORMWATER CHAMBER (OR APPROVED EQUAL).
4.5" MIN CLASS II AGGREGATE BASE
2.75" (TYPE A) HMA
EG PER PLAN
ASPHALT FINISHED SURFACE
725 CRESTON ROAD, SUITE B
PASO ROBLES, CA
805.239.3127
NCE NORTH COAST ENGINEERING
PRELIMINARY GRADING & DRAINAGE PLAN
PR 16-0231
PAGE 3 OF 4
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Brachychiton acerifolius
Flame Tree

Pistacia chinensis
Chinese Pistache

Ulmus parvifolia
Evergreen Elm

Arbutus 'Marina'
Strawberry Tree

Gleditsia triacanthos
Honey Locust

Chilopsis linearis
Desert Willow

Quercus agrifolia
Coast Live Oak

SUMMERS MURPHY & PARTNERS
ENVIRONMENTAL DESIGN

Mullahey Dodge

12/12/2016

Hydrozone Chart & Landscape Water Use Summary

Maximum Annual Water Allowance (MAWA)

the total square footage of landscape = 10,275 S.F.
the Hist. ETo for the area = 49

MAWA = 334 ccf / yr
MAWA = 249,832 gal / yr

Hydrozone Chart

Hydrozone # 1 (Shrub Drip) Plant Factor = 0.2 (Low)
square footage of hydrozone = 10,275
hydrozone irrigation efficiency = 0.9
EAWU = 93 ccf / yr

Estimated Annual Water Use (EAWU)

SubTotal EAWU = 93 ccf / yr
Irrigation System Efficiency Factor 0.85 %
EAWU = 109 ccf / yr
EAWU = 81,840 gal / yr

Landscape Water Use Summary

			ccf saved
MAWA	249,832 gal / yr	334 ccf / yr	EAWU < MAWA = 225
EAWU	81,840 gal / yr	109 ccf / yr	(this number must be positive)
Percentage of MAWA used	33%		

Statement of Water Conserving Irrigation Design

The following principles of irrigation design utilized on this project are directed specifically as conserving water and improvng the efficiency of the irrigation system:

- All irrigation shall be drip or dripline emitters. No overhead spray heads will be used.
- Irrigation hydrozones shall be adjusted according to water needs and weather.
- Utilization of irrigation system master valve.
- Utilization of irrigation system "smart controller" with water budgeting feature.
- Utilization of irrigation system flow sensor.
- Utilization of rain shut-off device connected to irrigation controller.

To help maintain the irrigation efficiency as intended in the design, Irrigation system shall be tested and maintained on a monthly basis by the maintenance staff.

Plant List

	*WUCOLS RATING
ACCENT TREES	
BRACHYCHITON ACERIFOLIUS / FLAME TREE	L
PISTACIA CHINENSIS / CHINESE PISTACHE	L
ULMUS PARVIFOLIA / EVERGREEN ELM	L
PARKING LOT / GROVE TREES	
ARBUTUS 'MARINA' / STRAWBERRY TREE	L
CHILOPSIS LINEARIS / DESERT WILLOW	VL
GLEDITSIA TRIACANTHOS / HONEY LOCUST	L
QUERCUS AGRIFOLIA / COAST LIVE OAK	VL
ACCENT SHRUBS	
BOUGAINVILLEA SPP. / BOUGAINVILLEA	L
CISTUS SPP. / ROCKROSE	L
ECHIUM CANDICANS / PRIDE OF MADEIRA	L
KNIPHOFIA UVARIA / RED-HOT POKER	L
PENSTEMON HETEROPHYLLUS 'MARGARITA BOP' / BLUE BEDDER	L
PEROVSKIA ATRIPLICIFOLIA / RUSSIAN SAGE	L
SHRUBS and PERENNIALS	
ARBUTUS UNEDO 'ELFIN KING'/ DWARF STRAWBERRY TREE	L
CALLISTEMON 'LITTLE JOHN' / DWARF BOTTLEBRUSH	L
HETEROMELLES ARBUTIFOLIA / TOYON	L
RHAMNUS CALIFORNICA 'EVE CASE' / COFFEEBERRY	L
RHAPHIOLEPIS INDICA 'PINK LADY' / INDIA HAWTHORN (PINK)	L
GROUNDCOVERS	
BACCHARIS PILULARIS 'PIGEON POINT' / DWARF COYOTE BRUSH	L
ERIGERON KARVINSKIANUS / FLEABANE	L
MYOPORUM PARVIFOLIUM 'PUTAH CREEK' / TRAILING MYOPORUM	L
ROSMARINUS OFFICINALIS 'HUNTINGTON CARPET' / (ROS)	L
SALVIA 'DARA'S CHOICE' / DARA'S CHOICE SAGE	L
BIOFILTRATION BASINS (use Bioretention Soil Media per City Standard):	
ACHILLEA MILLEFOLIUM 'SUNNY SEDUCTION' / YELLOW YARROW	L
CAREX DIVULSA / BERKELEY SEDGE	L
IRIS DOUGLASIANA / DOUGLAS IRIS	L
JUNCUS PATENS 'ELK BLUE' / CALIFORNIA GRAY RUSH	L
MUHLENBERGIA LINDHEIMERI / LINDHEIMER'S MUHLY	L
SALVIA SPATHACEA / HUMMINGBIRD SAGE	L
SATUREJA DOUGLASII / YERBA BUENA	L

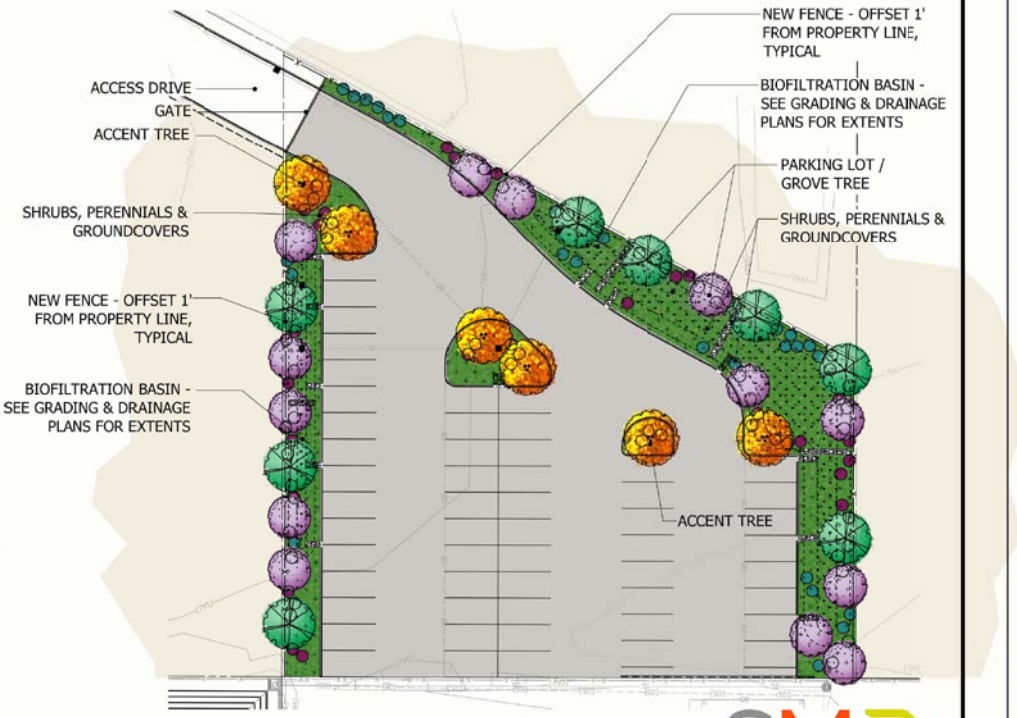
MULCH
MULCH ALL GROUND COVER AND PLANTER AREAS WITH 3" MINIMUM LAYER 'WALK-ON' BARK.

*WATER-USE EVALUATION OF PLANT MATERIALS
WATER USE OF PROPOSED PLANTS HAVE BEEN EVALUATED USING THE "WATER USE CLASSIFICATION OF LANDSCAPE SPECIES" (WUCOLS IV, UNIVERSITY OF CALIFORNIA COOPERATIVE EXTENSION.)

Water Conservation Notes

The following water conservation techniques shall be employed in this Project:

- Water conserving plants, defined as "Low" or "Very Low" in the "Water Use Classification of Landscape Species" (WUCOLS IV, University of California Cooperative Extension), shall be utilized in 100% of the total plant area.
- Irrigation system shall be separated into distinct hydrozones based on plant material types, exposure and orientation.
- Soil amendments and mulch shall be utilized to improve water holding capacity of soil.
- Automatic irrigation system shall utilize "smart controller" technology with water budgeting feature to adjust water application based on soil moisture and/or local weather data.
- Lawn is not used.



NORTH

SCALE: 1"=20'-0"

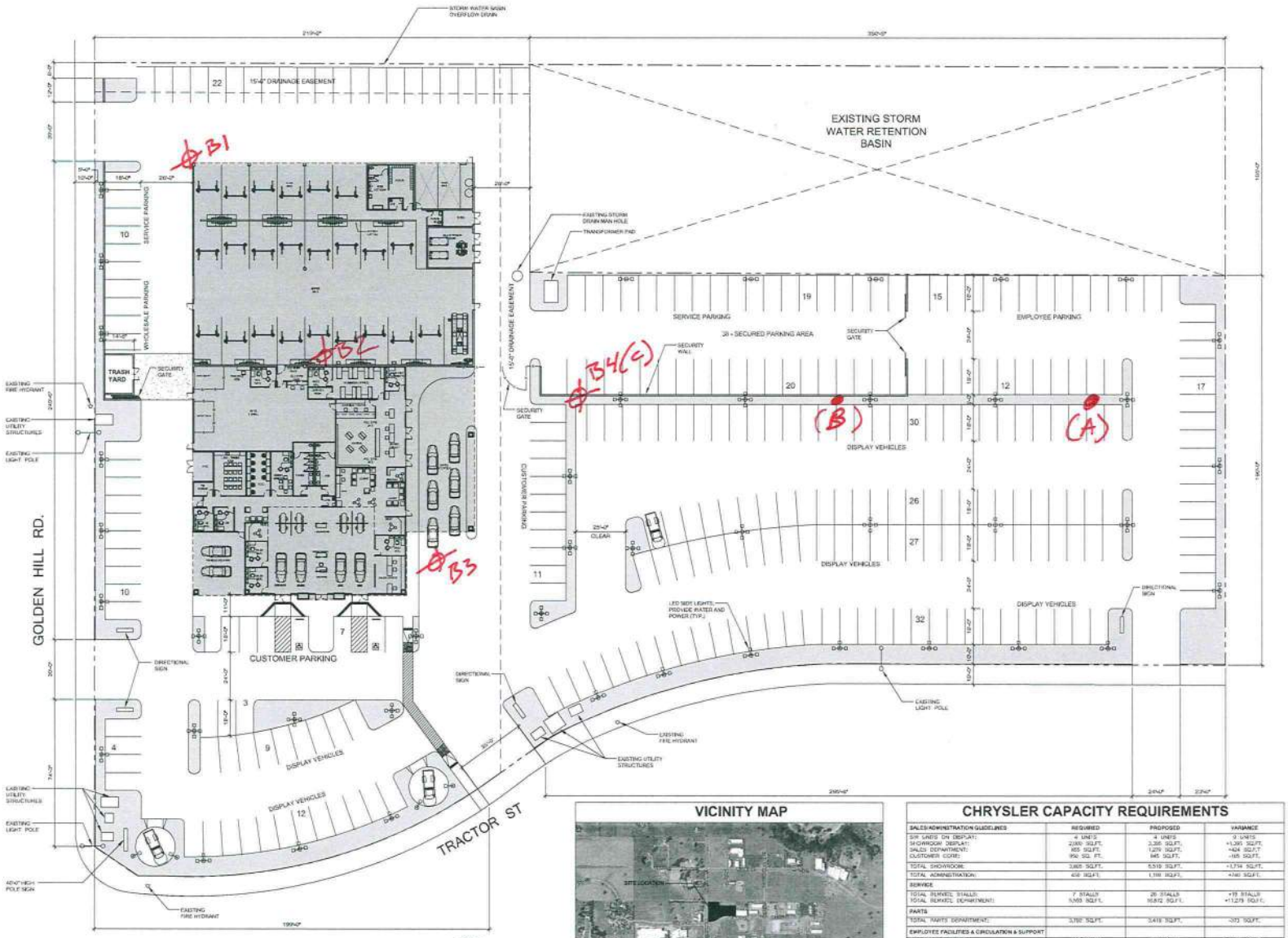
0 20 40 60 80

SMP
ENVIRONMENTAL DESIGN

SUMMERS/MURPHY & PARTNERS, INC.
979 OSOS STREET, SUITE B6
SAN LUIS OBISPO, CA 93401
(805) 439-3209

12-14-2016 SMP #742473

Exhibit I: Infiltration Testing Report



PROPOSED SITE PLAN

SCALE: 1" = 20'

NORTH

VICINITY MAP

CHRYSLER CAPACITY REQUIREMENTS

SALES/ADMINISTRATION GUIDELINES	REQUIRED	PROPOSED	VARIANCE
SIX UNITS ON DISPLAY:	4 UNITS 2,080 SQ.FT.	4 UNITS 3,355 SQ.FT.	0 UNITS +1,275 SQ.FT.
SHOWROOM (REPLAY):	855 SQ.FT.	1,279 SQ.FT.	+424 SQ.FT.
SALES DEPARTMENT:	950 SQ. FT.	845 SQ.FT.	-105 SQ.FT.
CUSTOMER CORE:			
TOTAL SHOWROOM:	3,805 SQ.FT.	5,519 SQ.FT.	+1,714 SQ.FT.
TOTAL ADMINISTRATION:	450 SQ.FT.	1,190 SQ.FT.	+740 SQ.FT.
SERVICE			
TOTAL SERVICE STALLS:	7 STALLS	26 STALLS	+19 STALLS
TOTAL SERVICE DEPARTMENT:	5,955 SQ.FT.	16,672 SQ.FT.	+11,273 SQ.FT.
PARTS			
TOTAL PARTS DEPARTMENT:	3,750 SQ.FT.	3,418 SQ.FT.	-373 SQ.FT.
EMPLOYEE FACILITIES & CIRCULATION & SUPPORT			
EMPLOYEE FACILITIES:	240 SQ.FT.	1,588 SQ.FT.	+1,207 SQ.FT.
BUILDING SUPPORT & CIRCULATION:	2,082 SQ.FT.	1,330 SQ.FT.	-752 SQ.FT.
TOTAL BUILDING			
TOTAL BUILDING AREA:	15,900 SQ.FT.	29,940 SQ.FT.	+13,965 SQ.FT.
TOTAL PARKING STORAGE			
NEW VEHICLE DISPLAY/STORAGE:	29,496 SQ.FT.	32,250 SQ.FT.	+1,754 SQ.FT.
USED/PRE-OWNED VEHICLE DISPLAY/STORAGE:	14,400 SQ.FT.	18,256 SQ.FT.	+3,856 SQ.FT.
CUSTOMER AND EMPLOYEE PARKING:	21,384 SQ.FT.	31,900 SQ.FT.	+9,516 SQ.FT.
TOTAL PARKING SPACES:	206 SPACES	318 SPACES	+112 SPACES
TOTAL PARKING AREA:	65,612 SQ.FT.	72,006 SQ.FT.	+6,588 SQ.FT.
TOTAL LAND & BUILDING			
TOTAL LAND & BUILDING AREA:	81,373 SQ.FT.	157,191 SQ.FT.	+75,818 SQ.FT.
TOTAL LAND & BUILDING + DSDC COMPLIANCE:	87,918 SQ.FT. 2.0 ACRES	157,191 SQ.FT. 3.6 ACRES	69,273 SQ.FT. +1.6 ACRES

DFA
DENNIS J. FLYNN ARCHITECTS, INC.
366 NORTH BUCKLEY STREET
FULLERTON, CALIFORNIA 92832
(714) 879-4650 FAX (714) 879-4620

08/12/13
07/22/13 CONCEPTUAL DESIGN
08/15/13 CONCEPTUAL DESIGN
08/29/13 CONCEPTUAL DESIGN
09/12/13 CONCEPTUAL DESIGN
09/24/13 CONCEPTUAL DESIGN

PASO ROBLES
TRACTOR ST
PASO ROBLES, CA 93446

CHRYSLER JEEP DODGE RAM

PROPOSED SITE PLAN

PROJECT 2101

PRELIMINARY BID PLANS. THESE DOCUMENTS HAVE NOT BEEN COMPLETELY REVIEWED BY THE OWNER, PROJECT TEAM OR THE BUILDING DEPARTMENT. THESE DOCUMENTS ARE SUBJECT TO REVISION. FUTURE PLAN REVISIONS MAY NOT BE CLEARLY INDICATED AS SUCH. ANY WORK MADE ON CONSTRUCTION PERFORMED BASED ON THESE DOCUMENTS IS AT THE RISK OF THOSE MAKING THE BID OR PERFORMING THE WORK.

A1.01

Exhibit A

Exhibit A

Earth Systems Pacific

Boring No. 1
PAGE 1 OF 1
JOB NO.: SL-17161-SA
DATE: 10/24/13

LOGGED BY: R. Gorman
DRILL RIG: Mobile B-53
AUGER TYPE: 6" Hollow Stem Auger

PASO ROBLES CHRYSLER DEALERSHIP
Tractor Street and Golden Hill Road
Paso Robles, California

DEPTH (feet)	USCS CLASS	SYMBOL	SOIL DESCRIPTION	SAMPLE DATA: INTERVAL (feet)	SAMPLE TYPE	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS PER 6 IN.
0	SC		CLAYEY SAND WITH GRAVEL: light brown, medium dense, dry (Fill)	1.0 - 3.0	○			29
2			brown, moist	2.0 - 3.5	■			20 20
5				5.0 - 6.5	■			5 6 9
6	SP		POORLY GRADED SAND: light brown, loose, moist (Older Alluvium)					
10			medium dense, coarse grained	10.0 - 11.5	●			11 11 13
13	SW		WELL GRADED SAND WITH GRAVEL: brown, medium dense, moist					
15				15.0 - 16.5	●			16 16 17
16			End of Boring @ 16.5' No subsurface water encountered					

LEGEND: ■ Ring Sample ○ Grab Sample ▭ Shelby Tube Sample ● SPT

NOTE: This log of subsurface conditions is a simplification of actual conditions encountered. It applies at the location and time of drilling. Subsurface conditions may differ at other locations and times.

Exhibit A

Earth Systems Pacific

Boring No. 2
PAGE 1 OF 1

LOGGED BY: R. Gorman
DRILL RIG: Mobile B-53
AUGER TYPE: 6" Hollow Stem Auger

JOB NO.: SL-17161-SA
DATE: 10/24/13

PASO ROBLES CHRYSLER DEALERSHIP
Tractor Street and Golden Hill Road
Paso Robles, California

DEPTH (feet)	USCS CLASS	SYMBOL	SOIL DESCRIPTION	SAMPLE DATA: INTERVAL (feet)	SAMPLE TYPE	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS PER 6 IN.
0	SP		POORLY GRADED SAND: light brown, loose, dry (Fill)					
2				2.0 - 3.5	Ring			5, 8, 7
4	SC		CLAYEY SAND WITH GRAVEL: brown, loose, moist, fine gravel (Older Alluvium)					
5				5.0 - 6.5	Ring			9, 13, 20
6			medium dense					
10	SW		WELL GRADED SAND: light brown, loose, moist	10.0 - 11.5	SPT			3, 5, 8
14	SC		CLAYEY SAND: brown, medium dense, very moist					
15				15.0 - 16.5	SPT			12, 18, 18
20				20.0 - 21.5	SPT			13, 10, 10
22			End of Boring @ 21.5' No subsurface water encountered					

LEGEND: Ring Sample; Grab Sample; Shelby Tube Sample; SPT

NOTE: This log of subsurface conditions is a simplification of actual conditions encountered. It applies at the location and time of drilling. Subsurface conditions may differ at other locations and times.

Exhibit A

Earth Systems Pacific

Boring No. 3
PAGE 1 OF 1
JOB NO.: SL-17161-SA
DATE: 10/24/13

LOGGED BY: R. Gorman
DRILL RIG: Mobile B-53
AUGER TYPE: 6" Hollow Stem Auger

PASO ROBLES CHRYSLER DEALERSHIP
Tractor Street and Golden Hill Road
Paso Robles, California

DEPTH (feet)	USCS CLASS	SYMBOL	SOIL DESCRIPTION	SAMPLE DATA: INTERVAL (feet)	SAMPLE TYPE	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS PER 6 IN.
0	SW		WELL GRADED SAND: light brown, loose, moist (Fill)	2.0 - 3.5	Ring			5 5 4
3	SP		POORLY GRADED SAND: brown, loose, moist (Older Alluvium)					
5			dense	5.0 - 6.5	Ring			9 12 50/5.0"
10	SW		WELL GRADED SAND: light brown, medium dense, very moist	10.0 - 11.5	SPT			9 12 15
15	SC		CLAYEY SAND: brown, medium dense, moist, fine grained	15.0 - 16.5	SPT			4 7 9
16.5			End of Boring @ 16.5' No subsurface water encountered					

LEGEND: ■ Ring Sample ○ Grab Sample ▭ Shelby Tube Sample ● SPT

NOTE: This log of subsurface conditions is a simplification of actual conditions encountered. It applies at the location and time of drilling. Subsurface conditions may differ at other locations and times.

Exhibit A

Earth Systems Pacific

Boring No. 4
PAGE 1 OF 1

LOGGED BY: R. Gorman
DRILL RIG: Mobile B-53
AUGER TYPE: 6" Hollow Stem Auger

JOB NO.: SL-17161-SA
DATE: 10/24/13

PASO ROBLES CHRYSLER DEALERSHIP
Tractor Street and Golden Hill Road
Paso Robles, California

DEPTH (feet)	USCS CLASS	SYMBOL	SOIL DESCRIPTION	SAMPLE DATA: INTERVAL (feet)	SAMPLE TYPE	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS PER 6 IN.
0	SP		POORLY GRADED SAND: light brown, loose, moist (Fill)					
5			dense					
7	SW		WELL GRADED SAND WITH GRAVEL: brown, loose, very moist (Older Alluvium)					
12			SANDY LEAN CLAY: brown, medium dense, very moist					
15			End of Boring @ 15.0' No subsurface water encountered					

LEGEND: ■ Ring Sample ○ Grab Sample ▭ Shelby Tube Sample ● SPT

NOTE: This log of subsurface conditions is a simplification of actual conditions encountered. It applies at the location and time of drilling. Subsurface conditions may differ at other locations and times.

Exhibit A

PROJECT: Paso Robles Chysler Dealership SL-17164-SA

INFILTRATION TEST RESULTS

INFILTRATION TEST: C

DATE DRILLED: 10/24/2013 **TEST HOLE DIAMETER: 8 inches**

DATE TESTED:10/24/2013 **TEST HOLE DEPTH: 15.0 feet**

TECHNICIAN: PF **TEST DURATION: .75 hours**

CONSTANT HEAD DATA

Time of Constant Head: 30 minutes

Volume Added During Constant Head: 16.0 ft^{3*}

*Could not fill above 8 feet with gravity fed garden hose

FALLING HEAD DATA

INTERVAL (Minutes)	READING (Feet)	INCREMENTAL FALL (Feet)	INFILTRATION RATE (Minutes / Inch)	INFILTRATION RATE (Inches / Hour)
---	8.00	---	---	---
2	9.70	1.70	0.10	612
10	15.00	5.30	0.16	382

Exhibit A

PROJECT: Paso Robles Chysler Dealership SL-17164-SA

INFILTRATION TEST RESULTS

INFILTRATION TEST: B

DATE DRILLED: 10/24/2013 **TEST HOLE DIAMETER: 8 inches**

DATE TESTED:10/24/2013 **TEST HOLE DEPTH: 4.5 feet**

TECHNICIAN: PF **TEST DURATION: 1.5 hours**

CONSTANT HEAD DATA

Time of Constant Head: 30 minutes

Volume Added During Constant Head: 1.1 ft^3

FALLING HEAD DATA

INTERVAL (Minutes)	READING (Feet)	INCREMENTAL FALL (Feet)	INFILTRATION RATE (Minutes / Inch)	INFILTRATION RATE (Inches / Hour)
---	0.97	---	---	---
15	1.18	0.21	5.95	10
15	1.30	0.12	10.42	6
15	1.45	0.15	8.33	7
15	1.51	0.06	20.83	3

Exhibit A

PROJECT: Paso Robles Chysler Dealership SL-17164-SA

INFILTRATION TEST RESULTS

INFILTRATION TEST: A

DATE DRILLED: 10/24/2013 **TEST HOLE DIAMETER: 8 inches**

DATE TESTED:10/24/2013 **TEST HOLE DEPTH: 4.5 feet**

TECHNICIAN: PF **TEST DURATION: 1.5 hours**

CONSTANT HEAD DATA

Time of Constant Head: 30 minutes

Volume Added During Constant Head: 0.2 ft^3

FALLING HEAD DATA

INTERVAL (Minutes)	READING (Feet)	INCREMENTAL FALL (Feet)	INFILTRATION RATE (Minutes / Inch)	INFILTRATION RATE (Inches / Hour)
---	0.97	---	---	---
15	1.18	0.21	5.95	10
15	1.30	0.12	10.42	6
15	1.45	0.15	8.33	7
15	1.51	0.06	20.83	3